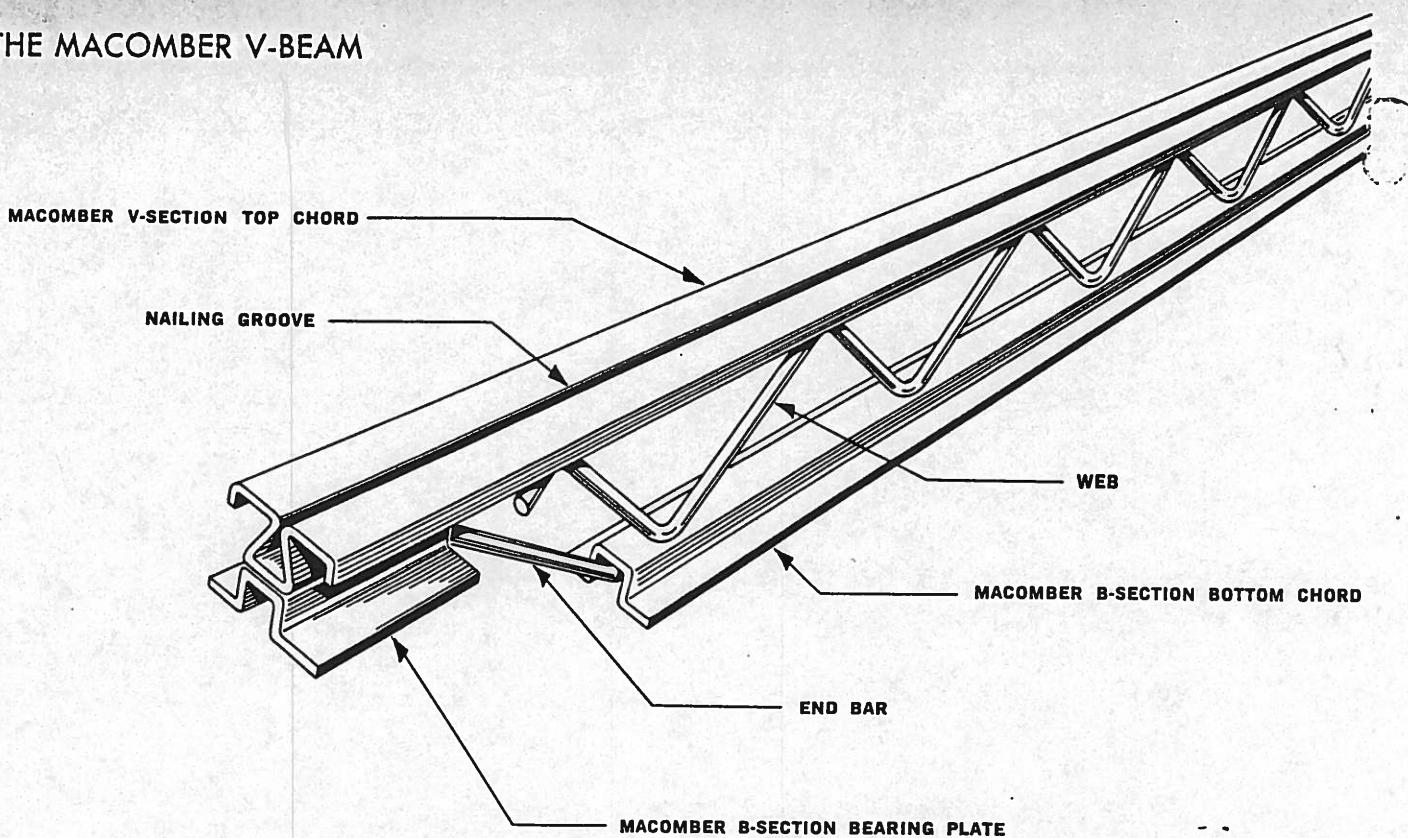


THE MACOMBER V-BEAM



A DEPENDABLE JOIST FROM A RELIABLE MANUFACTURER

The Macomber V-BEAM is the product of more than forty years of leadership in the steel building field. V-BEAMS are engineered to provide you with unexcelled performance and quality. The performance and design of this member is verified by the results of more than a thousand load tests on full size joists, conducted since 1950 in the V-BEAM development sequence. In addition, the success of hundreds of thousands of V-BEAMS on the job lends proof to our claims for this product.

