

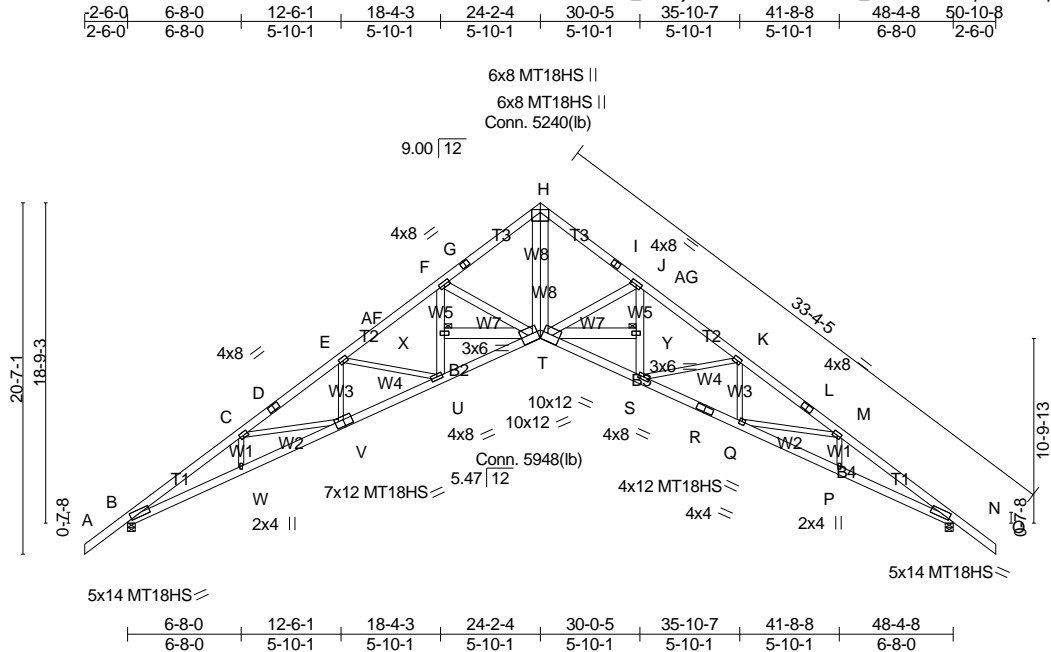
Job	Truss	Truss Type	Qty	Ply	
B151222	C2-Cond1	SCISSORS	16	1	

Structural Wood Corp., Waddington, NY 13694

Job Reference (optional)

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\*\*\* Design Problems \*\*\*  
REVIEW REQUIRED

Max Horizontal Deflection Exceeded In Span: Z-AC

TOTAL LOAD HORIZONTAL DEFLECTION LIMIT EXCEEDED. See note below

Plate Offsets (X,Y)-- [B:0-3-9,0-0-4], [N:0-3-9,0-0-4], [T:0-5-1,0-5-4], [T:0-5-1,0-5-4], [V:0-6-0,0-4-8]																			
<b>LOADING</b> (psf)		<b>SPACING-</b>		<b>2-0-0</b>		<b>CSI.</b>		<b>DEFL</b>		<b>in (loc)</b>		<b>l/defl</b>		<b>L/d</b>		<b>PLATES</b>		<b>GRIP</b>	
TCLL 34.8		Plate Grip DOL 1.15		TC 0.63		Vert(LL) -0.76		T		>766		240		MT20		197/144			
(Ground Snow=60.0)		Lumber DOL 1.15		BC 0.88		Vert(TL) -1.42		T-U		>409		180		MT18HS		197/144			
TCDL 10.0		Rep Stress Incr YES		WB 0.38		Horz(TL) -1.62		B		n/a		n/a							
BCLL 0.0		Code IRC2006/TPI2002		(Matrix-M)															
BCDL 10.0																Weight: 351 lb		FT = 20%	

LUMBER-		BRACING-		[MCT]
TOP CHORD	2x6 SPF 2100F 1.8E	TOP CHORD	Structural wood sheathing directly applied or 2-8-14 oc purlins.	
BOT CHORD	2x6 SPF 2100F 1.8E	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.	
WEBS	2x4 SPF 1650F 1.5E *Except*	JOINTS	1 Brace at Jt(s): X, Y	
W8,W5,W7: 2x6 SPF 2100F 1.8E, W6: 2x8 SP 2400F 2.0E				<div>MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</div>

