Inland-Ryerson Construction Products Company

1973

METAL DECKING
cellular, ribbed & fluted

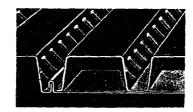


# Inryco<sup>®</sup> Hi-Bond<sup>®</sup> Celluflor<sup>®</sup> and Floor Deck

### **Hi-Bond Exclusive**

Ten years of extensive use in projects of all kinds and sizes have proven the successful performance of Inryco Hi-Bond deck, which eliminates the need for positive reinforcing bars, reducing material costs.

Introduced in 1961, the Hi-Bond lug made it possible for the first time to select a floor deck specifically designed to give composite slab action. Hi-Bond is now standard on all Inryco Celluflor and non-



cellular Inryco floor deck products.

The lugs are formed into the web of each rib of the deck (see illustration), providing a dual-action positive lateral and vertical mechanical bond between the deck and concrete—an effective composite design.

### Many Savings Possible

Besides the important savings inherent in composite design itself, Inryco floor decks make possible other opportunities to reduce costs.

New Types 3" V and NF<sub>v</sub> Celluflor, for instance, make possible even greater composite slab design savings because of their V-lock joint, which serves as an integral shear connector. Also, these 3" deep products are the first specifically designed and engineered for composite beam design, with wide ribs to receive shear connectors.

New 2" Type V Deck fills the design gap between 1½" and 3" decks. It is the most economical section for the 8' to 12' span range. (See page 16.)

Recent UL ratings on lighter gage cellular panels have also made possible a reduction in material costs. Raceway requirements now permit the use of 20 gage steel for both fluted and flat plates of cellular deck, provided cells are covered by a minimum of 1½" of concrete. This is a distinct economy over the former requirement: 18/16 gage covered by 2½" of concrete.

UL rated floor systems often save the **cost of applied fire-proofing**. Many building codes have 2 hour floor requirements. Inryco engineers conducted fire tests to earn the Underwriters' Laboratories' two and three hour fire ratings without fireproofing, thereby saving time and materials.

And, of course, there are the **traditional savings** in the use of steel deck over other types of construction: the elimination of temporary forms and shoring, the immediate use of the deck as a safe work platform for all trades, and the savings in labor because things move faster.

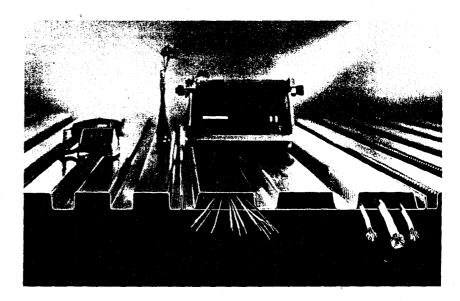
### Composite Design Assistance

Because of Inland-Ryerson's early involvement in composite beam design in the past decade, our engineers are highly skilled in many technical phases of it. This know-how is available to designers without charge through our sales engineers. Also available is a newly revised edition of the Inryco Composite Beam Design Manual.

### In-Floor Electrification

Inryco Celluflor's most common use is to accommodate electrical power and communications circuits. It has been thoroughly tested and approved by Underwriters' Laboratories, Inc., for safe electrical use and carries the UL label service for your assurance.

All types of Celluflor permit in-floor electrification within inches of virtually any location. It is a simple matter to reach a wired cell and the installation of floor outlets is quick and inexpensive.



### Savings Through Blends

Many times the electrical requirements of a building, both present and future, do not justify the use of a totally electrified floor. Instead, it may be adequate to provide only partial electrification.

Because they are fully compatible in both depth and side lock conditions, Inryco Celluflor and non-cellular Inryco floor decks can be used in economical blends that provide electrification cell patterns in a wide variety of combinations to meet virtually any design module the architect has decided to use.

### Savings in Cell Capacity

Inryco 15/8" NF and NF $_{\rm V}$  Celluflors offer 66% more cell capacity than standard profile 11/2" cellular decks, allowing important savings in future electrical service expansion. And because cells are wider, they permit 4" hand holes needed to accommodate large diameter cables.

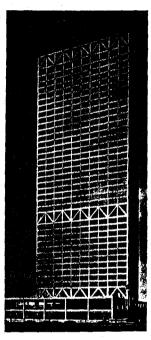
### Savings with Pre-set Inserts

Significant savings in electrification are made possible by the use of pre-set inserts. These are described in detail on pages 4 and 5.

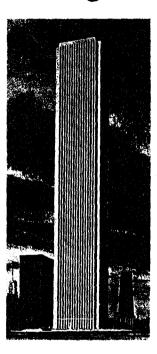
## Recent major projects using Inryco decks



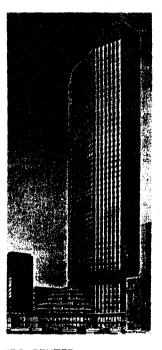
SEARS TOWER,
Chicago, Illinois
World's tallest structure,
110 stories. 3.7 million
sq. ft. of Inryco 3"V and
3"NFv deck.
Owner: Sears, Roebuck and
Company, Chicago.
Architects/Engineers:
Skidmore, Owings &
Merrill, Chicago.
Gen. Contractor: Diesel
Construction Co., Chicago.



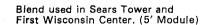
FIRST WISCONSIN CENTER, Milwaukee, Wis. 42 stories, largest in the state. 1.2 million sq. ft. of Inryco 3"V and 3"NFv deck. Owner: First Wisconsin National Bank of Milwaukee. Architects/Engineers: Skidmore, Owings & Merrill, Chicago; Fitzhugh Scott Architects, Inc., Milwaukee. Managing Contractor: Carl A. Morse, Inc., Chicago.

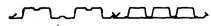


STANDARD OIL BUILDING, Chicago, III. 80 stories, fourth tailest building in the world. 2.5 million sq. ft. of Inryco B and BF deck. Owner: Standard Oil Co. (Indiana), Chicago. Architects: Edward Durell Stone & Associates, New York; Perkins & Will, Chicago Engineers: P. & W. Engineers, Inc. Gen. Contractor: Turner Construction Co., Chicago.



IDS CENTER, Minneapolis, Minn. 57 stories, towers over the Northwest. 1.4 million sq. ft. of Inryco 3"N and 3"NF deck. Owner: Investors Diversified Services, Inc., Minneapolis. Architects: Philip Johnson/ John Burgee, New York: Edward F. Baker Associates, Inc., Minneapolis. Structural Engineer: Severud, Perrone, Sturm, Conlin & Bandel, New York. Gen. Contractor: Turner Construction Co., Minneapolis.





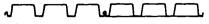
3" V deck (32") - 3" NF, Celluflor (28")

Blend used in Standard Oil Building. (5' Module)



11/2" B deck (36") - 11/2" BF Celluflor (24")

Blend used in IDS Center. (4' Module)



3" N deck (24") - 3" NF Celluflor (24")

## Inryco Pre-set Inserts

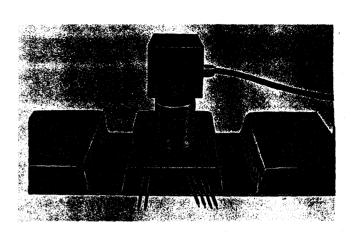
### SERIES 500

SINGLE PRE-SET OR AFTER-SET

Surface mounted fittings

Power or telephone



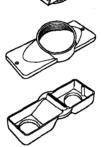


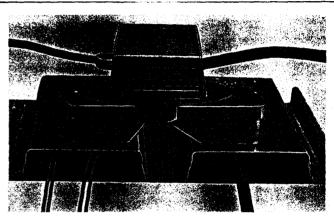
### **SERIES 1000**

**DUAL PRE-SET** 

Surface mounted fittings

Power and/or telephone



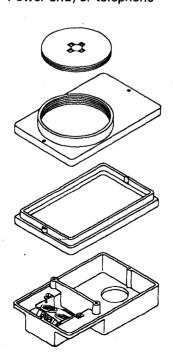


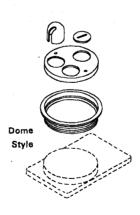
### **SERIES 2000**

**DUAL PRE-SET** 

Flush floor fittings

Power and/or telephone

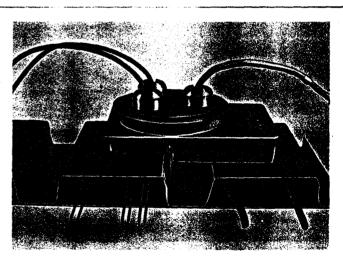


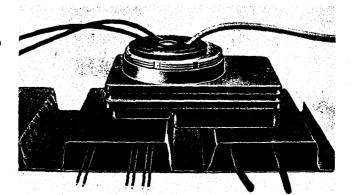














### INRYCO FLOOR SYSTEMS

One of the most costly operations in the electrification of a finished floor is the coring of concrete to reach cells for the installation of outlets. It has been estimated that the placement of a single such "afterset" costs as much as \$40.00 in labor and equipment. Telephone company statistics say that the average office telephone is moved every 10 months — or 1.2 moves per year — and that 4.2 to 6.0 phones are installed per 1,000 sq. ft. in the average usable office space.

If 100 phones were to be provided for, in a space 100 x 200 ft., the cost of original afterset locations would be 100 x \$40.00, or \$4,000.00. In three years, these same phones could be moved 1.2 times per year—or a total of 360 moves. Using aftersets, this would total \$14,400. Added to the cost of the original layout, it would total \$18,400 in 3 years. And this would only accommodate telephone service. The drilling and coring of 100 aftersets for power outlets would duplicate the cost.

If Inryco Pre-set inserts were used, the cost per location would be only about \$6.00 for a dual insert that provides both telephone and power service! Furthermore, for only \$4,500 — which is little more than the original cost of 100 aftersets @ \$4,000 — pre-set dual inserts could be installed on a 5 ft. x 5 ft. module through the entire space — totaling 750 telephone/power outlets in all!

This shows how dramatic savings can be when Inryco Pre-set inserts are designed into the original system.

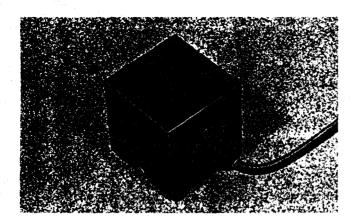
Pre-set inserts are installed on a specified module prior to the pouring of concrete. In a typical installation, only a specified number of these inserts would be considered "active" at the outset, using flush or surface mounted service fittings.

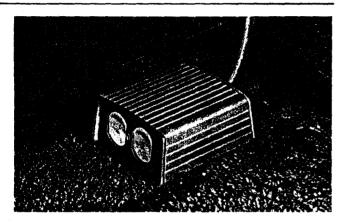
Another quantity of pre-sets might be considered "idle," but likely to be activated in the near future. These would be given brass or bronze cover plates adaptable to either carpet or tile. Similarly, after an outlet is no longer needed at a given location, the service fitting can be removed and an "idle" cover plate installed.

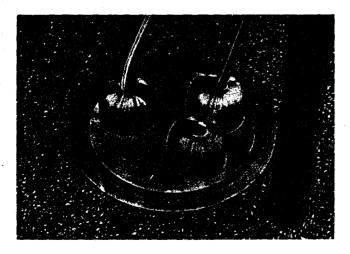
The ease and simplicity with which outlet changes can be made — together with the substantial savings that are possible — make Inryco Pre-set Inserts and service fittings a valuable element in the design of any floor system.

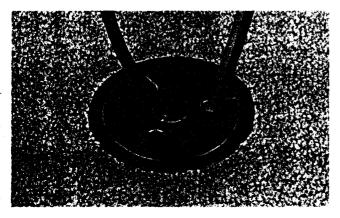
See the suggested specifications on page 27. The electrical contractor should field install the pre-sets and service fittings, retaining his responsibility for the project's electrical and communications integrity. More information, in detail, is available from your Inland-Ryerson Sales Engineer.

Factory-punched holes are available for all Inryco decks. Contact your Inland-Ryerson sales engineer for complete information.









# Floor Deck Selection Charts

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1½" SECTIONS	Deck Type	Deck Depth	Max. Length	Approx. Dead Load†	Concrete Usage Per Sq.‡	Cell Space	Cell Area Sq. In.	Max. Hand Hole	Hanger Tab System	Econom- ical Span
	s	11/2"	32′	41	.95	6″	***	*	Integral	6′-12′
	1 5⁄8″ NFs	15/8″	45′	42	.93	8″	8.93	4"	Integral	6′-12′
~~~~~	В	11/2"	32′	41	.95	6″	*	*	Lip or Integral	6′-12′
	15/8" NF	15/8″	45′	42	.93	8″	8.93	4"	Lip or Integral	6′-12′
	BF	11/2"	32'	43	.95	6″	5.63	3″	Lip or Integral	6′-12′
~~~~~	SR	1½″	32′	45	1.05	6"	*	*	Integral	6′-12′
2" SECTIONS	I	L	L	L		1	i	L	L	<u> </u>
	2″ V	2"	45′	43	1.05	16″	٠	*	Кеу	8′-12′
3" SECTIONS										
	N	3″	45′	45	1.05	8″	*	*	Lip or Integral	8′-14′
	3″ ŅF	3″	45′	47	1.05	8″	17.25	4"	Lip or Integral	8′-14′
	3″ V	3″	45′	51	1.20	16″	*	*	Integral	10′-16′
	3" NF <sub>v</sub>	3″	45′	51	1.20	8″	17.25	4"	Pull-Down	10′-16′

Inland-Ryerson Construction Products Company reserves the right to change the design or details of its products without notice. Specific information for job details and drawings should be obtained from your Inland-Ryerson Sales Engineer.

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<sup>†</sup> Based on 2½" regular weight concrete cover.

Based on 2½" concrete cover — add or deduct .15 cy./sq. for each ½" variance.

<sup>\*</sup> Not cellular — cannot be electrified.