SUGGESTED SPECIFICATION

Steel joists shall be designed to support the live and dead loads shown on the drawings and shall have factor of safety and live load deflections as follows:

Roof Joists

Factor of safety 1.65. Maximum live load deflection 1/360 - 1/240 or 1/180 of span.

Floor Joists

Factor of safety 1.65. Maximum live load deflection 1/360 of span.

See footnotes of V-BEAM load table for limitations on use of this member as floor joist.

The clear span of all joists shall not exceed 550 times the top chord radius of gyration about the vertical axis.

The Macomber V-BEAM is available in depths from 8" to 28" and in spans to 56'. The V-BEAM is made with cold rollformed chords. The Macomber V-Section is utilized as the top chord and a Macomber B-Section serves as the bottom chord, providing the joist with unusual lateral stability. The chords are proportioned in accordance with the latest AISI specifications.

It will be to your advantage to check on the V-BEAM for use in your next project, it's a dependable joist. For information, help, or a quotation on your needs, call or write to the Macomber representative nearest you.

Paint

Joists and accessories shall be painted one shop coat of manufacturer's standard protective paint which can be readily field painted in exposed areas without bleeding through.

Inspection and Test Reports

The joist manufacturer must provide resident inspection of his production by a nationally recognized independent testing laboratory, in addition to the product inspection done by his own forces. The manufacturer must submit, with his approval drawings, representative certified reports by such independent testing laboratory covering tests of his joists, as proof of the performance and quality of workmanship of his products.

If the manufacturer does not have resident testing laboratory inspectors in his plant, he may provide independent testing laboratory inspection of this specific project continuously during manufacture; and make load tests, supervised by that testing laboratory, of one out of every 50 joists (including one joist of each different top chord size and depth) to prove compliance with the safety factor and live load deflection requirements of this specification.

Bridging

Bridging shall be horizontal angles or other shapes, attached to the chords by welding or other mechanical means. The slenderness ratio of the bridging between joists shall not exceed 300. The slenderness ratio of the joist top chord between bridging lines shall not exceed 150. The slenderness ratio of the bottom chord between bridging points shall not exceed 150 for floor joists or 300 for roof joists.

Erection

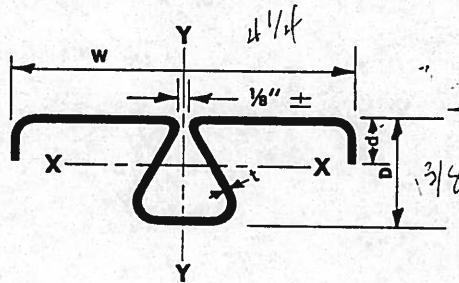
Joists shall be carefully handled to prevent damage. Joists shall be accurately spaced, anchored, and completely bridged prior to the application of construction loads.

DEFLECTIONS

The V-BEAM load table shows maximum live loads for deflection limits of 1/360 and 1/240 of the span. To assist design authorities, who must meet unusual deflection limitations, Macomber has developed and published the most extensive deflection information ever compiled on steel joists. This information may be obtained through your Macomber Representative.

