

108. Because required shear/moments are at face of elements, simplified shear-moment diagrams can be used.

$$V_1 = 0 - 10 = -10 \text{ kips}$$

$$V_2 = -10 + 12.8 - 1.5 = 1.3 \text{ kips}$$

$$V_{ab} = 1.3 \text{ kips}$$

$$V_3 = 1.3 - 1.0 = 0.3 \text{ kip}$$

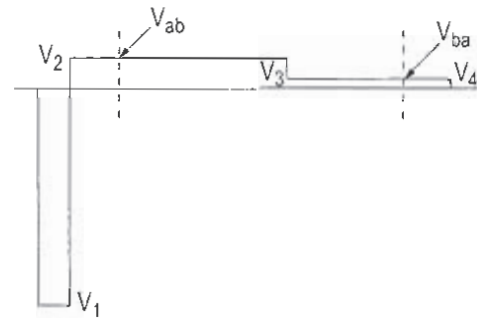
$$V_{ba} = 0.3 \text{ kip}$$

$$V_4 = 0.3 - 17 - 2 + 18.7 = 0.0 \text{ kip}$$

Calculate moments from shear diagram:

$$M_{ab} = (1.3 \text{ kips})(2 \text{ ft})(-10 \text{ kips})(1.5 \text{ ft}) = -12.4 \text{ ft-kips}$$

$$M_{ba} = 0 - (0.3 \text{ kip})(2 \text{ ft}) = -0.6 \text{ ft-kip}$$



SHEAR DIAGRAM

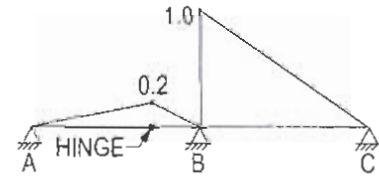
THE CORRECT ANSWER IS: (D)

109. Influence line for shear at right side of B:

$$V_{B_R} \text{ from uniform load} = \text{area under influence line} \times w$$

$$V_{B_R} \text{ from point load} = \text{maximum value of influence line} \times P$$

$$V_{B_R} = (1/2)(0.2)(60 \text{ ft})(1.0 \text{ klf}) + (1/2)(1.0)(60 \text{ ft})(1.0 \text{ klf}) + (1.0)(10 \text{ kips}) \\ = 46.0 \text{ kips}$$



THE CORRECT ANSWER IS: (D)

110. Distribution factor = $\frac{K}{\sum K \text{ of joint}}$

$$K = \frac{EI}{L} \quad \text{where } E \text{ is constant}$$

$$K_{AB} = \frac{200}{20} = 10$$

$$K_{BC} = \frac{300}{15} = 20$$

$$DF_{B-A} = \frac{10}{10 + 20} = 0.33$$

$$DF_{B-C} = \frac{20}{10 + 20} = 0.67$$

THE CORRECT ANSWER IS: (A)