Composite Beam	Coverage Width	Blends With
Yes	30" Standard 24", 36" Avail.	15∕8″ NFs
Yes	24″	s
Yes	30" Standard 24", 36" Avail.	BF, 15∕8″ NF
 Yes	24″	В
Yes	24″	В
Yes	30" Standard 24", 36" Avail.	None
Yes	32" Standard 30", 16" Avail.	None
 No	24″ Standard 17¾6″ Avail.	3″ NF
No	24" Standard 255%" Avail.	N
Yes	32" Standard 30", 16" Avail.	NF <sub>v</sub>
Yes	28" Standard 30" Avail	V

# Typical Blends Possible\*\*

USING CELLUFLOR AND NON-CELLULAR INRYCO FLOOR DECKS

Module	Blend	0′	2′	4' 6'
l L	OS OF 1½" PROFILES:			
4'-0"	24"S/24"15%NFs	~~~		7
4′-0″	24"B/24"15%NF	~~~~		
4′-0″	24"B/24"BF	~~~		
4′-6″	30"S/24"15%NFs	~~~		4
4′-6″	30"B/24"15%NF	~~~~		
4′-6″	30"B/24"BF	~~~	~~~~	
5′-0″	36″S/24″15∕ <sub>8</sub> NFs	~~~~	~~~~~	
5′-0″	36″B/24″15∕ <sub>8</sub> NF	~~~		<b></b>
5′-0″	36"B/24"BF	~~~	-	
6′-0″	24"S/24"S/24"15%NFs	~~~	for	
6′-0″	24"B/24"B/24"15%NF	~~~~	free	
6′-0″	24"B/24"B/24"BF	~~~	w~~~	
BLEN	DS OF 3" PROFILES:			
4′-0″	24"N/24"3NF	<b>√ √ √</b>		
4′-6″	30″V/24″NFv	~~~		
4′-8″	32"V/24"NF <sub>v</sub>	~~~	Tell	
4′-10″	30″V/28″NF <sub>v</sub>	~~~	7~	
5′-0″	17"N/17"N/26"3NF	~~~~		
5′-0″	30"V/30"NF <sub>v</sub>	~~~	Tr.	
5′-0″	32″V/28″NF <sub>v</sub>	~~~	Tell	
6′-0″	24"N/24"N/24"3NF	\~~~	$\psi$	
6′-0″	16"3V/32"3V/24″3NF <sub>v</sub>	2	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	

FOR MORE INFORMATION

Additional data on Inryco Floor Decks can be found on the pages following. However, further detailed information, including composite design and test data, can be obtained at any time from your Inland-Ryerson Sales Engineer. District Sales offices are listed on the back cover of this catalog.

\*\*Many more blends are possible. Contact your Inryco sales engineer.

## Fire Ratings of Inryco Floor Systems

Inryco Hi-Bond Floor Systems are fire-rated by Underwriters' Laboratories, Inc., and Inryco Celluflor is UL approved as cellular metal raceway. Complete information can be found in the 1972 UL Fire Resistance Index. A comparison of ratings in the same book will show that Inland-Ryerson has the most comprehensive and complete classification of fire-rated floor assemblies.

Provided the Celluflor is covered by a minimum of  $1\frac{1}{2}$ " of concrete, both the fluted section and the flat plate may be made of 20 gage steel.

The chart below shows a brief synopsis of some of the tests. For specific information regarding the construction features, the UL Fire Resistance Index should be consulted, since minor changes can significantly affect the performance of a floor assembly under fire exposure.

		FIRE-RATED FLO	OR ASSEMBLIES			
Restrained Assembly Rating	Concrete Cover†	U.L. Fire Index R		inryco Floor Deck Covered		
Assembly Hatting	COVEIT	New	Replaces			
Acoustical Tile	and Panel Material					
3	Reg. 21/2"	D 002	UL 211-3	B, N, BF, BB, NF		
2	Reg. 21/2"	D 001	UL 70-2	B, BF, N, NF		
Cementitious M	lixture Directly Appl	ied				
3	Reg. 21/2"	D 701*	UL 21-3	B, BR, BF, BB		
3	Reg. 21/2"	D 708*	UL 205-3	B, BR, BF, N, NF		
3	Reg. 21/2"	D 724*	UL 70-3	B, BR, BF, N, NR, NF, HF		
2	Reg. 21/2"	D 722*	UL 257-2	B, BF, N, NF, NF <sub>V</sub> , S		
2	Reg. 21/2"	D 704*	UL 248-2	B, BB, BF, N, NF, S		
Sprayed Fiber D	Directly Applied					
3	Reg. 21/2"	D 816*	UL 62-3	B, BF, S, N, NF, HF		
3	Reg. 21/2"	D 831*††	UL 89-3	B, BF, S, N, NF		
2	Reg. 21/2"	D 808*††	UL 271-2	B, BF, S, N, NF		
2	Reg. 21/2"	D 842*	UL 215-2	B, BF, S, N, NF		
2	Lt. wt. 21/2"	D 847*††	None	3V, 3NF <sub>v</sub>		
Unprotected Sta	el Floor					
3	Lt. wt. 43/16"	D 902	UL 225-3	B, BF, N, NF, NF <sub>v</sub> , 2V, 3V		
2	Lt. wt. 31/4"	D 840	UL 267-2	B, BF, S, N, NF, NF <sub>v</sub> , 2V, 3V, SR, BR		
2	Lt. wt. 31/4"	D 906	None	3V, 3NF <sub>Y</sub>		
2	Lt. wt. 31/4"	D 826*	UL 295-2	B, BF, N, NF, NF <sub>v</sub> , 3V, S, SR, BR		
2	Reg. 41/2"	D 902	UL 300-2	B, BF, N, NF, NF <sub>v</sub> , 2V, 3V, S, SR, BF		
11/2	Reg. 4"	D 902	UL 32-11/2	B, BF, N, NF, NF <sub>v</sub> , 2V, 3V, S, SR, BF		
1	Reg. 3"	D 902	UL 59-1	B, BF, N, NF, NF <sub>v</sub> , 2V, 3V, S, SR, BF		

<sup>\*</sup>Indicates header or trench header duct.

Recommended temperature and shrinkage reinforcing, non-continuous spans

### TYPES B, BR, S, SR, BF, 1 1 1 1 NF, 1 1 NFs, 2"V

Slab t	4"	41/2"	43/4"	5″	51/4"	53/4"			
Span			Reinforc	ing					
to 8'3" 8'3" to 10'0" 10'0" to 12'0"	6x6-10/10 6x6-8/8 6x6-6/6	6x6-10/10 6x6-8/8 6x6-6/6	6x6-10/10 6x6-8/8 6x6-6/6	6x6-8/8 6x6-6/6 6x6-6/6	6x6-8/8 6x6-6/6 6x6-6/6	6x6-8/8 6x6-6/6 6x6-6/6			
TYPES N, 3"	NF, 3"NF <sub>y</sub> , 3	3″V				· · · · · · · · · · · · · · · · · · ·			
Slab t	5″	51/2"	6"	61/4"	61/2"	7″			
Span			Reinforc	ing					
to 10'0" 10'0" to 16'0"	6x6-10/10 6x6-8/8	6x6-10/10 6x6-8/8	6x6-10/10 6x6-8/8	6x6-10/10 6x6-8/8	6x6-8/8 6x6-6/6	6x6-8/8 6x6-6/6			

<sup>†</sup>Concrete cover depth is that thickness over top of deck.

<sup>††</sup>These rating are available with Asbestos Free Spray.

## Finishes Available

### HI-BOND CELLUFLOR

Types BF, NF, NFs and NFv Inryco Celluflor are only available in Galvanized Steel, since all UL labeled cellular metal raceways must be galvanized. The top fluted sections are Ti-Co Galvanized Light Commercial Coat (3/4 oz. nominal) and flat plates are Ti-Co Galvanized Paintite A (1/2 oz. nominal) needing no special preparation prior to field painting.

### HI-BOND FLOOR DECK (Non-Cellular)

Inryco Types B, S, N, V and SR Hi-Bond are available either Phosphatized/Painted or Galvanized. Phosphatized/Painted is the standard floor finish — cold-rolled steel, phosphate treated on the upper surface, painted with Duoprimer on the underside. Both paint coats are thermosetting, hence do not become soft and flow when subjected to heat, which permits their use in both fire-proofed and "exposed" UL fire-rated floor assemblies.

Regarding the upper surface in contact with the concrete, while the phosphate/iron alloy serves as a corrosion deterrent, it is expected that during the normal construction phase, some rusting will occur. In the absence of running water or extreme atmospheric conditions, no significant loss of steel section results.

The "rough" surface of Phosphatized/Painted Hi-Bond makes an ideal work platform, even when wet.

It is also superior to other finishes for field welding — both the arc welding of deck to structural steel; and the welding of studs through the decking.

Phosphatized/Painted is the standard finish, red oxide in color, and recommended for the majority of applications; that is, interiors where excessive moisture is of no concern and where the underside of the deck is protected by fire-proofing, paint, or a suspended ceiling, or where the red oxide color is unobjectionable, such as in industrial uses.

Galvanized Hi-Bond is also available — Ti-Co Galvanized Paintite A (½ oz. nominal) needing no special preparation prior to field painting. This finish is recommended for exterior applications, or those interior applications where excessive moisture may be present.

# **Hanger Tabs**

Inryco Lip Hanger Tabs are available for all decks with male/female side joints. The hanger tab is installed during deck erection, hooked over the male leg before the female leg of the next panel is placed over it. Because of its unique design, the hanger tab can be seen and moved from above after erection before concrete is poured. The tab protrudes from the underside and there is no chance of it being covered up by a later application of fire-proofing. Allowable load per tab: 100 pounds.

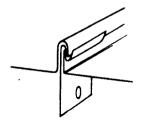
**Inryco Integral Hanger Tabs** are also available, prepunched in the factory and ready for use at the job with no extra field labor or materials needed.

Integral Tabs are available in 18 and 16 gage steel sections with Deck Types B, S, SR, N, V, BF, 15/8" and 3" NF. The limiting load on any hanger is 100 lbs., with a minimum size of #8 hanger wire. (Care must be taken in the activation of the hanger loop, and under no circumstances should a tab be used

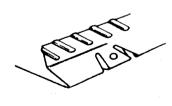
which has been damaged. Concrete drippage may occur through the tab and concrete will have to be cleared from the loop when activating the tabs on the fluted sections.) For fluted decks with Integral Hanger Tabs, Non-Composite Section Properties should be reduced by 10%.

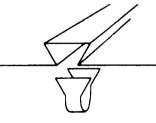
Inryco Type NF<sub>v</sub> Pull-Down Hanger Tabs are formed into the bottom lip of Type NF<sub>v</sub> Celluflor only. This tab, in effect, becomes a "pig-tail" embedded in the concrete, and only those tabs to be used need be activated. Allowable load per tab: 250 lbs.

Inryco Key-type Hanger Tabs fit into trapezoidal shaped ribs of 2" Type V Floor Deck only. Key tabs can be inserted at any longitudinal spacing, and at any time during the life of the building. When inserted during construction, key tabs will not cause concrete spillage. Available in gages from 20 to 16. Allowable load per tab: 150 lbs.



(Accessory)



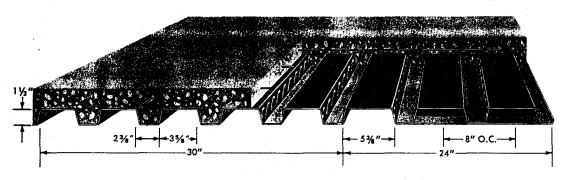


Type NFv Pull-Down Tab

integral Tab

"Key" Hanger Tab

# Types S and 15/8"NFs



This new NFs deck has an "S" type lock that makes it compatible with 11/2" Type S noncellular deck. A blend of these two provides 2 large cells for electrical services on many modules.

**ALLOWABLE SUPERIMPOSED LOADS** 

Type S Hi-Bond Floor Deck with Regular Concrete

/	REGULAR Slab(in.) 4 Mr(ft. lbs.) 1600 Vr(lbs.) 600 Wt.(pfs.) 41  CONDITION S D T					REG Slab(in.) Mr(ft. lb Vr(lbs.) Wt.(psf.)	s.)	5 2000 800 53		REGULAR Slab(in.) 6 Mr(ft. lbs.) 2400 Vr(lbs.) 1000 Wt.(psf.) 65				
CONDI	TION	S	D	T	CONDI	TION	S	D	Т	CONDI	LION _	S	D	T
GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD	
22	6-0 6-6	200 —	200 185	200 185	22	6-0 6-6	_	200 —	200 200	22	6-0	_	_	200
	7-0 6-6	185 171	185 171	171	20	6-0 6-6	200 200	200 200 200	200 200 200	20	6-0 6-6	200	200 200	200 200
20	7-0 7-6 8-0		160	171 160 150		7-0 7-6		_	200	18	6-6 7-0	200 200	200	200 200
	7-6 8-0	160 150	160 150	160 150	18	7-0 7-6 8-0	200 200	200 200 200	200 200 200		7-6 8-0	_	200	200 200
18	8-6 9-0 9-6	141	141	141 133	· · · · · · · · · · · · · · · · · · ·	8-6 8-0	200	200	188		7-6 8-0 8-6	200 200	200 200 200	200 200 200
16	9-0 9-6 10-0 10-6	133 126	133 126 120	126 133 126 120 114	16	8-6 9-0 9-6 —	188	188 178 — —	188 178 168 —	16	9-0			200 — — —

	LIGHT Slab (in.) Mr (ft. lb Vr (lbs.) Wt. (psf.	) is.)	4 1600 600 30		,	LIGHT Slab(in.) Mr(ft. lb: Vr(lbs.) Wt.(psf.	s.)	HT 4 <sup>3</sup> ⁄ <sub>4</sub> 1900 750 36		LIGHTWEIGHT Slab(in.) 53/4 Mr(ft. lbs.) 2300 Vr(lbs.) 950 Wt.(psf.) 44				
COND	ITION	S	D	Т	CONDI	TION	s	D	Т	CONDI	TION	S	D	Т
GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD	-	GAGE	SPAN (ftin.)		LOAD	
22	6-0 6-6 7-0	200 185 171	200 185 171	200 185 171	22	6-0 6-6 7-0	200 200 —	200 200 200	200 200 200	22	6-0 6-6 7-0	200 — —	200 200 —	200 200 200
	7-6 8-0	=	160	160 150		7-6 7-0	200	200	200	20	6-6 7-0	200 200	200 200	200 200
20	7-6 8-0 8-6	160	160 150 141	160 150 141	20	7-6 8-0 8-6	200	200 187	200 187 176	20	7-6 8-0	_	200	200 200
	9-0			133		8-0	187	187	187	18	7-6 8-0	200 200	200	200 200
40	8-6 9-0	141 133	141	133	18	8-6 9-0	176 —	176 167	176 167	,,,	8-6 9-0	_	200 200	200
18	9-6 10-0 10-6		126 120	126 120 114		9-6 10-0		158	158 150		8-6 9-0	200 200	200 200	200 200
<u></u>	9-6 10-0 10-6	126 120 114	126 120 114	126 120 114	16	9-0 9-6 10-0	167 158 —	167 158 150	167 158 150 138	16	9-6 10-0 10-6	=	200 184 —	200 184 167
16	11-0	114	106	106	10	10-6 11-0	=	138	126		_		_	_
	11-6	-	_	97		_	-	_	-		_		-	_