MACOMBER CHORD PROPERTIES

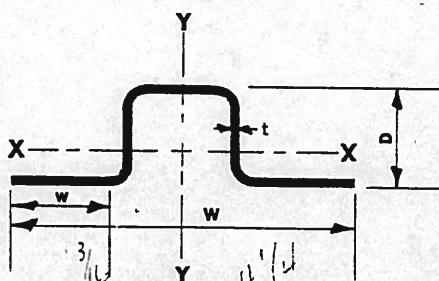
V-SECTION TOP CHORD



DIMENSIONS ARE SUBJECT TO
BLANK-GAGE AND ROLLING TOLERANCE

V-Beam Size	Area in. ²	Gage t in.	W in.	d	D	AXIS X-X			r _{y y} in.
						I	S	r	
A	0.374	0.057	2 1/16	5/16	1.31	0.08	0.09	0.46	0.70
B	0.454	0.068	2 1/16	5/8	1.32	0.09	0.10	0.46	0.72
C	0.563	0.085	2 1/16	5/8	1.34	0.11	0.13	0.45	0.72
D	0.698	0.098	3 1/2	5/16	1.35	0.14	0.16	0.46	0.87
E	0.833	0.115	3 1/2	1 1/16	1.37	0.16	0.18	0.45	0.90
F	0.921	0.126	3 1/2	3/4	1.38	0.19	0.21	0.45	0.94
G	1.008	0.126	4 1/16	1 1/16	1.38	0.21	0.22	0.45	1.12
H	1.202	0.148	4 1/16	3/4	1.40	0.23	0.24	0.45	1.15
I	1.372	0.139	5 1/16	7/8	1.39	0.25	0.25	0.43	1.60
J	1.705	0.176	5 1/16	7/8	1.43	0.32	0.32	0.43	1.61

B-SECTION BOTTOM CHORD



DIMENSIONS ARE SUBJECT TO
BLANK-GAGE AND ROLLING TOLERANCE

V-Beam Size	Area in. ²	Gage t in.	W in.	w	D	AXIS X-X			r _{y y} in.
						I	S	r	
A	0.352	0.084	2 1/16	5/16	1.02	0.05	0.09	0.38	0.78
B	0.436	0.099	3 1/16	3/4	1.04	0.06	0.10	0.38	0.83
C	0.542	0.124	3 1/8	1 1/16	1.06	0.08	0.13	0.38	0.82
D	0.654	0.124	4 1/16	1 3/16	1.06	0.09	0.14	0.38	1.06
E	0.778	0.173	3 3/8	3/4	1.11	0.12	0.18	0.39	0.85
F	0.854	0.173	3 3/4	1	1.11	0.13	0.18	0.38	0.96
G	0.929	0.173	4 1/4	1 3/16	1.11	0.13	0.19	0.38	1.08
H	1.054	0.173	4 5/16	1 3/16	1.11	0.14	0.19	0.37	1.27
I	1.240	0.190	5 3/8	1 3/4	1.13	0.16	0.21	0.37	1.43
J	1.623	0.223	6 1/4	2 1/4	1.16	0.21	0.25	0.36	1.64

