



$$I_z = A \cdot r_z^2$$

$$I_u = I_x + I_y - I_z$$

$$J = \frac{A \cdot t^2}{3}$$

$$r_o^2 = u_o^2 + z_o^2 + \frac{I_u + I_z}{A}$$

$$C_1 = \frac{x_o^2}{2} \cdot [y_o^2 - (y_o - b)^2] + \frac{y_o^4 - (y_o - b)^4}{4} + \frac{y_o}{3} \cdot [x_o^3 - (x_o - d)^3] + y_o^3 \cdot d$$

$$C_2 = \frac{x_o}{3} \cdot [y_o^3 - (y_o - b)^3] + x_o^3 \cdot b + \frac{x_o^4 - (x_o - d)^4}{4} + \frac{y_o^2}{2} \cdot [x_o^2 - (x_o - d)^2]$$

$$\beta_z = \frac{t \cdot (c_1 \cdot \sin(\alpha) + c_2 \cdot \cos(\alpha))}{I_z} - 2 \cdot u_o$$

$$\beta_u = \frac{t \cdot (c_1 \cdot \cos(\alpha) - c_2 \cdot \sin(\alpha))}{I_u} - 2 \cdot z_o$$

FIGURE 11.7 Definition of cross-section properties.

the end-restraint effect becomes complicated. Only a few solutions are available (Trahair, 1969; Kitipornchai and Lee, 1986). An acceptable design office solution is the use of effective-length factors so that Eqs. 11.2 and 11.3 are

$$P_u = \frac{\pi^2 E I_u}{(K_u L)^2} \quad (11.7)$$

$$P_z = \frac{\pi^2 E I_z}{(K_z L)^2} \quad (11.8)$$

where  $K_u L$  and  $K_z L$  are the effective length in the  $u$  and  $z$  directions, respectively.

from the Handbook:  $\alpha, \bar{x}, \bar{y}, I_x, I_y, r_z, A$

C: centroid (0,0)

S: shear center ( $u_o, z_o$ )

$x, y$ : non-principal axes

$u, z$ : principal axes

$t$ : thickness of the angle

$$x_o = \bar{x} - \frac{t}{2}$$

$$y_o = \bar{y} - \frac{t}{2}$$

$$u_o = y_o \cdot \sin(\alpha) + x_o \cdot \cos(\alpha)$$

$$z_o = y_o \cdot \cos(\alpha) - x_o \cdot \sin(\alpha)$$

$$d = D - \frac{t}{2}$$

$$b = B - \frac{t}{2}$$

### 11.3.3 Inelastic Behavior

Equations 11.2 and 11.3 (or Eqs. 11.7 and 11.8) can be used to predict the inelastic behavior of angle columns (1986) using the finite element method to replace the elastic modulus  $E$  in the tangent modulus  $E_t$ . The shape factor  $K$  is given in Chapter 3 of this guide. Use of the tangent modulus method is discussed in Appendix E3 of the AISC LRFD Specification as follows:

1. Determine the elastic critical load  $P_{e1}$ .
2. Compute an equivalent slenderness ratio  $\lambda_{eq}$  as follows:

$$\lambda_{eq} = \frac{L}{r_{eq}}$$

3. Determine the buckling load  $P_n$  from the AISC LRFD specification.

It should be noted, however, that the AISC (2000) specification does not require the use of the equivalent slenderness ratio for hot-rolled single angles.

Kitipornchai (1983) suggested the use of the equivalent slenderness ratio from curve-fitting single angles

$$\left(\frac{L}{r}\right)_{eq} = 0$$

and for unequal-leg angles

$$\left(\frac{L}{r_z}\right)_{eq} = \left[\left(\frac{L}{r_z}\right)^3 - 8\right]^{1/3}$$

where  $\alpha_1 = D/B$  and  $\alpha_2 = B/t$ .

Tests performed by Kennedy and his co-workers (1979) using this method provided a satisfactory comparison with experimental results. This method has been provided by Marsh (1969) for aluminum angles with single- and double-leg angles dictated by