Type S Hi-Bond Floor Deck with Lightweight Concrete

REGULAR

LIGHTWEIGHT

### SECTION PROPERTIES

Type S Floor Deck

Gage	Weight		S(+)	S(-)	S <sub>β</sub>	Area	Y
	(psf.)	(in.)4	(in.) <sup>3</sup>	(in.)3	(in.) <sup>3</sup>	(in.)²	(in.)
22	1.80	0.16	0.19	0.20	0.20	0.51	0.91
20	2.15	0.21	0.25	0.25	0.25	0.61	0.91
18	2.85	0.31	0.34	0.35	0.34	0.82	0.92
16	3.55	0.40	0.44	0.44	0.44	1.01	0.92

REGULAR

12-0

11-6

12-0

16-18

97

89

97

89

97

89

LIGHTWEIGHT

89

89

Type 1 5%" NFs Celluflor

REGULAR

Gage	Weight		S(+)	S(-)	.S <sub>8</sub>	Area	Y
	(psf.)	(in.)4	(in.)3	(in.)3	(in.) <sup>3</sup>	(in.)²	(in.)
20-20	3.00	0.35	0.26	0.36	0.63	0.90	0.72
18-20	3.60	0.47	0.40	0.44	0.72	1.08	0.80
18-18	4.35	0.53	0.41	0.55	0.86	1.20	0.74
16-18	5.00	0.66	0.57	0.62	0.95	1.38	0.80

**ALLOWABLE** SUPERIMPOSED LOADS

Type 15/8"NFs Hi-Bond Celluflor with

Regular Concrete

Slab(in.) Slab(in.) Slab(in.) Mr(ft. lbs.) 1600 Mr(ft. lbs.) 2000 Mr(ft. lbs.) 2400 Vr(lbs.) 700 Vr(lbs.) 900 1100 Vr(lbs.) Wt.(psf.) 38 Wt.(psf.) 50 Wt.(psf.) 62 CONDITION s D Т CONDITION D Т CONDITION s D Т SPAN (ft.-in.) SPAN (ft.-in.) SPAN (ft.-in.) GAGE LOAD LOAD GAGE LOAD GAGE 200 200 200 200 200 6-0 200 200 200 6-0 200 6-0 6-6 200 200 200 6-6 200 200 200 20-20 6-6 200 200 20-20 200 200 200 200 7-0 200 200 7-0 7-0 20-20 7-6 200 7-6 187 187 200 6-6 200 200 200 8-0 175 175 200 7-0 200 200 200 7-0 200 200 8-6 165 1165 200 200 200 200 200 7-6 7-6 200 200 18-20 200 187 200 7-6 187 8-0 200 187 8-0 18-20 8-0 175 175 175 8-6 200 200 8-6 200 200 8-6 165 165 165 9-0 198 198 9-0 200 18-20 9-6 177 9-0 156 156 156 200 200 7-6 200 9-6 142 142 8-6 200 200 200 8-0 200 200 10-0 128 128 18-18 198 9-0 198 200 200 8-6 10-6 116 18-18 9-6 177 200 200 177 9-0 156 9-0 156 156 10-0 160 160 9-0 200 200 200 9-6 142 142 18-18 9-0 198 198 198 9-6 200 200 10-0 128 128 9-6 177 177 177 10-0 192 192 10-6 116 116 10-0 160 160 160 10-6 174 16-18 145 10-6 116 116 116 16-18 10-6 145 11-0 106 106 106 11-0 132 132 16-18 11-6 97 97 11-6 121

Slab(in.) Slab(in.) 43/4 Slab(in.) 53/4 2300 1600 Mr(ft. lbs.) 1900 Mr(ft. lbs.) Mr(ft. lbs.) 700 Vr(lbs.) 850 Vr(lbs.) 1050 Vr(lbs.) Wt.(psf.) 37 Wt.(psf.) 45 Wt.(psf.) 31 CONDITION D Τ CONDITION D T CONDITION S D Т SPAN (ft.-in.) SPAN (ft.-in.) SPAN (ft.-in.) GAGE LOAD GAGE LOAD GAGE LOAD 200 200 200 200 200 200 200 200 6-0 200 6-0 200 200 6-0 200 200 200 200 6-6 200 200 6-6 200 6-6 200 200 200 7-0 200 200 7-0 200 20-20 7-0 200 200 200 200 7-6 187 187 187 20-20 7-6 200 200 200 7-6 200 200 20-20 200 200 8-0 8-0 8-0 175 175 175 8-6 165 165 8-6 200 200 7-6 200 200 200 9-0 9-0 188 188 156 156 8-0 200 200 200 9-6 142 142 200 200 200 200 200 200 8-0 8-6 18-20 8-6 200 200 200 9-0 200 200 8-6 165 165 165 200 9-0 9-0 188 188 188 9-6 200 156 156 156 18-20 9-6 9-6 142 142 142 168 168 168 10-0 184 18-20 10-0 128 128 128 10-0 152 152 9-0 200 200 200 10-6 116 116 10-6 138 138 200 200 9-6 11-0 106 106 11-0 126 18-18 10-0 184 97 184 11-6 9-6 168 168 168 10-6 167 167 10-0 128 128 128 10-0 152 152 18-18 10-6 167 167 167 10-6 116 116 10-6 138 138 152 11-0 152 18-18 11-0 126 106 106 11-0 126 11-6 139 139 11-6 97 97 11-0 126 126 126 16-18 128 12-0 12-0 89 89 11-6 115 115 16-18

12-0

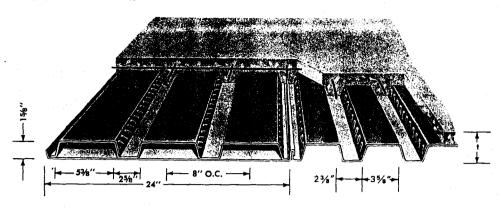
106

106

LIGHTWEIGHT

Type 1%" NFs Hi-Bond Celluflor with Lightweight Concrete

# Types B, BF and 15/8"NF



LIGHTWEIGHT

Available with the male-female side lap system, Types B, BF and 15%" NF can be blended for economical flexibility in the matching of cell patterns to various modules. Type 15%" NF offers 66% greater cell capacity and permits 4" hand holes in top surface.

LIGHTWEIGHT

ALLOWABLE SUPERIMPOSED LOADS

Type B
Hi-Bond Floor Deck with
Regular Concrete

<i>\</i>	REGULAR Slab(in.) 4 Mr(ft. lbs.) 1600 Vr(lbs.) 700 Wt.(psf.) 41					REGULAR Slab(in.) 5 Mr(ft. lbs.) 2000 Vr(lbs.) 900 Wt.(psf.) 53					REGULAR Slab(in.) 6 Mr(ft. lbs.) 2400 Vr(lbs.) 1100 Wt.(psf.) 65				
CONDI	rion	S	D	T	CONDI	CONDITION		S D T		CONDITION		S	D	Т	
GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD		
22	6-0 6-6 7-0	200 200	200 200 200	200 200 200	22	6-0 6-6	200	200 200	200 200	22	6-0	_	200	200	
-	7-6	200	200	185	20	6-6 7-0	200	200 200	200	20	6-0 6-6 7-0	200 —	200 200	200	
20	7-0 7-6 8-0 8-6	187	187 175	187 175 165	18	7-6 7-0 7-6 8-0	200 200 200	200 200 200	200 200 200 200	18	6-6 7-0 7-6	200 200 200	200 200 200	200 200 200 200	
18	8-0 8-6 9-0	175 165 156	175 165 156	175 165 156		8-6 9-0	_	200 198	200 198		8-0 8-6	_ _	200	200 200	
10	9-6 10-0	_	142	142 128		8-6 9-0 9-6	200 198	200 198 177	200 198 177		8-0 8-6 9-0	200	200 200 200	200 200 200	
16	9-6 10-0 10-6 11-0 11-6	142 — — — —	142 128 116 106	142 128 116 106 97	16	10-0 10-6 — — —		160	160 145 — —	16	9-6 — — —			200 — — — —	

N V	lab(in.) Ar(ft. lbs /r(lbs.) Vt.(psf.)	•	4 1600 700 30		N V	lab(in.) Ar(ft. lbs /r(lbs.) Vt.(psf.)	s.) 1	43/ <sub>4</sub> 1900 850 36		Slab(in.) 53/4 Mr(ft. lbs.) 2300 Vr(lbs.) 1050 Wt.(psf.) 44				
CONDIT	TION	S	D	Т	CONDI	rion	S	D	T.	CONDI	TION	S	D	T
GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)	LOAD			GAGE SPAN (ftin.)		LOAD		
22	6-0 6-6 7-0 7-6	200 200 200	200 200 200 187	200 200 200 187	22	6-0 6-6 7-0 7-6	200 200 200	200 200 200	200 200 200 200	22	6-0 6-6 7-0	200 200 —	200 200 —	200 200 200
	8-0 7-6	187	187	175 187	20	7-6 8-0	200	200 200	200 200 200	20	7-0 7-6 8-0	200	200 200	200 200 200
20	8-0 8-6 9-0	175 — —	175 165	175 165 156		8-6 8-0 8-6	200 200	200 200	200 200 200		7-6 8-0 8-6	200 200 200	200 200 200	200 200 200
18	8-6 9-0 9-6 10-0	165 156 142 128	165 156 142 128	165 156 142 128	18	9-0 9-6 10-0 10-6	188	188 168 152	188 168 152 138	18	9-0 9-6 10-0		200 200 —	200 200 184
	10-6 11-0	  -	116	116 106		9-6 10-0	168 152	168 152	168 152		9-0 9-6 10-0	200	200 200 184	200 200 184
16	10-6 11-0 11-6 12-0	116 — —	116 106 97 89	116 106 97 89	16	10-6 11-0 11-6 12-0	-   -   -	138 126 115	138 126 115 106	16	10-6 11-0	 	167 — —	167 152 —

LIGHTWEIGHT

Type B
Hi-Bond Floor Deck with
Lightweight Concrete

### SECTION PROPERTIES Types B and BF

Gage	Weight (psf.)	 (in.)4	S(+) (in.)3	S(-) (in.)3	S <sub>B</sub> (in.)3	Area (in.) <sup>2</sup>	Y (in.)
TYPE B						-	
22 20 18 16	1.95 2.35 3.35 4.15	0.18 0.24 0.34 0.44	0.21 0.27 0.40 0.50	0.21 0.27 0.40 0.50	0.21 0.27 0.40 0.50	0.55 0.65 0.86 1.07	0.87 0.87 0.88 0.88
TYPE B	F		·	<b></b>	1		<u> </u>
20-20 18-20 18-18 16-18	3.80 4.40 4.95 5.80	0.38 0.49 0.55 0.67	0.31 0.46 0.48 0.65	0.43 0.53 0.58 0.71	0.75 0.85 1.01 1.17	1.07 1.26 1.42 1.62	0.57 0.64 0.58 0.64

### INRYCO FLOOR SYSTEMS

LIGHTWEIGHT

### SECTION PROPERTIES Type 1 5/8" NF

Gage	Weight (psf.)	 	S(+) (in.)3	S(-) (in.)3	S <sub>8</sub> (in.) <sup>3</sup>	Area (in.)²	Y (in.)
20-20	3.60	0.38	0.27	0.37	0.75	1.04	0.66
18-20	4.25	0.50	0.41	0.45	0.85	1.22	0.74
18-18	4.80	0.57	0.43	0.57	1.02	1.38	0.67
16-18	5.70	0.70	0.59	0.64	1.12	1.56	0.73

### ALLOWABLE SUPERIMPOSED LOADS

### Types 1%"NF and BF Hi-Bond Celluflor with Regular Concrete

	REG	ULAF	·		T	REG	ULAF				REG	ULAR		
[	Slab(in.) Vir(ft. lb: Vr(lbs.) Wt.(psf.)	s.)	4 1600 700 42		\ \	Slab(in.) Ar.(ft. lb /r(lbs.) Vt.(psf.)	s.) :	5 2000 900 54		, V	llab(in.) Ar(ft. lbs /r(lbs.) Vt.(psf.)	s.)	6 2400 1100 66	
COND	TION	S	D	Т	CONDI	TION	S	D	Т	CONDI	rion	S	D	T
GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD	
20-20	6-0 6-6 7-0 7-6 8-0	200 200 200 187	200 200 200 187 175	200 200 200 187 175	20-20	6-0 6-6 7-0 7-6 8-0	200 200 — —	200 200 200 200 200 200	200 200 200 200 200 200	20-20	6-0 6-6 7-0 7-6	200 — — —	200 200 200 200	200 200 200 200
18-20	8-6 8-6 9-0 9-6	175 165 156	165 175 165 156 142	165 175 165 156 142	18-20	7-0 7-6 8-0 8-6 9-0	200 200 200 —	200 200 200 200 200 198	200 200 200 200 200 198	18-20	6-6 7-0 7-6 8-0 8-6 9-0	200 200 200 — —	200 200 200 200 200	200 200 200 200 200 200
	10-0 10-6	_	128	128 116		9-6 8-6	200	177 200	177 200	18-18	7-6 8-0	200	200 200	200 200
18-18	9-0 9-6 10-0	156 — —	156 142 128	156 142 128	18-18	9-0 9-6 10-0	=	198 177 160	198 177 160	10-10	8-6 9-0 8-0	200	200 200 200	200 200 200
16-18	9-6 10-0 10-6 11-0 11-6 12-0	142 128 116 106	116 142 128 116 106 97 89	116 142 128 116 106 97 89	16-18	9-0 9-6 10-0 10-6 11-0 11-6	198 177 160 — —	198 177 160 145 132	198 177 160 145 132 121	16-18	8-6 9-0 9-6 10-0 10-6 11-0	200 200 — — — —	200 200 200 192 174	200 200 200 192 174 159