V-BEAM DIMENSIONS AND PROPERTIES

Designation Number	Nominal Depth (in.)	I of Chords (in. ⁴)	End Panel Length		Interior Panel Length	Bearing Plate		Standard End Bearing Depth (in.)
			Minimum	Maximum		Width (in.)	Thickness (in.)	
8A	8	9.5	2'-0"	2'-10"	2'-0"	4½	¾	2½
10A	10	15.5	2'-0"	2'-10"	2'-0"	4½	¾	2½
12A	12	22.8	2'-0"	2'-10"	2'-0"	4½	¾	2½
8B	8	11.7	2'-0"	2'-10"	2'-0"	4½	¾	2½
10B	10	19.0	2'-0"	2'-10"	2'-0"	4½	¾	2½
12B	12	28.1	2'-0"	2'-10"	2'-0"	4½	¾	2½
14B	14	39.0	2'-0"	2'-10"	2'-0"	4½	¾	2½
16B	16	51.6	2'-0"	2'-10"	2'-0"	4½	¾	2½
10C	10	23.7	2'-0"	2'-10"	2'-0"	4½	¾	2½
12C	12	35.0	2'-0"	2'-10"	2'-0"	4½	¾	2½
14C	14	48.5	2'-0"	2'-10"	2'-0"	4½	¾	2½
16C	16	64.2	2'-0"	2'-10"	2'-0"	4½	¾	2½
18C	18	82.1	2'-0"	2'-10"	2'-0"	4½	¾	2½
10D	10	29.6	2'-0"	2'-10"	2'-0"	6	¾	2½
12D	12	43.6	2'-0"	2'-10"	2'-0"	6	¾	2½
14D	14	60.2	2'-0"	2'-10"	2'-0"	6	¾	2½
16D	16	79.6	2'-0"	2'-10"	2'-0"	6	¾	2½
18D	18	101.6	2'-0"	2'-10"	2'-0"	6	¾	2½
20D	20	126.4	2'-0"	2'-10"	2'-0"	6	¾	2½
22D	22	153.9	2'-0"	2'-10"	2'-0"	6	¾	2½
24D	24	184.0	2'-0"	2'-10"	2'-0"	6	¾	2½
26D	26	216.9	2'-0"	2'-10"	2'-0"	6	¾	2½
28D	28	252.4	2'-0"	2'-10"	2'-0"	6	¾	2½
12E	12	51.7	2'-0"	2'-10"	2'-0"	6	¾	2½
14E	14	71.6	2'-0"	2'-10"	2'-0"	6	¾	2½
16E	16	94.6	2'-0"	2'-10"	2'-0"	6	¾	2½
18E	18	120.6	2'-0"	2'-10"	2'-0"	6	¾	2½
20E	20	150.3	2'-0"	2'-10"	2'-0"	6	¾	2½
22E	22	183.0	2'-0"	2'-10"	2'-0"	6	¾	2½
24E	24	218.9	2'-0"	2'-10"	2'-0"	6	¾	2½
26E	26	258.0	2'-0"	2'-10"	2'-0"	6	¾	2½
28E	28	300.3	2'-0"	2'-10"	2'-0"	6	¾	2½
12F	12	57.4	2'-0"	2'-10"	2'-0"	6	¾	2½
14F	14	79.3	2'-0"	2'-10"	2'-0"	6	¾	2½
16F	16	104.7	2'-0"	2'-10"	2'-0"	6	¾	2½
18F	18	133.7	2'-0"	2'-10"	2'-0"	6	¾	2½
20F	20	166.2	2'-0"	2'-10"	2'-0"	6	¾	2½
22F	22	202.3	2'-0"	2'-10"	2'-0"	6	¾	2½
24F	24	241.9	2'-0"	2'-10"	2'-0"	6	¾	2½
26F	26	285.1	2'-0"	2'-10"	2'-0"	6	¾	2½
28F	28	331.8	2'-0"	2'-10"	2'-0"	6	¾	2½
14G	14	87.3	2'-0"	2'-10"	2'-0"	6	¾	2½
16G	16	115.1	2'-0"	2'-10"	2'-0"	6	¾	2½
18G	18	146.9	2'-0"	2'-10"	2'-0"	6	¾	2½
20G	20	182.5	2'-0"	2'-10"	2'-0"	6	¾	2½
22G	22	222.0	2'-0"	2'-10"	2'-0"	6	¾	2½
24G	24	265.3	2'-0"	2'-10"	2'-0"	6	¾	2½
26G	26	312.5	2'-0"	2'-10"	2'-0"	6	¾	2½
28G	28	363.6	2'-0"	2'-10"	2'-0"	6	¾	2½
16H	16	134.2	2'-0"	2'-10"	2'-0"	6	¾	2½
18H	18	171.2	2'-0"	2'-10"	2'-0"	6	¾	2½
20H	20	212.6	2'-0"	2'-10"	2'-0"	6	¾	2½
22H	22	258.5	2'-0"	2'-10"	2'-0"	6	¾	2½
24H	24	308.9	2'-0"	2'-10"	2'-0"	6	¾	2½
26H	26	363.8	2'-0"	2'-10"	2'-0"	6	¾	2½
28H	28	423.2	2'-0"	2'-10"	2'-0"	6	¾	2½
16I	16	157.1	2'-0"	2'-10"	2'-0"	6	¾	2½
18I	18	200.1	2'-0"	2'-10"	2'-0"	6	¾	2½
20I	20	248.3	2'-0"	2'-10"	2'-0"	6	¾	2½
22I	22	301.8	2'-0"	2'-10"	2'-0"	6	¾	2½
24I	24	360.4	2'-0"	2'-10"	2'-0"	6	¾	2½
26I	26	424.3	2'-0"	2'-10"	2'-0"	6	¾	2½
28I	28	493.3	2'-0"	2'-10"	2'-0"	6	¾	2½
20J	20	319.5	2'-0"	2'-10"	2'-0"	6	¾	2½
22J	22	388.0	2'-0"	2'-10"	2'-0"	6	¾	2½
24J	24	463.2	2'-0"	2'-10"	2'-0"	6	¾	2½
26J	26	544.9	2'-0"	2'-10"	2'-0"	6	¾	2½
28J	28	633.4	2'-0"	2'-10"	2'-0"	6	¾	2½

V-BEAM DESIGN

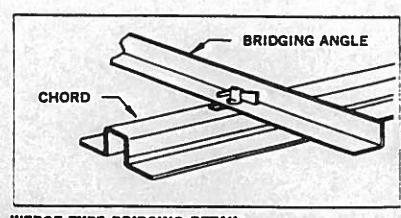
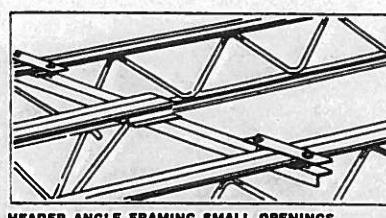
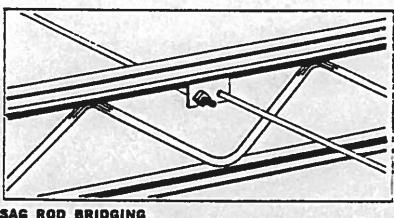
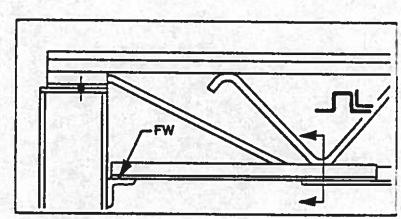
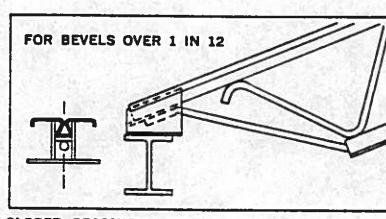
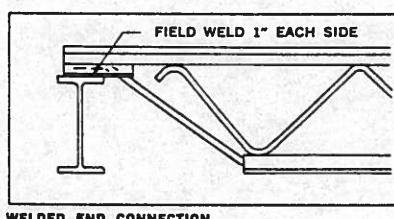
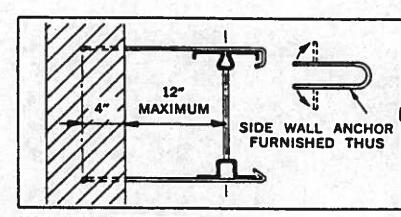
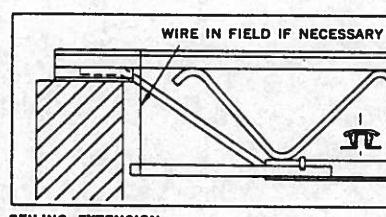
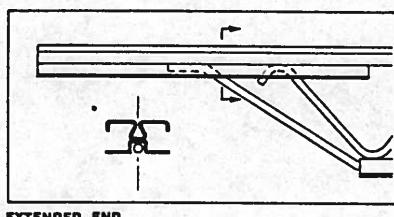
With this, the latest in the series of V-BEAMS, Macomber continues its leadership in the steel building field. This member embodies the experience gained since 1950 when Macomber introduced the original V-BEAM to the market. Development of the V-BEAM is based on information from the great number of tests performed on full sized members, since that date.

The V-BEAM top chord, consisting of the V-Section, has been in constant use on all Macomber products since 1946. Test data and engineering information on this section support its usage. The B-Section making up the bottom chord has been in use since 1960 in the Macomber "S" series joists, and more recently in the J & H series joists. The information gathered justifies its use in the V-BEAM.

The components of the V-BEAM — chords, bearing plates, end bars and web system — lend themselves admirably to modern fabricating standards and Macomber's up-to-date fabricating facilities.

In preparing the load tables, it was recognized that under some conditions, the deflection of a member governs its design. To simplify the selection of V-BEAMS under these conditions, additional loads, in color, have been incorporated listing the live load carrying capacities for each joist size and span, for limiting deflections of 1/360 and 1/240 of the span.

V-BEAM CONSTRUCTION DETAILS



MACOMBER V-BEAM LOAD TABLE (Continued)

Allowable Total Loads in Pounds per Lineal Foot. Factor of Safety = 1.65.