	N V	ilab(in.) Ar(ft. ibs Ir(lbs.) Vt.(psf.)	s.)	4 1600 700 31		N N	Slab(in.) Mr.(ft. ib Mr(lbs.) Mt.(psf.)	s.) :	43/4 2000 900 37		\ \	Slab(in.) Ar(ft. lb: /r(lbs.) Vt.(psf.)	s.) 2	5 <sup>3</sup> ⁄ <sub>4</sub> 2300 1050 45	
	CONDI	TION	S	D	Т	CONDI	TION	S	D	T	CONDI	TION	S	D	T
•	GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD	
•	20-20	6-0 6-6 7-0 7-6 8-0 8-6 9-0	200 200 200 187 175	200 200 200 187 175 165 156	200 200 200 187 175 165 156	20-20	6-0 6-6 7-0 7-6 8-0 8-6 9-0	200 200 200 200 — —	200 200 200 200 200 200 200 188	200 200 200 200 200 200 200 188	20-20	8-0 6-6 7-0 7-6 8-0 8-6	200 200 200 — —	200 200 200 200 200 200 200	200 200 200 200 200 200 200
		9-6	_	142	142		8-0	200	200	200		7-6 8-0	200 200	200 200	200 200
	18-20	8-6 9-0 9-6 10-0 10-6 11-0	165 156 142 128	165 156 142 128 116 106	165 156 142 128 116 106	18-20	8-6 9-0 9-6 10-0 10-6 11-0	200 188 168 —	200 188 168 152 138	200 188 168 152 138 126	18-20	8-6 9-0 9-6 10-0 10-6	200 200 — — —	200 200 200 184	200 200 200 184 167
		11-6	128	97	97	40.40	9-6 10-0	168	168 152	168 152	18-18	9-0 9-6 10-0	200	200 200 184	200 200 184
	18-18	10-6 11-0 11-6	_	116 106 97	116 106 97	18-18	10-6 11-0	_	138 126	138 126		10-6 9-6	200	167 200	167 200
		12-0	_	89	89		10-0 10-6	152 138	152 138	152 138	10.10	10-0 10-6	184 167	184 167	184 167
	16-18	10-6 11-0 11-6 12-0	116 106 97 89	116 106 97 89	116 106 97 89	16-18	11-0 11-6 12-0	126 115 —	126 115 106	126 115 106	16-18	11-0 11-6 12-0		152 139 128	152 139 128

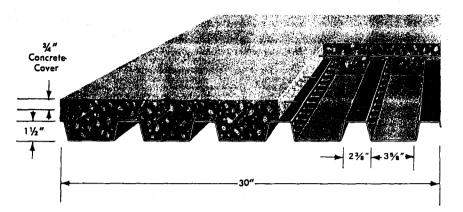
LIGHTWEIGHT

LIGHTWEIGHT

# Types 15/8" NF and BF

Hi-Bond Celluflor with Lightweight Concrete

# Type SR



Recommended for continuous slab design\* in industrial type floor construction, whether the Hi-Bond is shored during concrete pouring or not. When used on industrial type floors, loading conditions should be thoroughly investigated. Hi-Bond slabs are basically one way reinforced slabs, designed for uniformly

### TOTAL SUPERIMPOSED LOADS FOR CONTINUOUS SPANS

Type SR with Regular Concrete

Cor	crete	R	EGULA	R	Cor	crete	R	EGULA	R j	Con	crete	R	EGULA	R
SI	ab t		4"		SI	ab t		5″		SI	ab t		6″	
Tem	p. Bars	#3	@ 13"	o.c.	Tem	p. Bars	#3	@ 12" (	o.c.	Tem	o. Bars	#3	@ 11" (	o.c.
Тор	Bars	#3 @ 12"	#4 @ 12"	#4 @ 8"	Тор	Bars	#3 @ 12"	#4 @ 12"	#4 @ 8″	Тор	Bars	#3 @ 12"	#4 @ 12"	#4 @ 8
Vr Mr	ft. lbs.) (lbs.) (ft. lbs.) (psf.)	1600 800 500 45	1600 800 880 45	1600 800 1300 45	Vr -Mr(	ft. lbs.) (lbs.) ft. lbs.) (psf.)	2000 1000 680 57	2000 1000 1200 57	2000 1000 1760 57	Vr( -Mr(	ft. lbs.) lbs.) ft. lbs.) (psf.)	2400 1200 890 69	2400 1200 1530 69	240 120 231 6
GAGE	SPAN	L	OAD — ps	sf	GAGE	SPAN	L	OAD — p	sf	GAGE	SPAN	L	DAD — ps	sf
io Sh	oring:				No S	horing:				No S	horing:			
22	6-0 6-6	166 142	250 238	250 238	22	6-0	226	250	250	20	6-0 6-6	250 250	250 250	25 25
20	7-0 7-6	122 107	216 188	228 212	20	6-6 7-0	193 166	250 250	250 250	18	7-0 7-6	218 190	250 250	25 25
18	8-0 8-6 9-0	94 83 74	166 146 130	200 188 177	18	7-6 8-0 8-6	145 127 113	250 225 200	250 250 236	16	8-0 8-6	167 148 —	250 250 —	25 25
16	9-6 10-0	66 60	117	168 156	16	9-0 9-6 —	100 90	178 160	222 210	10	_ 	=		-
horir	ng at Mi	dspan:	1		Shor	ing at M	/ //idspan	:		Shori	ng at M	/ //idspan	:	·
22	7-0 7-6 8-0 8-6 9-0 9-6 10-0 10-6 11-0 11-6	77 62 49 — — — — —	145 133 122 101 85 72 60	145 133 122 112 103 95 88 73 63 53	22	6-6 7-0 7-6 8-0 8-6 9-0 9-6 10-0 11-6 11-0	136 109 88 70 56 43 —	199 181 165 151 139 121 103 87 52 42	199 181 165 151 139 128 119 110 102 88 78 65	22	6-0 6-6 7-0 7-6 8-0 8-6 9-0 9-6 10-0 10-6 11-0	229 184 149 121 98 79 63 49	250 239 216 189 181 167 153 134 115 71 55	25 23 21 18 18 16 15 14 13
						12-6 13-0		=	55 47	20	11-6 12-0 12-6	=	47 	10 9 7
		_ _ _			20	13-6 — — —	_ _ _		40 - - -	18	13-0 13-6 14-0 14-6			6 5 4 4

distributed loads. When they are subjected to concentrated loads; such as those caused by wheel-load of fork-lift trucks, additional transverse reinforcing is required.

### SECTION PROPERTIES

Type SR

Gage	Weight (psf.)	[ (in.)4	S(+) (in.) <sup>3</sup>	S(-) (in.) <sup>3</sup>	S <sub>B</sub> (in.) <sup>3</sup>	Area (in.)2	Y (in.)
22	1.80	0.18	0.20	0.19	0.32	0.51	0.62
20	2.15	0.23	0.26	0.25	0.39	0.61	0.63
18	2.85	0.32	0.35	0.34	0.51	0.81	0.63
16	3.55	0.41	0.44	0.44	0.63	1.01	0.64

TOTAL SUPERIMPOSED LOADS FOR CONTINUOUS SPANS

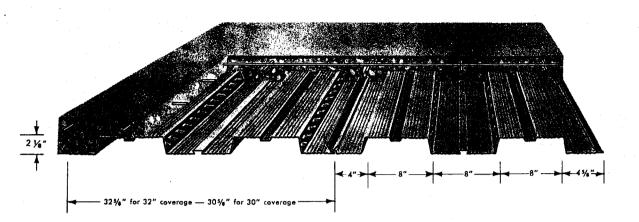
Type **SR** with Lightweight concrete

Con	crete	LIGI	HTWEIG	НТ	Con	crete	LIG	HTWEIG	SHT	Con	crete	LIG	HTWEIG	HT
Sla	ab t		4"		SI	ab t		43/4"		SI	ab t		53/4"	
Temp	. Bars	#3	@ 13" (	o.c.	Tem	o. Bars	#3	@ 12"	o.c.	Temp	o. Bars	#3	@ 11"	o.c.
Тор	Bars	#3 @ 12"	#4 @ 12"	#4 @ 8"	Тор	Bars	#3 @ 12"	#4 @ 12"	#4 @ 8"	Тор	Bars	#3 @ 12"	#4 @ 12"	#4 @ 8
Vr(	ft. lbs.) lbs.) ft. lbs.) (psf.)	1600 800 500 33	1600 800 880 33	1600 800 1300 33	Vr -Mr(	ft. lbs.) lbs.) ft. lbs.) (psf.)	1900 950 630 39	1900 950 1120 39	1900 950 1650 39	Vr( -Mr(	ft. lbs.) lbs.) ft. lbs.) (psf.)	2300 1150 805 47	2300 1150 1430 47	230 115 210 4
SAGE	SPAN	LC	DAD — ps	ıf	GAGE	SPAN	L	OAD — p	sf	GAGE	SPAN	L	DAD — p	sf
lo Sh	oring:				No S	horing:				No Si	horing:			
22	6-0 6-6	166 142	250 246	250 246	22	6-0 6-6	210 179	250 250	250 250	22	6-0 6-6	250 229	250 250	25 25
	7-0 7-6	122 107	215 188	229 213	20	7-0 7-6	154	250	250 250	20	7-0 7-6	197 172	250 250	25 25
20	8-0 8-6	94 83	165 146	188		8-0 8-6	118	210 186	238	18	8-0 8-6	151 134	250 238	25 25
18	9-0 9-6 10-0	74 66 60	130 117 105	178 168 156	18	9-0 9-6	93 84	166 149	211		9-0 9-6	119	190	25
16	10-6 11-0 11-6	45 41 38	80 73 66	118 108 98	16	10-0 10-6 —	76 57 —	134 101 —	190 150 —	16	10-0 — — —	97	172 — —	23
horin	g at Mi	idspan:	<u> </u>		Shori	ng at M	idspan	:		Shori	ng at M	idspan	:	-
22	8-0 8-6 9-0 9-6 10-0 11-6 11-0 12-0 12-6 13-0	61 50 43 — — — — — —	132 113 97 84 72 47 40 —	132 123 115 107 100 85 75 65 57 50 44	22	7-6 8-0 8-6 9-0 9-6 10-0 11-6 11-0 11-6 12-0 13-6	95 79 66 54 45 — — —	173 159 147 127 110 95 62 53 46	173 159 147 137 127 119 111 97 86 76 67 59	22	7-0 7-6 8-0 8-6 9-0 9-6 10-0 11-6 11-0 11-6 12-0	150 125 104 87 72 60 50 —	227 209 193 178 165 143 125 83 71 61 52	22 20 19 17 16 15 14 13 12 11 9
·					20	14-0	-		45	20	13-0 13-6 14-0 14-6	=	_	7 6 6 5
	-	-	=	-		-	-	-	-	18	15-0	<del>                                     </del>	<u> </u>	4

<sup>\*</sup>Negative reinforcing bars 3/4" below the top of the slab over supports.

# New! Type 2"V





The unique profile of new 2"V Deck creates ribs that have the most effective width-to-depth ratio available in floor decks at the present time.

This means higher stud values, and also makes 2"V the most economical section for the 8'-12' span range.

Ripple-like stiffening ribs in both top and bottom flanges give 2"V an added strength and rigidity on the job, plus a skid-resistant surface that helps provide a safe work platform for all trades. An extensive test program has proven that the stiffening ribs in 2"V do not adversely affect the placement or strength of headed shear connectors.

2"V Deck also has Inland-Ryerson's V-lock joint which acts as an integral shear connector, and a new trapezoidal rib into which a key hanger tab can be inserted during construction without causing spillage of concrete, or after construction at any time.

2"V Deck is available in lengths up to 45'-0".



## ALLOWABLE SUPERIMPOSED LOADS — TYPE 2"V Regular concrete slabs.

_	ai coi		, Slan	<b>3.</b>	- : -	_										_			
30″ (	Coverag	10			30″	Coverag	ė			32″	Coveraç	je			32″ (	Coverag	e		
	RE	GULA	R			RE	GULA	R			RE	EGULA	R			RE	GULA	R	
	Slab (ir Vr (lbs. Mr (ft. Wt. (ps	lbs.)	4½ 1150 3000 42			Slab(ir Vr(lbs. Mr(ft. Wt.(ps	) lbs.)	5 1250 3200 48			Slab(ir Vr(lbs. Mr(ft. Wt.(ps	.) lbs.)	4½ 1200 3200 43			Slab(ir Vr(lbs. Mr(ft. Wt.(ps	) lbs.)	5 1300 3400 49	
CONE	NOITIC	S	D	Т	CONE	OITION	S	D	T	CONE	NOITION	S	D	Т	COND	NOITION	S	D	Т
GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD	-1	GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD	
20	8-0 8-6 9-0	170	195 165	195 165 135	20	8-0 8-6	_	215 170	215 170	20	8-0 8-6 9-0	176	208 174	208 174 146	20	8-0 8-6	<del>-</del>	226 187	226 187
18	8-6 9-0 9-6 10-0 10-6 11-0 11-6	225 200 165 140	245 220 195 165 130	245 220 195 165 130 125 105	18	8-0 8-6 9-0 9-6 10-0 10-6 11-0	250 250 210 180 — —	250 250 245 200 175 150	250 250 245 200 175 150 130	18	8-6 9-0 9-6 10-0 10-6 11-0 11-6	243 207 176 149	250 233 202 175 152	250 233 202 175 152 133 115	18	8-0 8-6 9-0 9-6 10-0 10-6 11-0	250 250 223 187 —	250 250 250 222 191 165	250 250 250 222 191 165 142
. 16	10-6 11-0 11-6 12-0 12-6 13-0	175 155 130 —	195 170 150 135 120	195 170 150 135 120 105	16	10-0 10-6 11-0 11-6 12-0 12-6	225 190 160 — — —	240 210 185 165 145	240 210 185 165 145 120	16	10-6 11-0 11-6 12-0 12-6 13-0	187 162 141 —	210 186 165 146 130	210 186 165 146 130 115	16	10-0 10-6 11-0 11-6 12-0 12-6	234 202 172 — — —	250 233 205 180 159 —	250 233 205 180 159 140

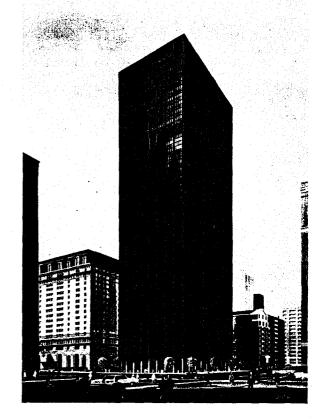




30 NORTH LASALLE Chicago, Illinois Owner: 30 North LaSalle Partnership & Tishman Construction Company, Chicago.

Architects-Engineers:
Thomas E. Stanley, Chicago,
Dallas and New York.
Agent for Construction:
Tishman Construction
Company, Chicago.