WHERE LIVE LOAD IS NOT INDICATED, LIVE LOAD CAPACITY EQUALS TOTAL LOAD CAPACITY.
CARRYING CAPACITIES TO RIGHT OF HEAVY LINE ARE GOVERNED BY RESISTING MOMENT OF CHORDS.
CARRYING CAPACITIES TO LEFT OF HEAVY LINE ARE GOVERNED BY THE END REACTION.

SPANS TO THE RIGHT OF THE DASHED LINES ARE TO BE USED FOR ROOF CONSTRUCTION ONLY.

MACOMBER V-BEAM LOAD TABLE

Allowable Total Loads in Pounds per Lineal Foot. Factor of Safety = 1.65.

Size	Nom. Depth (In.)	Resist. Moment (In.-K.)	Max. End Reaction (Lbs.)	CLEAR SPAN IN FEET																								Size						
				8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32						
8A	8	76.36	1630	407	362	326	296	271	250	232	217	198	84															8A						
						259	200	157	126	102																								
8B	8	93.21	2245	561	498	449	408	374	345	317	276	242																	8B					
						426	320	246	193	155	126	104																						
10A	10	97.63	1943	485	431	388	353	323	299	277	259	242	225	200	180	162													10A					
									256	205	167	137	114	96	82	70																		
10B	10	119.09	2984						497	459	405	352	310	274	245	219	198													10B				
									314	252	205	168	140	118	100	86																		
10C	10	147.81	2917						486	448	416	389	364	340	304	272	246													10C				
									392	314	255	210	175	148	125	107																		
10D	10	184.84	3614						602	556	516	481	451	425	380	341	308													10D				
									490	392	319	263	219	184	157	134																		
12A	12	118.96	1988	497	441	397	361	331	305	284	265	248	233	220	209	198	179	163	149	137									12A					
											246	202	169	142	121	103	89	77	68	60														
12B	12	144.90	3124						520	480	446	416	377	334	298	267	241	219	199	182	167									12B				
									465	372	303	249	205	175	149	127	110	96	84	74														
12C	12	179.87	3630						605	558	518	484	453	414	370	332	299	271	247	226	208									12C				
										464	377	311	259	218	185	159	137	119	104	92														
12D	12	224.48	3979						663	612	568	530	497	468	442	414	374	339	309	282	259									12D				
									470	387	323	272	231	198	171	149	130	114																
12E	12	266.60	3773						628	580	539	503	471	443	419	397	377	359	343	328	308									12E				
										459	383	322	274	231	198	171	149	130	114															
12F	12	293.69	4716																										12F					
14B	14	170.78	3699						528	493	444	393	351	315	284	258	235	215	197	182	168	156	145	134						14B				
									517	420	346	289	243	207	177	153	133	116	102	90	80	72	64											
14C	14	212.00	4367						623	582	545	489	436	391	353	320	292	267	245	226	209	193	180									14C		
										523	431	359	302	257	220	190	165	145	127	113	100	89	80											
14D	14	264.12	4023						574	536	502	473	447	423	402	383	363	332	305	281	260	241	224									14D		
										446	375	319	274	236	205	180	158	140	124	111	99													
14E	14	313.75	4850						606	570	538	510	485	461	432	395	363	334	309	285	266										14E			
										530	447	380	325	281	244	214	188	166	148	132	118													
14F	14	345.45	5266						658	619	585	554	526	501	475	435	399	368	340	315	293										14F			
									587	495	421	361	311	271	237	208	184	164	146	131														
14G	14	377.45	5141						642	604	571	541	514	489	467	447	428	402	372	345	320										14G			
										545	463	397	343	298	261	229	203	180	161	144														
16B	16	196.66	2699						337	317	299	284	269	257	245	234	224	209	193	179	167										16B			
										273	234	202	176	154	135	120	106	95	85															
16C	16	244.06	4074						509	479	452	428	406	368	336	307	282	260	240	223	207	193	180	169	158						16C			
									475	400	340	292	252	219	192	169	149	133	118	106	95	86	78	71										
16D	16	303.75	4810						601	565	534	506	481	458	418	382	351	324	299	277	258	240	225	210	197						16D			
										497	422	362	313	272	238	209	185	164	147	132	118	107	97	88										
16E	16	360.90	5290						661	622	587	556	529	503	480	454	417	384	355	330	306	286	267	250	234							16E		
										502	430	372	323	283	249	220	196	175	156	141	127	115	105											
16F	16	397.21	5163																													16F		
16G	16	433.75	5070																													16G		
16H	16	493.09	6292																														16H	

CAPACITY SHOWN ON FIRST LINE FOR EACH SIZE IS TOTAL LOAD CAPACITY.