42 stories — 1 million square feet of 2"V Inryco Hi-Bond Floor Deck.



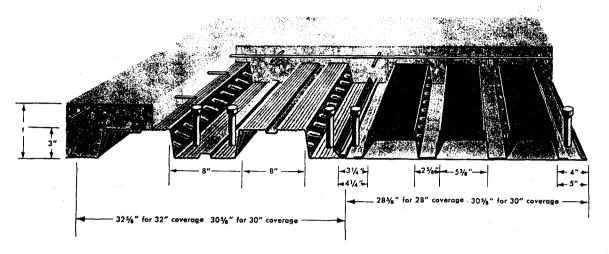
### SECTION PROPERTIES - Type 2"V

Gage	Weight (psf.)	(in.)4	S(+) (in.)3	S(-) (in.) <sup>3</sup>	S <sub>8</sub> (in.)3	Area (in.)2	Y (in.)
30" CO	VERAGE		4				•
20 18 16	2.00 2.60 3.20	0.42 0.62 0.79	0.33 0.52 0.69	0.35 0.50 0.67	0.40 0.55 0.69	0.59 0.79 0.98	1.12 1.13 1.14
32" CO	VERAGE	<b>.</b>			<del></del>	<b></b>	•
20 18 16	1.95 2.55 3.15	0.42 0.61 0.79	0.31 0.50 0.68	0.34 0.50 0.68	0.42 0.57 0.72	0.58 0.77 0.97	1.07 1.08 1.08

### 

30"	Coverag	je			30" (	Coverag	je			32"	Coverag	je			32″ (	Coverag	je		
	LIGH	TWE	GHT			LIGH	TWEI	3HT			LIGH	TWEI	3HT			LIGH	TWEI	ЗНТ	
	Slab (in Vr (lbs. Mr (ft. Wt. (ps	) lbs.)	4½ 1150 3000 32			Slab (in Vr (lbs. Mr (ft. Wt. (ps	) lbs.)	51/4 1325 3350 40			Slab (in Vr (lbs. Mr (ft. Wt. (ps	) lbs.)	4½ 1200 3200 33			Slab(in Vr(lbs. Mr(ft Wt.(ps	) lbs.)	51/4 1375 3550 41	
COND	NOITION	S	D	T	COND	ITION	S	D	Т	CONE	ITION	S	D	T	COND	ITION	S	D	Т
GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD	
20	8-0 8-6 9-0 9-6	195 155 —	210 180 150 125	210 180 150 125	20	8-0 8-6 9-0	215 — —	240 210 170	240 210 170	20	8-0 8-6 9-0 9-6	200 167 —	220 191 163 140	220 191 163 140	20	8-0 8-6 9-0 9-6	227 — —	250 223 188	250 223 188 158
	10-0			110		8-6 9-0	250 240	250 240	250 240		10-0	_		120		8-6	250	250	250
18	9-0 9-6 10-0 10-6 11-0	210 180 155 135 115	225 200 175 150	225 200 175 150	18	9-6 10-0 10-6 11-0	205 180 —	220 210 170	220 210 170 150	18	9-0 9-6 10-0 10-6 11-0	228 195 169 146	229 214 185 166 144	229 214 185 166	18	9-0 9-6 10-0 10-6	250 226 192 —	250 250 224 195	250 250 224 195
· •	11-6 12-0	-  -	130 110	130 110 95		10-6 11-0	230 195	250 210	250 210		11-6 11-6 12-0		129	144 129 111		11-0 11-6		_	170 149
16	11-0 11-6 12-0 12-6 13-0 13-6 14-0	162 143 124 111	162 143 124 111 98 88	162 143 124 111 98 88 78	16	11-6 12-0 12-6 13-0 —	175 	190 170 150 — — —	190 170 150 120 —	16	11-6 12-0 12-6 13-0 13-6 14-0	158 127 114 — —	176 127 114 100 —	176 127 114 100 90 80	16	10-6 11-0 11-6 12-0 12-6 13-0 13-6	239 208 182 — — — — —	250 235 210 187 166 —	250 235 210 187 166 148 132

# $\mathsf{Types}\, 3'' V_{\mathsf{and}}\, 3'' N F_{\mathsf{v}}$



### SECTION PROPERTIES - 3"V

Gage	Weight (psf.)	     (in.)4	S(+) (in.)3	S(-) (in.)3	S <sub>8</sub> (in.) <sup>3</sup>	Area (in.)2	Y (in.)
30" C	OVERAG	E	<b>-</b>		<u> </u>	L	
20 18 16	2.35 3.05 3.85	0.90 1.31 1.67	0.52 0.80 1.04	0.56 0.80 1.03	0.60 0.82 1.04	0.64 0.85 1.06	1.58 1.59 1.60
32" C	OVERAG	E		·	<b>.</b> . —		
20 18 16	2.30 3.00 3.80	0.89 1.30 1.66	0.49 0.76 1.03	0.55 0.80 1.04	0.63 0.86 1.08	0.62 0.83 1.04	1.52 1.52 1.53

The profile of 3"V Deck allows more room than ordinary deck for the placement of shear connectors, making it an ideal component for composite construction. Two recent major projects using 3"V are shown on page 3 of this catalog.

A new feature of 3"V this year is the cold-rolling of ripple-like stiffening ribs into its top and bottom flanges. These miniscule ribs give 3"V added strength and rigidity on the job and provide an extremely skid-resistant surface for the added safety of

## ALLOWABLE SUPERIMPOSED LOADS, TYPE $3^{\prime\prime}V$ Both regular and lightweight concrete slabs.

30″	Covera	ge			30″	Covera	ge			32″	Covera	ge			32"	Covera	ge		
	LIGH Slab (in Mr (ft. Vr (lbs. Wt.(ps	lbs.)	GHT 5½ 3000 1150 35			LIGH Slab(ir Mr(ft. Vr(lbs. Wt.(ps	lbs.)	GHT 6½ 3300 1300 42			LIGH Slab(ir Mr(ft. Vr(lbs. Wt.(ps	ibs.)	GHT 5½ 3200 1200 38			LIGH Slab (ir Mr (ft. Vr (lbs. Wt. (ps	lbs.)	GHT 6½ 3500 1350 45	
CONE	NOITION	S	D	Т	CONI	DITION	s	D	T	CON	DITION	S	D	Т	CONI	DITION	s	D	Т
GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD	
20	10-0 10-6 11-0 11-6	165 143 124	185 163 144 127	185 163 144 127	20	10-0 10-6 11-0 11-6	181 155 —	210 183 160 140	210 183 160 140	20.	10-0 10-6 11-0 11-6	153 131 —	176 154 135 118	176 154 135 118	20	10-0 10-6 11-0 11-6		197 171 148	197 171 148 128
20	12-0 12-6		112 99	112 99		12-0	218	218	122		12-0	186	204	103		10-0 10-6 11-0	250 238 208	250 250 231	250 250 231
	13-0		-	88		11-6 12-0	200 178	200	200		11-6 12-0	164 145	183 164	183 164	18	11-6 11-6 12-0	182 159	208 186	208 186
10	11-6 12-0 12-6 13-0 13-6	179 160 143 127 114	181 167 154 142 130	181 167 154 142 130	18	12-6 13-0 13-6 14-0 14-6	158 139 123	169 156 145 131	169 156 145 131 118	18	12-6 13-0 13-6 14-0 14-6	128 113 —	147 132 119 107	147 132 119 107 96		12-6 13-0 13-6 14-0	_ _ _	165 148 132	165 148 132 117
18	14-0 14-6	102	118	118		15-0	_	=	107		15-0	_	_	87		12-6 13-0	179 166	179 166	179 166
	15-0 15-6	_	107 97 —	107 97 88		14-0 14-6 15-0	135 126 117	135 126 117	135 126 117		13-6 14-0 14-6	140 131 118	140 131 122	140 131 122	16	13-6 14-0 14-6	154 143 127	154 143 133	154 143 133
16	14-6 15-0 15-6 16-0	114 107 100 94	114 107 100 94	114 107 100 94	16	15-6 16-0 —		110 103 —	110 103 —	16	15-0 15-6 16-0	106	114 107 100	114 107 100	10	15-0 15-6 16-0	- - -	124 117 —	124 117 109

workmen. Extensive testing has proven that the ribs do not adversely affect the placement or strength of headed shear connectors.

Where electrification is required, Type  $NF_{\nu}$  can be blended with Type V. Both have the same "V" lock-joint running the full length of each panel. The joint has Hi-Bond lugs formed into it, causing it to act as an integral shear connector. Hi-Bond lugs in the webs of both Types V and  $NF_{\nu}$  deck provide additional composite action.



A built-in, pull-down hanger tab for the support of suspended ceilings is formed into the lock-joint on  $NF_{\nu}$  panels. (See p. 9)

Both V and  $NF_V$  deck are available in lengths up to 45'0''.

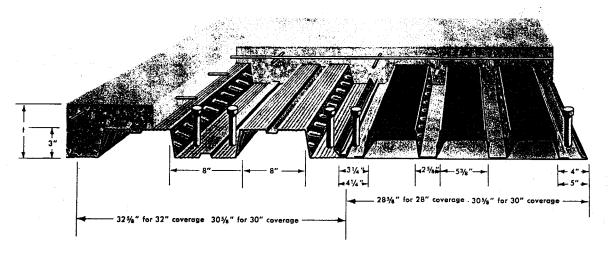
### SECTION PROPERTIES - Type 3"NFv

Gage	Weight (psf.)	1 (in.)4	S(+) (in.)3	S(-) (in.) <sup>3</sup>	S <sub>8</sub> (in.) <sup>3</sup>	Area (in.)2	Y (in.)
28" (	COVERAG	E					
20-20 20-18 18-20 16-18	3.90 4.50 4.60 6.00	1.27 1.37 1.72 2.38	0.52 0.53 0.76 1.07	0.83 0.89 0.99 1.42	1.44 1.77 1.63 2.16	1.10 1.26 1.29 1.66	1.10 0.99 1.25 1.23
30" (	COVERAG	E					•
20-20 20-18 18-20 16-18	3.80 4.30 4.40 5.90	1.21 1.30 1.64 2.28	0.48 0.50 0.72 1.00	0.78 0.83 0.93 1.33	1.42 1.74 1.59 2.11	1.05 1.22 1.24 1.59	1.08 0.86 1.22 1.20

## ALLOWABLE SUPERIMPOSED LOADS, TYPE 3"NFv Both regular and lightweight concrete slabs.

28″, C	28" Coverage 28" Coverage									30″ C	overag	e			30″ C	overag	e		
• N	LIGHT Slab(in.) Ar(ft. lb /r(lbs.) Vt.(psf.	s.)	HT 5½ 4110 1350 40		5 N V	LIGHTWEIGHT Slab(in.) 5½ Mr.(ft. lbs.) 4200 Vr(lbs.) 1400 Wt.(psf) 44					LIGHTWEIGHT Slab(in.) 6½ Mr(ft. lbs.) 4770 Vr(lbs.) 1590 Wt.(psf) 51								
CONDI	TION	S	D	T	CONDI	DITION S D T			CONDI	TION	S	D	Т	CONDITION		S	D	Т	
GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)	×	LOAD	)
20-20	10-0 10-6 11-0	250 . —	250 250 245	250 250 245	20-20	10-0 10-6 11-0	=	250 250 250	250 250 250	20-20	10-0 10-6 11-0		250 250 250	250 250 250	20-20	10-0 10-6	_	250 250	250 250
20 20	11-6 12-0	_	235 221	235 221		11-6	_	250	250		10-0	_	250	250	20-18	10-0 10-6		250 250	250 250
20-18	10-0 10-6 11-0	250 —	250 250 245	250 250 245	20-18	10-0 10-6 11-0 11-6		250 250 250 250	250 250 250 250	20-18	10-6 11-0 11-6	250	250 250 229 250	250 250 229 250	18-20	10-0 10-6 11-0 11-6	250 250 250	250 250 250 250	250 250 250 250
	11-6 12-0 10-6 11-0	250 249	235 225 250	235 225 250 249		10-6 11-0 11-6	250 250 250	250 250 250	250 250 250	10.20	10-6 11-0 11-6	250 250 243	250 250 243	250 250 243	10-20	12-0 12-6 13-0	_	250 235 206	250 235 206
18-20	11-6 12-0 12-6 13-0 13-6	238 228 203	249 238 228 210 195 180	238 228 210 195 180	18-20	12-0 12-6 13-0 13-6 14-0	249 — — — —	250 240 222 205 184	250 240 222 205 184	18-20	12-0 12-6 13-0 13-6 14-0	203 — — — —	233 214 190 169 150	233 214 190 169 150		11-6 12-0 12-6 13-0 13-6	250 250 244 226 209	250 250 244 226 209	250 250 244 226 209
	14-0 14-6 15-0	_ _ _	168 153 137	168 153 137		12-6 13-0 13-6 14-0	240 222 205 191	240 222 205 191	240 222 205 191		12-6 13-0 13-6 14-0	215 199 184 171	215 199 184 171	215 199 184 171	16-18	14-0 14-6 15-0 15-6		195 181 170 159	195 181 170 159
16-18	13-0 13-6 14-0 14-6 15-0 15-6 16-0	195 180 168 156 146	195 180 168 156 146 137 128	195 180 168 156 146 137 128	16-18	14-6 15-0 15-6 16-0		178 166 156 146 —	178 166 156 146 —	16-18	14-6 15-0 15-6 16-0	- - - - - - -	160 149 140 131	160 149 140 131		-   -   -   -   -   -     -     -     -     -       -	     	3	3

# $\mathsf{Types}\, 3'' V_{\mathsf{and}} 3'' N F_{\mathsf{V}}$



### SECTION PROPERTIES - 3"V

Gage	Weight (psf.)	[in.)4	S(+) (in.) <sup>3</sup>	S(-) (in.)3	S <sub>8</sub> (in.) <sup>3</sup>	Area (in.)²	Y (in.)
30″ C	OVERAG	E					<b>1</b>
20 18 16	2.35 3.05 3.85	0.90 1.31 1.67	0.52 0.80 1.04	0.56 0.80 1.03	0.60 0.82 1.04	0.64 0.85 1.06	1.58 1.59 1.60
32″ C	OVERAG	E	•		<b>L</b>	·	
20 18 16	2.30 3.00 3.80	0.89 1.30 1.66	0.49 0.76 1.03	0.55 0.80 1.04	0.63 0.86 1.08	0.62 0.83 1.04	1.52 1.52 1.53

The profile of 3"V Deck allows more room than ordinary deck for the placement of shear connectors, making it an ideal component for composite construction. Two recent major projects using 3"V are shown on page 3 of this catalog.