CAPACITY SHOWN IN BLUE IS LIVE LOAD CAPACITY FOR DEFLECTION OF 1/360 OF SPAN.

CAPACITY SHOWN IN RED IS LIVE LOAD CAPACITY FOR DEFLECTION OF 1/240 OF SPAN.

SPANS TO THE RIGHT OF THE DASHED LINES ARE TO BE USED FOR ROOF CONSTRUCTION ONLY.

MACOMBER V-BEAM LOAD TABLE (Continued)

Allowable Total Loads in Pounds per Lineal Foot. Factor of Safety = 1.65.

Size	Nominal Depth (In.)	Resisting Moment (In.-K.)	Max. End Reaction (Lbs.)	CLEAR SPAN IN FEET																				
				20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36				
24D	24	462.30	5275	527	502	479	458	439	422	405 381	390 340	376 305	363 274	342 248	320 224	300 204	283 186 279	266 170 255	251 156 234	237 143 215				
24E	24	549.51	5694	569	542	517	495	474	455	438	421 405	406 363	392 326	379 295	367 267	355 243	336 221 332	316 202 304	299 185 278	282 170 256				
24F	24	604.24	6889						574	551	529 501	510 447	492 401	475 361	447 326	419 295	393 268	369 245 367	348 224 336	328 205 308	310 188 283			
24G	24	658.96	6784						565	542	521	502 490	484 440	467 396	452 357	437 324	424 294	403 268	380 245 368	358 225 338	338 207 310			
24H	24	748.60	8850										632 512	593 461	554 416	519 377	487 343	458 313	431 286 429	407 262 393	385 241 361			
24I	24	883.39	8599										614 597	593 538	573 486	554 440	537 400	521 365	505 333 500	480 306 455	454 281 421			
24J	24	1145.63	8768										626	604	584	565	548 514	531 469	515 429	501 393	487 361			
26D	26	501.93	6332	633	603	575	550	527	506 505	487 449	459 401	426 359	397 323	371 292	348 265	326 241	307 219	289 200	273 184	258 169 253				
26E	26	596.66	6130						510	490	471	454	437 428	422 385	408 348	395 315	383 286	365 261	344 239	324 219	306 201 302			
26F	26	656.00	6041						503	483	464	447	431	416	402 384	389 348	377 316	366 288	355 264	345 242	335 222 333			
26G	26	715.27	8190										630	606 578	585 518	564 466	529 421	496 382	465 347	437 316	412 289	389 265	367 243 365	
26H	26	812.48	8013										616	593	572	552 543	534 490	516 444	500 404	485 368	468 337	442 309	417 283	
26I	26	958.54	9430											673	650 633	628 572	608 518	589 471	571 429	552 393	521 360	493 331		
26J	26	1242.78	9303											664	641	620	600	581	563 552	547 504	531 462	516 425		
28D	28	541.57	7720											534 523	495 467	460 418	429 376	401 340	375 308	352 280	331 255	312 233	294 214	278
28E	28	643.81	7433											571	550	530 498	510 448	476 405	446 367	419 333	394 304	371 278	350 255	331 234
28F	28	707.75	7310											562	541	522	504 495	487 447	471 405	456 368	433 336	408 307	385 281	364 259
28G	28	771.57	7211											554	534	515	497	480	465 444	450 404	437 368	424 336	412 308	396 283
28H	28	876.36	8156											627	604	582	562	543	526 517	509 470	494 428	479 392	466 359	450 330
28I	28	1033.69	9220												658	635	614	594	576 548	558 499	542 457	526 419	512 385	
28J	28	1339.93	8950												639	617	596	577	559	542	526	511	497 494	

CAPACITY SHOWN ON FIRST LINE FOR EACH SIZE IS TOTAL LOAD CAPACITY.

CAPACITY SHOWN IN BLUE IS LIVE LOAD CAPACITY FOR DEFLECTION OF 1/360 OF SPAN.