A new feature of 3"V this year is the cold-rolling of ripple-like stiffening ribs into its top and bottom flanges. These miniscule ribs give 3"V added strength and rigidity on the job and provide an extremely skid-resistant surface for the added safety of

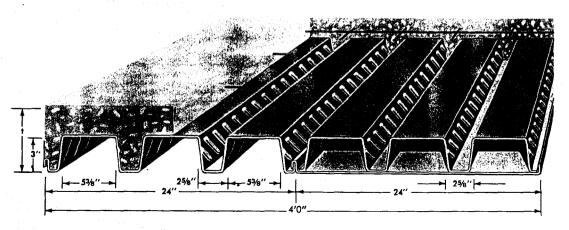
# ALLOWABLE SUPERIMPOSED LOADS, TYPE $3^{\prime\prime}V$ Both regular and lightweight concrete slabs.

30″	Covera	Coverage 30" Coverage									Covera	ge			32″	Covera	ge	VEIGHT 6½						
	LIGH Slab(ir Mr(ft. Vr(lbs. Wt.(ps	lbs.)	GHT 5½ 3000 1150 35			LIGH Slab (ir Mr (ft. Vr (lbs. Wt. (ps	ibs.)	GHT 6½ 3300 1300 42			LIGH Slab(ir Mr(ft. Vr(lbs. Wt.(ps	lbs.)	GHT 5½ 3200 1200 38		LIGHTW Slab(in.) Mr(ft. lbs. Vr(lbs.) Wt.(psf.)			6½ s.) 3500 1350						
CONI	NOITIC	s	D	T	CON	DITION	S	D	T	CON	NOITIO	s	D	Т	CONI	NOITIC	s	D	Т					
GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD		GAGE	SPAN (ftin.)		LOAD	<del></del>					
20	10-0 10-6 11-0 11-6	165 143 124	185 163 144 127	185 163 144 127	20	10-0 10-6 11-0 11-6	181 155 —	210 183 160 140	210 183 160 140	20.	10-0 10-6 11-0 11-6	153 131 —	176 154 135 118	176 154 135 118	20	10-0 10-6 11-0 11-6	 	197 171 148 —	197 171 148 128					
20	12-0 12-6 13-0		112 99 —	112 99 88		12-0 11-0 11-6 12-0	218 200 178	218 200 183	218 200 183		12-0 11-0 11-6 12-0	186 164 145	204 183 164	204 183 164	18	10-0 10-6 11-0 11-6 12-0	250 238 208 182 159	250 250 231 208 186	250 250 231 208 186					
18	11-6 12-0 12-6 13-0 13-6	179 160 143 127 114	181 167 154 142 130	181 167 154 142 130	18	12-6 13-0 13-6 14-0 14-6	158 139 123 —	169 156 145 131	169 156 145 131	18	12-6 13-0 13-6 14-0 14-6	128 113 —	147 132 119 107	147 132 119 107 96		12-6 13-0 13-6 14-0		165 148 132	165 148 132 117					
10	14-0	102	118	118		15-0	_	-	107		15-0	_	_	87		12-6 13-0	179 166	179 166	179 166					
	14-6 15-0 15-6	_ _ _	107 97 —	107 97 88	-	14-0 14-6 15-0	135 126 117	135 126 117	135 126 117		13-6 14-0 14-6	140 131 118	140 131 122	140 131 122	16	13-6 14-0 14-6	154 143 127	154 143 133	154 143 133					
16	14-6 15-0 15-6 16-0	114 107 100 94	114 107 100 94	114 107 100 94	16	15-6 15-6 16-0		110	110 103 —	16	15-6 15-6 16-0	106	114 107 100	114 107 100	10	15-0 15-6 16-0	- - -	124 117 —	124 117 109					



# Type 3"N and 3"NF Celluflor





Standard 24" wide 3" N and 3" NF panels blend to match modules in 2' increments: 4' 6' 8' etc.

Two new panel widths have been added to the 3" N Series to provide greater flexibility in the meeting of architectural modules. Blending a 25½" wide Type 3" NF Celluflor panel with two 17¾6" wide Type 3" N deck panels now makes it possible to offer the ample cell capacity of 3" NF on a 5 ft. module. Still available are regular 24" panels in both types.

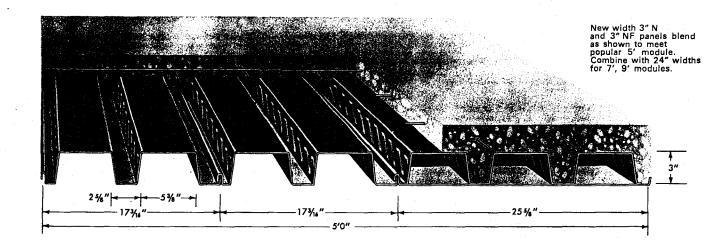
### SECTION PROPERTIES - Type 3"N

Gage	Weight (psf.)	(in.)4	S(+) (in.) <sup>3</sup>	S(-) (in.)3	S <sub>B</sub> (in.) <sup>3</sup>	Area (in.)²	Y (in.)
22	2.20	0.64	0.37	0.45	0.41	0.63	1.83
20	2.65	0.81	0.48	0.56	0.50	0.76	1.83
18	3.40	1.20	0.70	0.77	0.70	1.01	1.83
16	4.20	1.62	0.91	0.97	0.91	1.26	1.84

ALLOWABLE SUPERIMPOSED LOADS, TYPE 3"N Regular and Lightweight concrete slabs.

·	REGULAR   REGULAR   Slab(in.)   51/2   Slab(in.)   6									\ \	Slab(in.)       5½       Slab(ir         Mr(ft. lbs.)       2100       Mr(ft.         Vr(ibs.)       900       Vr(ibs         Wt.(psf.)       32       Wt.(psf.)						os.) 2400 1050 .) 38		
CONDI	TION	S	D	T	CONDI	TION	s	D	T	CONDI	TION	S	D	T	CONDITION		S	D	Т
GAGE	SPAN (ftin.) LOAD			GAGE	E SPAN LOAD		GAGE	SPAN (ftin.)		LOAD		GAGE SPAN		LOAD					
22	8-0 8-6 9-0 9-6	200 — —	200 200 179 151	200 200 179 151	22	8-0 8-6 9-0	200 — —	200 200 190	200 200 190	22	8-0 8-6 9-0 9-6	200 200 181	200 200 200 178	200 200 200 178	22	8-0 8-6 9-0 9-6	200 200 —	200 200 200 200	200 200 200 200
	8-6 9-0 9-6	200 200 178	200 200 186	200 200 186	20	8-6 9-0 9-6 10-0	200	200 200 200 184	200 200 200 184		10-0 10-6 11-0	<u>-</u>	154 134 —	154 134 116		10-0 9-0 9-6	200 200	172 200 200	172 200 200
20	10-0 10-6 11-0	=	168 152 135	168 152 135		10-6 11-0 9-6	200	166	166 142 200		9-6 10-0 10-6	186 168 152	186 168 152	186 168 152	20	10-0 10-6 11-0	192 — —	192 174 159	192 174 159
	10-0 10-6	168 152	168 152	116 168 152	18	10-0 10-6 11-0	184 167 152	184 167 152	184 167 152	20	11-0 11-6 12-0 12-6	137 — — —	139 127 117	139 127 117 108		11-6 12-0 10-6	174	174	145 133 174
18	11-0 11-6 12-0 12-6 13-0	139 127 —	139 127 117 108 99	139 127 117 108 99		11-6 12-0 12-6 13-0		139 128 118 —	139 128 118 109	18	11-6 12-0 12-6 13-0	127 117 108 99	127 117 108 99	127 117 108 99	18	11-0 11-6 12-0 12-6 13-0	159 145 133 123	159 145 133 123 114	159 145 133 123 114
	13-6	447	417	92		11-6 12-0	139	139 128	139 128		13-6 14-0	=	92 86	92 86		13-6 14-0	=	105	105 98.
16	12-0 12-6 13-0 13-6 14-0	117 108 99 92 —	117 108 99 92 86	117 108 99 92 86	16	12-6 13-0 13-6 14-0 —	118 - - - - -	118 109 101 94 —	118 109 101 94	16	13-6 14-0 — —	92 86 — —	92 86 — —	92 86 — —	16	13-0 13-6 14-0 —	114 105 98	114 105 98 —	114 105 98 —





### SECTION PROPERTIES — Type 3"NF

Gage	Weight (psf.)	 (in.)4	S(+) (in.)3	S(-) (in.) <sup>3</sup>	S <sub>8</sub> (in.)3	Area (in.)2	Y (in.)
20-20	4.10	1.41	0.59	0.79	1.46	1.18	1.13
20-18	4.80	1.52	0.61	1.00	1.78	1.34	1.00
18-20	4.90	1.84	0.88	0.97	1.66	1.41	1.35
16-18	6.50	2.53	1.22	1.36	2.21	1.80	1.21

allowable superimposed loads, type  $3^{\prime\prime}\,NF$ 

Regular and Lightweight concrete slabs.

Ticguiu	negular and Lightweight concrete stabs.										1.								T D 200				
N	REG Slab(in.) Ar(ft. lbs /r(lbs.) Vt.(psf.)	s.) :	5½ 3400 900 47		,	REG Slab(in.) Mr(ft. lbs /r(lbs.) Vt. (psf	•	6 3700 1000 52		N V	LIGHT Slab(in.) Ar(ft. lb: /r(lbs.) Vt.(psf.	s.)	HT 5½ 3400 900 34		LIGHTWEIGHT Slab(in.) 61/4 Mr(ft. lbs.) 3850 Vr(lbs.) 1050 Wt.(psf.) 40								
CONDI	TION	S	D	T	CONDI	TION	S	D	T	CONDI	TION	s	D	Т	CONDITION		s	D	T				
GAGE	SPAN (ftin.)		LOAD	D GAGE SPA		SPAN (ftin.)	LOAD ·		GAGE	SPAN (ftin.)	1		GAGE	SPAN (ftin.)		LOAD							
20-20	8-0 8-6 9-0 9-6 10-0 10-6 11-0 11-6	200 200 200 189 180 171 —	200 200 200 189 180 171 164 157 150	200 200 200 189 180 171 164 157	20-20	8-0 8-6 9-0 9-6 10-0 10-6 11-0 11-6	200 200 200 200 200 — — —	200 200 200 200 200 190 182 174 167	200 200 200 200 200 190 182 174 167	20-20	8-0 8-6 9-0 9-6 10-0 11-6 11-6 12-0 12-6	200 200 200 189 180 171 164 157	200 200 200 189 180 171 164 157 150	200 200 200 189 180 171 164 157 150	20-20	8-0 8-6 9-0 9-6 10-0 11-6 11-6 12-0 12-6	200 200 200 200 200 200 191	200 200 200 200 200 200 191 183 175 168	200 200 200 200 200 200 191 183 175 168				
20-18	10-6 11-0 11-6 12-0 12-6	171	171 164 157 150 144	171 164 157 150 144	20-18	10-0 10-6 11-0 11-6 12-0	200	200 190 182 174 167	200 190 182 174 167		13-0 13-6 11-6 12-0	157	138 133 157 150	138 133 157 150	20-18	13-0 11-0 11-6 12-0	191	162 191 183 175	162 191 183 175				
18-20	11-0 11-6 12-0 12-6 13-0	164 157 150 144 138	164 157 150 144 138	164 157 150 144 138	18-20	12-0 12-6	190 182 174 167 160	190 182 174 167 160	190 182 174 167 160	20-18	12-6 13-0 13-6 14-0		144 138 133 129	144 138 133 129	18-20	12-6 13-0 11-6 12-0 12-6	183 175 168	168 162 183 175 168	168 162 183 175 168				
	13-6 14-0	_	133 129	133 129		13-0 13-6 14-0	_	154 148 143	154 148 143		12-0 12-6 13-0	150 144 138	150 144 138	150 144 138	10-20	13-0 13-6 14-0	162 156	162 156 150	162 156 150				
16-18	13-6 14-0 — —	133 — — —	133 129 —	133 129 — —	16-18	13-0 13-6 14-0	154 148 143	154 148 143	154 148 143	18-20	13-6 14-0 —	133 129 —	133 129 —	133 129 — —	16-18	14-0 — —	150 — —	150 — —	150 — —				

## **Design Data**

### **DESIGN DATA — HI-BOND COMPOSITE SLABS:**

All section properties are determined in strict accord with the "Specification for the Design of Light Gage Cold-Formed Steel Structural Members."

In determining values for Hi-Bond slabs without shoring, they are designed as follows:

 Before concrete has attained design strength and deck acts as form:

$$\begin{split} L_{\text{max}} &= \sqrt{\frac{8 \ (+S) \ f}{12 w_c}} \quad \text{Single Span} \\ L_{\text{max}} &= \sqrt{\frac{11 \ (+S) \ f}{12 w_c}} \quad \\ L_{\text{max}} &= \sqrt{\frac{9 \ (-S) \ f}{12 w_c}} \quad \\ L_{\text{max}} &= \sqrt{\frac{11 \ (+S) \ f}{12 w_c}} \quad \\ L_{\text{max}} &= \sqrt{\frac{11 \ (+S) \ f}{12 w_c}} \quad \\ L_{\text{max}} &= \sqrt{\frac{10 \ (-S) \ f}{12 w_c}} \quad \\ \end{split}$$

Where L = clear span in feet

S = section modulus (+ or -)

f = 20,000 psi

w<sub>c</sub> = construction load (dead weight of slab plus 20 psf)

L<sub>max</sub> is also limited to an L/180 deflection with a 1" maximum. In above, A.C.I. Moment Coefficients used.