CAPACITY SHOWN IN RED IS LIVE LOAD CAPACITY FOR DEFLECTION OF 1/240 OF SPAN.

SPANS TO THE RIGHT OF THE DASHED LINES ARE TO BE USED FOR ROOF CONSTRUCTION ONLY.

MACOMBER V-BEAM LOAD TABLE (Continued)

Allowable Total Loads in Pounds per Lineal Foot. Factor of Safety = 1.65.

CLEAR SPAN IN FEET																				Size
37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	
225 132 198	213 122 183																			24D
267 157 236	253 145 217	240 134 201	228 124 186																	24E
294 173 260	278 160 240	264 148 222	251 137 206																	24F
320 190 286	304 176 264	288 162 244	274 150 226	261 140 210	249 131 195	237 121 182	226 113 170	216 106 159	207 99 148	198 93 139	190 87 131									24G
364 222 333	345 205 307	328 189 284	311 175 263	296 163 244	282 151 227	269 141 212	257 132 198	246 123 185	235 115 173	225 108 162	216 101 152									24H
430 259 388	407 239 358	387 221 331	368 205 307	350 190 285	333 177 265	318 165 247	304 154 231	290 144 216	278 134 202	266 126 189	255 118 178									24I
473 333	461 307	449 284 426	438 263 395	427 244 367	417 227 341	407 212 318	394 198 297	377 185 277	360 173 259	345 162 243	331 152 225									24J
244 155 233	231 143 215																			26D
290 185 278	275 171 256	261 158 237	248 146 220																	26E
19 204 307	302 189 383	287 175 262	273 162 243																	26F
348 224 337	330 207 311	313 191 287	298 177 266	283 165 247	270 153 230	257 143 214	246 133 200	235 124 187	225 116 175	215 109 164	206 102 154	198 96 145	190 91 136							26G
395 261 392	375 241 362	356 223 335	338 207 310	322 192 288	307 178 268	292 166 249	279 155 233	267 145 218	255 136 204	245 127 191	235 119 179	225 112 168	216 105 158							26H
466 305 457	442 281 422	420 260 390	399 241 362	380 224 336	362 208 312	345 194 291	330 181 272	315 169 254	301 158 238	289 148 223	277 139 209	266 131 197	255 123 185	245 116 174	236 109 164					26I
502 391	489 361	477 334	465 310	453 287 431	443 267 401	432 249 374	422 232 349	409 217 326	391 203 305	375 191 286	359 179 269	345 168 253	331 158 238	318 149 224	306 141 211					26J
263 181	250 167																			28D
313 215	297 199	282 184 276	268 170 256																	28E
344 238	326 220	310 203 305	294 188 283																	28F
375 261	356 241	338 223 334	321 206 310	305 192 288	291 178 268	278 166 249	265 155 233	254 145 217	243 136 204	232 127 191	223 119 179	214 112 168	205 105 158							28G
426 304	404 280	384 259 361	365 240 335	347 223 312	331 208 290	315 193 271	301 180 253	288 169 237	276 158 237	264 148 222	253 139 209	243 131 196	233 123 184							28H
498 354	477 327	453 302 421	430 280 391	409 260 363	390 242 336	372 225 316	355 210 295	340 197 276	325 184 259	311 173 243	299 162 229	287 152 229	275 143 203	264 135 203	254 127 191	245 120 181	236 114 171	227 107 161	219 102 153	28I
483 455	471 420	458 388 360	447 360 334	436 334	426 311	416 290	406 270	397 253 379	389 236 355	380 222 333	372 208 312	365 196 294	357 184 276	343 173 260	330 164 246	318 154 232	306 146 219	295 138 207	284 131 197	28J

WHERE LIVE LOAD IS NOT INDICATED, LIVE LOAD CAPACITY EQUALS TOTAL LOAD CAPACITY.
CARRYING CAPACITIES TO RIGHT OF HEAVY LINE ARE GOVERNED BY RESISTING MOMENT OF CHORDS.

CARRYING CAPACITIES TO LEFT OF HEAVY LINE ARE GOVERNED BY THE END REACTION.

SPANS TO THE RIGHT OF THE DASHED LINES ARE TO BE USED FOR ROOF CONSTRUCTION ONLY.