 Using the values for moment and shear (M, and V,), allowable loading is then determined using the lesser value. (M, and V, are determined empirically, based on a safety factor of no less than 2).

$$w = \frac{8M}{L^2}$$
 or  $w = \frac{2V_r}{L}$ 

3. The allowable superimposed loading is further reduced if necessary so that the following design stresses are not exceeded

$$f_{a} = \frac{33,000 - f_{\text{DL}}}{2} \begin{tabular}{ll} Where & f_{a} \ (psi) \ is the allowable steel stress in the deck, and $f_{\text{DL}}$ (psi) is the steel stress in the bottom flange, induced by the dead weight of the slab. \end{tabular}$$

 $f_c = 1350 \text{ psi}$   $f'_c = 3000 \text{ psi minimum}$ 

- 4. All superimposed loads are based on simple span design of the Hi-Bond slab. The tabulated allowable superimposed loads have been arbitrarily cut off at 200 psf and 250 psf, since Hi-Bond slabs are basically one way reinforced slabs designed for uniformly distributed loads, and loadings greater than 200 or 250 psf usually indicate heavy concentrated moving loads for which additional negative and transverse reinforcing is required.
- 5. Continuous span Hi-Bond slabs are designed as follows:

Unshored Hi-Bond Shored Hi-Bond 
$$+ M_r = \frac{w_i L^2}{10} + M_r = \frac{w_i L^2}{10}$$

$$+ M_r = \frac{w_i L^2}{10} + M_r = \frac{w_i L^2}{10}$$

$$+ M_r = \frac{w_i L^2}{10} = 10' \cdot 0'' - M_r = \frac{w_i L^2}{12} \le 10' \cdot 0''$$

$$+ M_r = \frac{w_i L^2}{10} > 10' \cdot 0'' - M_r = \frac{w_i L^2}{10} > 10' \cdot 0''$$

$$+ W_r = \frac{w_i L^2}{10} > 10' \cdot 0'' - M_r = \frac{w_i L^2}{10} > 10' \cdot 0''$$

$$+ W_r = \frac{w_i L^2}{10} > 10' \cdot 0'' - M_r = \frac{w_i L^2}{10} > 10' \cdot 0''$$

$$+ W_r = \frac{w_i L^2}{10} > 10' \cdot 0'' - M_r = \frac{w_i L^2}{10} > 10' \cdot 0''$$

$$+ W_r = \frac{w_i L^2}{10} > 10' \cdot 0'' - M_r = \frac{w_i L^2}{10} > 10' \cdot 0''$$

$$+ W_r = \frac{w_i L^2}{10} > 10' \cdot 0'' - M_r = \frac{w_i L^2}{10} > 10' \cdot 0''$$

$$+ W_r = \frac{w_i L^2}{10} > 10' \cdot 0'' - M_r = \frac{w_i L^2}{10} > 10' \cdot 0''$$

$$+ W_r = \frac{w_i L^2}{10} > 10' \cdot 0'' - M_r = \frac{w_i L^2}{10} > 10' \cdot 0''$$

$$+ W_r = \frac{w_i L^2}{10} > 10' \cdot 0'' - M_r = \frac{w_i L^2}{10} > 10' \cdot 0''$$

$$+ W_r = \frac{w_i L^2}{10} > 10' \cdot 0'' - M_r = \frac{w_i L^2}{10} > 10' \cdot 0''$$

$$+ W_r = \frac{w_i L^2}{10} > 10' \cdot 0'' - M_r = \frac{w_i L^2}{10} > 10' \cdot 0''$$

$$+ W_r = \frac{w_i L^2}{10} > 10' \cdot 0'' - M_r = \frac{w_i L^2}{10} > 10' \cdot 0''$$

$$+ W_r = \frac{w_i L^2}{10} > 10' \cdot 0'' - M_r = \frac{w_i L^2}{10} > 10' \cdot 0''$$

# Openings in Hi-Bond Floors

Normally, openings in Hi-Bond floors can be classified in three broad categories. The first are those openings less than 10" wide measured at right angles to the deck span. Openings such as these require no special treatment. They can be formed prior to the concrete pour and once the concrete has attained 75% of its design strength, the deck can then be cut out.

For openings 10" to 30", the deck can be reinforced prior to the concrete pour with reinforcing bars or small channels, welded to the deck around the perimeter of the opening, to distribute the loads to the adjoining panels.

For those openings greater than 30", the most practical method generally is to supply supplemental structural framing.

# Use of Header Ducts with Hi-Bond Celluflor

Either Standard Header Ducts or Trench Header Ducts can be used to deliver electrical and telephone wires to the cells of a Celluflor system. In both cases the ducts are normally installed on top of the Hi-Bond Floor Deck, thus interrupting the concrete.

For standard header ducts, 1½" deep and 3" to 6" wide, of 14 gage steel, normally no special treatment is required. The duct itself is stronger than the concrete it replaces and the load carrying capacity of the Hi-Bond slab is rarely affected.

For trench header ducts, normally 21/2" to 4" deep and 9" to 36" wide, location is most important. Because they eliminate all of the concrete above the Hi-Bond deck, some means other than the composite deck/slab system must be utilized to analyze the load carrying capacity of the floor system. In general, trench header ducts should be located as close as possible to supports. The Hi-Bond deck can then be investigated non-compositely and designed to carry the loads. If trench header ducts are required at mid-span, the floor slab can be designed as a double cantilever with negative reinforcing over the adjacent supports. In cases where composite steel beams are used, the placement of the trench header duct directly over the support will interfere with the shear connectors. In this case, the beam will have to be designed non-compositely, or the trench header set to one side, the shear connectors used, and the beam designed compositely with a concrete flange on one side only.

### **Definition of Terms**

These terms are used throughout this catalog:

- +S Section modulus of steel deck, top flange in compression.
- —S Section modulus of steel deck, bottom flange in compression.
- S<sub>8</sub> Section modulus of steel deck, to bottom flange.
- I Moment of inertia, steel deck.
- As Area of steel, steel deck.
- L Clear span in feet. All spans shown in load tables are clear spans.

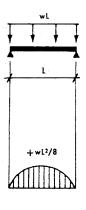
- V Distance from bottom flange to neutral axis of deck.
- $\begin{array}{c} W_c \text{Dead load, concrete} \\ + \text{ steel deck.} \end{array}$
- V<sub>R</sub> Resisting shear, composite.
- M<sub>R</sub> Resisting moment, composite.
- F<sub>y</sub> 33,000 psi (yield stress)
- f .6F<sub>y</sub> = 20,000 psi (design stress)
- w Allowable superimposed load in psf, as shown in all tables.





### DESIGN DATA - NON-COMPOSITE SLABS

All section properties are determined in strict accord with the "Specification for the Design of Light Gage Cold-Formed Steel Structural Members." Allowable loads should be determined using the following criteria:



 $M = wl^2/8$   $\Delta = 5wl^4/384 EI$ 

For + \$ Reqd., Use chart \$ For | Reqd., Use chart | +wL<sup>2</sup>/12 +wL<sup>2</sup>/12 -wL<sup>2</sup>/8

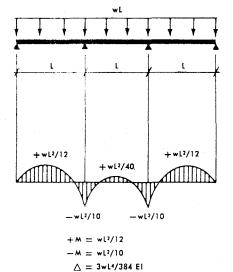
> $+ M = wL^{2}/12$  $- M = wL^{2}/8$

△ = 3wL4/384 El

For +S Reqd., Multiply chart S by 8/12

For —S Read., Use chart S

For 1 Read., Multiply chart 1 by 3/5



For +S Read., Multiply chart S by 8/12

For —\$ Read., Multiply chart \$ by 8/10

For I Read., Multiply chart I by 3/5

### REQUIRED SECTION PROPERTIES FOR TOTAL UNIFORM LOADS AND SIMPLE SPANS — NON-COMPOSITE DESIGN

 $f_{\rm S} = 20,000~{\rm psi}$ .

 $E=29.5\times10^6\,\mathrm{psi}$ .

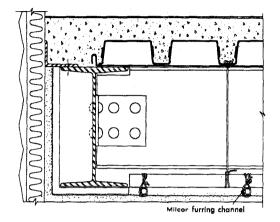
						· L	OADS I	N POUN	DS PER	SQUARE	FOOT			
SPANS	CRITERI	Α	25	50	75	100	125	150	175	200	225	250	275	300
		S	.07	.14	.21	.27	.34	.42	.49	.54	.63	.68	.75	.81
6″-0″	L/240 — L/360 —	1	.08 .12	.16 .25	.24 .37	.33 .49	.41 .61	.48 .74	.56 .86	.66 .99	.72 1.11	.82 1.22	.90 1.34	.99 1.48
		S	.09	.18	.27	.37	.46	.54	.63	.74	.81	.92	1.01	1.10
7′-0″	L/240 — L/360 —	1	.13 .20	.26 .39	.39 .59	.52 .79	.65 .99	.78 1.18	.91 1.38	1.05 1.57	1.17 1.77	1.30 1.98	1.43 2.18	1.57 2.35
		S	.12	.24	.36	.48	.60	.72	.84	.96	1.08	1.20	1.32	1.44
8′-0"	L/240 — L/360 —	1	.20 .29	.39 .59	.59 .88	.78 1.17	.98 1.46	1.18 1.76	1.38 2.05	1.56 2.34	1.77 2.64	1.96 2.92	2.16 3.21	2.34 3.52
		S	.15	.31	.46	.61	.76	.92	1.07	1.22	1.38	1.52	1.67	1.83
9′-0″	L/240 — L/360 —		.28 .42	.56 .83	.84 1.25	1.11 1.67	1.39 2.09	1.68 2.50	1.96 2.92	2.22 3.34	2.52 3.75	2.78 4.18	3.06 4.60	3.34 5.00
		s	.19	.38	.57	.75	.94	1.14	1.33	1.50	1.71	1.88	2.07	2.25
10′-0″	L/240 — L/360 —	l I	.38 .57	.76 1.14	1.14 1.71	1.53 2.29	1.91 2.86	2.28 3.42	2.66 3.99	3.05 4.58	3.42 5.13	3.82 5.72	4.20 6.29	4.58 6.86
		S	.23	.45	.68	.91	1.14	1.36	1.59	1.82	2.04	2.28	2.51	2.73
11′-0″	L/240 — L/360 —		.51 .76	1.02 1.52	1.53 2.28	2.03 3.05	2.54 3.81	3.06 4.56	3.57 5.32	4.06 6.09	4.59 6.84	5.08 7.62	5.59 8.38	6.09 9.14
		s	.27	.54	.81	1.08	1.35	1.62	1.89	2.16	2.43	2.70	2.97	3.24
12'-0"	L/240 — L/360 —		.66 .99	1.32 1.98	1.98 2.97	2.64 3.95	3.30 4.94	3.96 5.94	4.62 6.93	5.27 7.91	5.94 8.91	6.60 9.88	7.26 10.87	7.91 11.86
		S	.32	.63	.95	1.27	1.59	1.90	2.22	2.54	2.85	3.18	3.50	3.81
13′-0″	L/240 — L/360 —	1	.84 1.26	1.68 2.51	2.52 3.77	3.35 5.03	4.19 6.29	5.04 7.54	5.88 8.80	6.70 10.06	7.56 11.31	8.38 12.58	9.22 13.84	10.05 15.08
		S	.37	.74	1.11	1.47	1.84	2.22	2.59	2.94	3.33	3.68	4.05	4.41
14′-0″	L/240 — L/360 —		1.05 1.57	2.09 3.14	3.14 4.71	4.19 6.28	5.24 7.85	6.28 9.42	7.33 10.99	8.37 12.55	9.42 14.13	10.48 15.70	11.53 17.27	12.56 18.83
		S	.42	.85	1.27	1.69	2.11	2.54	2.96	3.38	3.81	4.22	4.64	5.07
15′-0″	L/240 — L/360 —	1	1.29 1.93	2.57 3.86	3.86 5.79	5.15 7.72	6.44 9.65	7.72 11.58	9.01 13.51	10.30 15.44	11.58 17.37	12.88 19.30	14.17 21.23	15.45 23.17

### NOTES

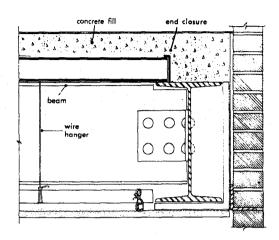
- 1. Chart based on M = wL<sup>2</sup>/8,  $\triangle$  = 5wL<sup>4</sup>/384 EI.
- 2. For loads other than those shown, straight line interpolations may be made for both the S and I properties.
- 3. For spans other than those shown, S property must be interpolated by the square power, and I by the third power.

## **Construction Details**

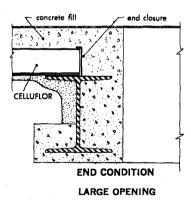
### Side condition

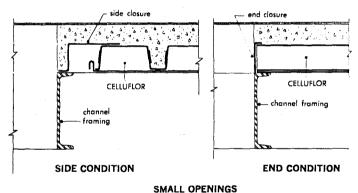


### End condition

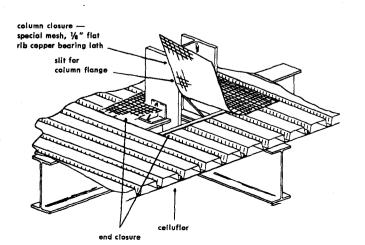


### Framed openings — Pipes — Ductwork etc.

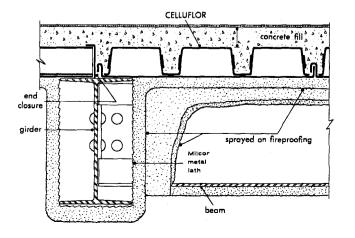




### Wet Column (Not to scale.)



### Cell direction change

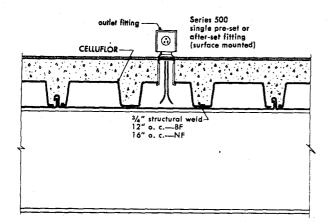




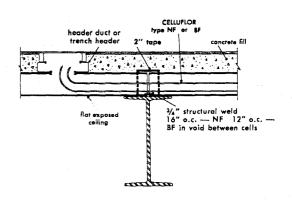


### **Construction Details**

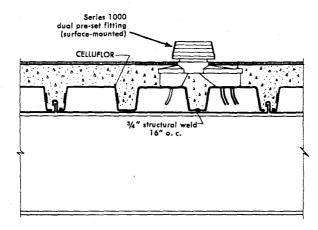
### Electrification



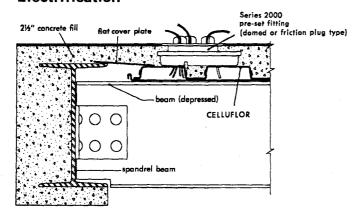
### Electrification



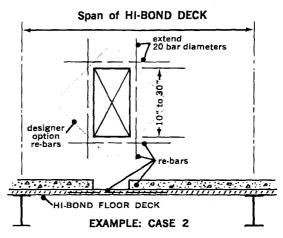
### Electrification



### Electrification



### Openings in Hi-Bond Floors



## **Specifications**

# INRYCO Celluflor and Floor Deck

### 1. Scope

- a. Included are all steel subfloors and their accessories. For location, type, and gage of steel subfloors see structural drawings.
- b. Not included under this section of the specification (work included under other sections):
- (1) Structural steel columns, girders, beams, and all miscellaneous bracings or supports of any kind for the steel subfloors.
- (2) All reinforcing bars and reinforcing mesh.
- (3) The cutting of holes in the subfloor for the passage of all materials for other trades.
- (4) The cutting or drilling required for the attachment of materials of other trades to the steel subfloor.
- (5) The final placement and attachment of welding access hole covers, if required, and the taping of abutting ends of electrified panels.
- (6) Electrification of the steel subfloor including headers, jump headers, outlets, and any other materials required to carry wires outside of the cellular panel cells.
- (7) Concrete fill (Note: for Hi-Bond composite construction a minimum concrete strength of f'c=3,000 psi is required. For steel subfloor construction a low water-cement ratio and proper curing techniques are recommended to control crazing or temperature relief cracks).
- (8) Fireproofing on the underside of steel subfloors.
- (9) Any additional holes or cutting not indicated on the erection drawings shall be checked with and authorized in writing by the general contractor, since these holes or cut areas may block vital electrical cells or may be of size or shape requiring additional structural supports.

### 2. Material

- a. All non-cellular subfloor deck and accessory items shall be formed from carbon steel sheets conforming to ASTM A 611-70 (formerly ASTM A 245-64) with a minimum yield strength of 33,000 psi. Prior to forming, the sheets shall be given an iron phosphate treatment, and the exposed side of the deck (that side not covered by the concrete) shall be given two coats of factory baked-on enamel (phosphatized/painted).
- b. All cellular subfloor deck and accessory items shall be formed from galvanized steel sheets conforming to ASTM A 446-71 with a minimum yield strength of 33,000 psi. Galvanized coating shall be in accordance with ASTM A 525-71 and Federal Specification QQ-S-775d Type I Class e. (Top fluted sections Ti-Co Galvanized Light Commercial nominal  $\frac{1}{2}$  oz.; flat plates Ti-Co Galvanized Paintite A nominal  $\frac{1}{2}$  oz.).
- c. (To be inserted if non-cellular subfloor deck is an exterior application, or an application where excessive moisture is present.) All non-cellular subfloor deck and accessory items shall be formed from galvanized steel sheets conforming to ASTM A 446-71 with a minimum yield strength of 33,000 psi. Galvanized coating shall be in accordance with ASTM A 525-71 and Federal Specification QQ-S-775d Type I Class e. (Ti-Co Galvanized Paintite A nominal ½ oz.).

### 3. Construction

- a. Steel subfloor shall conform to the Inland-Ryerson Construction Products Company's Type(s) ... as to depth, cell area, cell spacing and design. (Refer to page ... for specific characteristics of various Inland-Ryerson profiles. The designer may wish to be specific regarding these points rather than making reference to catalog data.)
- b. When two sections are combined to form a cellular panel, they shall be structurally resistance welded to develop their full section properties.
- c. (To be inserted if panels are to be used as electrical raceways.) Panels shall be listed and labeled by Underwriters' Laboratories, Incorporated.
- d. Deformations shall be provided in all vertical webs of the steel subfloor adequate to structurally bond the overlying structural fill material. This action shall be demonstrated in

tests approximating building requirements and such tests shall be made available to the structural engineer upon request.

e. (To be inserted for 2" or 3" Type V Deck.) Continuous deformations, longitudinal to the panel, shall be provided in the flanges of all deck panels.

### 4. Design

- a. Sections and calculation of their properties shall conform to the American Iron and Steel Institute's "Specifications for the Design of Light Gage Cold-Formed Steel Structural Members."
- b. (To be inserted if non-composite type subfloor is used.) Load carrying capacity shall be computed on the basis of maximum flexural stress of 20,000 psi and a maximum deflection (as caused by live load) of Span/360.
- c. (To be inserted if steel subfloor and concrete are designed compositely.) Load carrying capacity shall be computed using the following criteria: 20,000 psi maximum steel flexural stress for dead load plus construction load; yield stress less flexural stress under dead load, divided by 2 for live load; 1350 psi maximum concrete flexural stress.

### 5. Accessories

Where required or shown on the plans:

- a. Furnish sheet metal closures for open ends of all cell raceways at columns, walls, and openings shown on contract drawings.
- b. Provide sheet steel cover plates (or closure tape) as required to close panel end conditions where panels change direction or abut.
- c. Furnish material for column closures to close openings between panels and structural columns.
- d. Provide welding hole cover, with friction fastening, to close welding access holes when required.

### 6. Cutting and Drilling Steel Subfloors

- a. Where large predetermined openings for stairs, elevators, etc., occur, the steel subfloors shall be engineered by the manufacturer to fit these conditions as shown on structural design drawings. The reinforcing required for these openings shall be provided by others.
- b. Where holes or openings 6" in diameter and larger are required in subfloors, such holes shall be made by the respective trades requiring them and openings reinforced by these trades. The designer shall determine whether these holes need to be reinforced during construction.
- c. Holes smaller than 6" in diameter required for passage of other work or for attachment to the subfloors shall be made by the respective trade involved. The designer shall determine whether these holes need to be reinforced during construction.
- d. Diagonal supports at columns, and any other miscellaneous supports required to carry the steel floor, shall be furnished and installed by the structural steel contractor.

### 7. Erection

- a. The steel units shall be placed on supporting steel framework and adjusted to final position before being permanently fastened. Each unit shall be brought to proper bearing on the supporting beams. If the supporting beams are not in proper alignment or at proper level, the subcontractor shall immediately notify the general contractor who shall have corrections made.
- b. Panels shall be fastened to the steel framework at ends and at intermediate supports by welds not less than  $\frac{3}{4}''$  diameter, spaced not more than 12" across the width of B, S, SR or BF panels and not more than 16" across the width of  $1\frac{5}{8}''$  NF and NFs, 2" V, 3" N, V, NF or NF $_{\nu}$  panels.
- c. End closures of the panel are to be fastened by tack welding or sheet metal screws not more than 4'0" on center.
- d. Side closures of the panel are to be fastened by tack welding not more than  $3^{\prime}0^{\prime\prime}$  on center. Sheet metal screws shall not be used.
- e. Cut and place column closures as indicated on erection drawings.
- f. (To be inserted when panels are used as electrical raceway.) Panels shall be aligned and placed in accordance with the "Standards for Safety Cellular Metal Floor Raceways and Fittings" published by Underwriters' Laboratories, Inc. Taping of abutting ends by the electrical contractor after inspection of the cells.

## **Specifications**

# INRYCO Preset Inserts and Service Fittings

(These items to be inserted in the Cellular Deck Specification)

### 1. Scope

#### a. Work Included:

- (1) Factory Punched Holes: All cellular panels for use in designated areas shall be provided with factory-punched holes for installation of specified pre-set inserts as indicated on the drawings.
- (2) Pre-Set Inserts: The Cellular deck manufacturer shall provide Pre-Set Inserts, as specified, to the general contractor for installation by the electrical contractor in areas indicated on the drawings.
- (3) Service Fittings: The Cellular deck manufacturer shall provide service fittings, as specified, to the general contractor for installation by the electrical contractor.
- b. Work Not Included: Installation or activation of any preset inserts, after-set inserts, or service fittings.

### 2. Material

#### a. Pre-Set Inserts.

(1) Where indicated, all pre-set inserts shall be as manufactured by Inland-Ryerson. All inserts shall provide 2½ dia. accesses into cellular raceways. Design all inserts for cold-forging of the entire circumference to the decking to guarantee a water-tight fit, continuity of ground, and to resist dislodging during placement of concrete. The insert shall have no obstructions, burrs, or rough penetrations, and shall be long enough to accommodate the depth of the slabs as indicated on the drawings. Cover caps shall be provided to prevent ingress of water and wet concrete and to support floor loads over inactive inserts.

Ail inserts shall be UL Approved.

- (2) All inserts shall be:
- (a) Series 500: 2½" dia. galvanized steel with a 2" NPSM screw thread to receive fittings at the floor level.
- (b) Series 1000: Die-cast zinc alloy,  $\frac{1}{8}$ " thick, with outside dimensions of 3" x 93%". The box shall be divided for power and signal lines, with ample room for telephone cables and amphenol connectors. The base shall be  $\frac{1}{2}$ " high and provide access holes  $\frac{6}{8}$ " o.c. If required, a  $\frac{3}{4}$ " high extension shall be provided for areas where the slab thickness is  $\frac{3}{4}$ " above the deck. The top of the box shall provide a 3" dia. straight pipe thread access to the dual box and be connected to the base with drive screws.

Dividers shall be provided between the telephone and electrical portion of the box.

(c) Series 2000: Die-cast zinc alloy,  $\frac{1}{8}$ " thick, with outside dimensions of  $5\frac{3}{4}$ " x  $9\frac{1}{8}$ ". The box shall be divided for power and signal, with ample room for telephone cables and amphenol connectors.

The base shall be  $1\frac{1}{2}$ " high and provide access holes  $6\frac{1}{8}$ " o.c. if required, a  $\frac{3}{4}$ " high extension shall be provided for areas where the slab thickness is  $3\frac{1}{4}$ " above the deck.

The top of the box shall provide a 5" dia. straight pipe thread access to the dual box and be connected to the base with drive screws. Dividers shall be provided between the telephone and electrical portion of the box.

### b. Pre-Set Insert Service Fittings.

- (1) Pre-set insert service fittings shall be as manufactured by Inland-Ryerson. All fittings shall be UL approved.
- (2) All fittings shall be:
- (a) Series 500: Single-service cube of anodized (or superhard-coat anodized) aluminum with receptacle or neoprene telephone cable gasket, as required. Fittings for carpeted areas shall be designed so installation can be made by slitting only, without cutting holes in the carpet. Fittings for tile areas shall provide a water-tight bearing on the surface of the tile.
- (b) Series 1000: Dual-service, low-profile fitting in brushed or anodized) aluminum, providing duplex grounding electrical receptacle and telephone service opening with neoprene cable gasket, as required. Fittings shall be provided with removable

### INRYCO FLOOR SYSTEMS

tops for easy access and a divider plate to separate high and low tension compartments. Blank fitting plates shall be supplied when a service fitting is used for single service.

(c) Series 2000: Dual-service, flush-floor fitting, to be one of following:

Security dome type: 6" dia., flush, bronze or brushed aluminum closure plate and ring, with three security domes. Threaded slugs to be provided for inactive outlets. Aluminum receptacle cover plate provided for installation within the insert box. Active-but-idle fittings to be finished with cover plate ring and closure plate, to receive carpet (or tile).

<u>Friction plug type:</u> 4" dia. closure ring and plug, in anodized (or super-hard-coat anodized) aluminum. Plug prepared for power and telephone and fitted snug with integral nylon retaining ring. Sloped aluminum receptacle cover plate provided for installation within the insert box. Active-but-idle fittings to be finished with blank plug.

(3) Service Fittings shall be supplied with electrical, telephone, or blank faces, as required. The number of each for initial installation shall be:

a) \_\_\_\_\_each Series 500 single-service cubes.

(b) \_\_\_\_\_ each Series 1000 dual-service, low profile fittings.

(c) \_\_\_\_\_each Series 2000 dual-service, flush-floor fittings.

(These items to be inserted in the Electrical Section of the Specifications:)

### Scope

Installation of all pre-set inserts and service fittings supplied under the metal subfloor section of the specifications.

### 2. Materials

Pre-Set Inserts and Fittings: shall be Inryco Series (500, 1000 or 2000) as described in the metal subfloor section of the specifications.

#### 3. Installation

#### a. Pre-Set Inserts.

- (1) Receive all pre-set inserts from the general contractor.
- (2) Distribute and install in areas designated on the drawings, cold-forging to the metal deck in factory-punched holes, in such a manner to provide a continuous, smooth, watertight connection for the entire circumference of the insert, and to resist a minimum of 1000 lbs. of pressure prior to pull-out.
- (a) Series 500: Installed upon completion, insert metal cap to prevent ingress of water or concrete.
- (b) Series 1000: Install so the larger void will be placed over the telephone raceway. Extensions, if required, and tops shall be connected to the base with drive screws or self-threading screws. Caps shall be placed prior to pouring concrete to prevent ingress of water or concrete.
- (c) Series 2000: Install so the larger void will be placed over the telephone raceway.

Extensions, if required, and tops shall be connected to the base with drive screws or self-threading screws. Caps shall be placed prior to pouring concrete to prevent ingress of water or concrete.

### b. Pre-Set Insert Service Fittings.

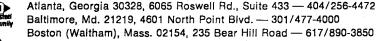
- (1) Receive all service fittings from the general contractor.
- (2) Distribute and install as follows:
- (a) Series 500: Locate pre-set insert. Remove concrete cover and cap. Install fitting base, using carpet or tile type fitting as required. Activate and install top of fitting.
- (b) Series 1000: Locate the pre-set insert. Remove concrete cover and cap. Install base and bring to the surface level. Place the divider plate with extension to fit into the slot provided in the pre-set insert. Activate, place top and face plates as required.
- (c) Series 2000: Locate the pre-set insert. Remove concrete cover and cap, activate and install fittings as follows:

<u>Security Dome:</u> Place height adjusting trim ring and closure plate with security domes and/or threaded slugs as required. <u>Friction Plug:</u> Place adjusting ring to bring to surface level. Place cover and trim ring, adjusting to surface level. Install force-fit hand hole cover.

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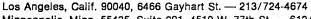
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