REACTIONS. (lb/size) B=2875/0-5-8 (min. 0-3-3), N=2875/0-5-8 (min. 0-3-3)  
Max Horz N=-519(LC 8)  
Max UpliftB=-305(LC 10), N=-305(LC 10)  
Max GravB=2909(LC 15), N=2909(LC 16)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
TOP CHORD B-C=-8280/350, C-D=-7944/250, D-E=-7864/279, E-AF=-6925/114, F-AF=-6622/152, F-G=-5550/0, G-H=-5388/9, H-I=-5388/0, I-J=-5550/0, J-AG=-6623/152, K-AG=-6926/114, K-L=-7876/282, L-M=-7956/253, M-N=-8276/351  
BOT CHORD B-W=-90/6985, V-W=-101/7115, U-V=0/6942, T-U=0/5942, S-T=0/5943, R-S=0/6937, Q-R=0/6898, P-Q=-101/7113, N-P=-91/6982  
WEBS H-T=0/5958, E-V=-10/319, U-X=-90/737, F-X=-90/738, S-Y=-90/738, J-Y=-89/738, K-Q=-12/323, C-V=-257/198, E-U=-943/251, F-T=-1456/292, J-T=-1457/292, K-S=-936/253

- NOTES-
- 1) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=48ft; eave=6ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
  - 2) TCLL: ASCE 7-05; Pg=60.0 psf (ground snow); Ps=34.8 psf (roof snow); Category II; Exp C; Partially Exp.; Ct= 1
  - 3) Roof design snow load has been reduced to account for slope.
  - 4) Unbalanced snow loads have been considered for this design.
  - 5) This truss has been designed for greater of min roof live load of 16.0 psf or 1.00 times flat roof load of 42.0 psf on overhangs non-concurrent with other live loads.
  - 6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
  - 7) Special provisions shall be made by the building designer to allow for -0.87in of horizontal movement due to live load and -1.62in due to total load.
  - 8) Total load horizontal deflection of -1.62in exceeds 1.50in at joint B.
  - 9) All plates are MT20 plates unless otherwise indicated.

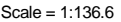
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Job	Truss	Truss Type	Qty	Ply	
B151222	C2-Cond1	SCISSORS	16	1	Job Reference (optional)

- NOTES-**
- 10) All plates are 4x6 MT20 unless otherwise indicated.
  - 11) All additional member connections shall be provided by others for forces as indicated.
  - 12) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 13) Bearing at joint(s) B, N considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
  - 14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 305 lb uplift at joint B and 305 lb uplift at joint N.
  - 15) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
  - 16) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

**LOAD CASE(S)** Standard

Structural Wood Corp., Waddington, NY 13694



Max Horizontal Deflection Exceeded In Span: Z-AC

TOTAL LOAD HORIZONTAL DEFLECTION LIMIT EXCEEDED. See note below

<b>LUMBER-</b> TOP CHORD 2x6 SPF 2100F 1.8E BOT CHORD 2x6 SPF 2100F 1.8E WEBS 2x4 SPF 1650F 1.5E *Except* W8,W5,W7: 2x6 SPF 2100F 1.8E, W6: 2x8 SP 2400F 2.0E	<b>BRACING-</b> TOP CHORD BOT CHORD JOINTS <div>           Structural wood sheathing directly applied or 4-9-5 oc purlins.            Rigid ceiling directly applied or 6-0-0 oc bracing.            1 Brace at Jt(s): X, Y  <div>MiTek recommends that Stabilizers and required cross bracing</div> </div>
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**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**TOP CHORD**  
B-C=-3855/333, C-D=-2930/230, D-E=-2782/259, E-AF=-1761/94, F-AF=-1458/132,  
F-G=-292/136, G-H=-31/419, H-I=-31/419, I-J=-292/136, J-AG=-1459/131,  
K-AG=-1762/93, K-L=-2806/262, L-M=-2954/233, M-N=-3853/334

**BOT CHORD**  
B-W=-1959/483, V-W=-1454/497, U-V=-1927/177, T-U=-2511/438, S-T=-2511/438,  
R-S=-1888/175, Q-R=-1932/156, P-Q=-1455/496, N-P=-1959/482

**WEBS**  
H-T=-803/146, E-V=-11/414, U-X=-90/772, F-X=-90/772, S-Y=-90/773, J-Y=-90/772,  
K-Q=-13/427, C-V=-757/200, E-U=-1015/252, F-T=-1475/292, J-T=-1476/292,  
K-S=-1010/254, M-Q=-736/198

- 1) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=48ft; eave=6ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-05; Pg=60.0 psf (ground snow); Category II; Exp C; Partially Exp.; Ps= 1
- 3) Roof design snow load has been reduced to account for slope.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 16.0 psf or 1.00 times flat roof load of 42.0 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.
- 7) All plates are MT20 plates unless otherwise indicated.
- 8) All plates are 4x6 MT20 unless otherwise indicated.
- 9) All additional member connections shall be provided by others for forces as indicated.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

10) This truss has b  
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Job	Truss	Truss Type	Qty	Ply	
B151222	C2-Cond2	SCISSORS	16	1	Job Reference (optional)

- NOTES-**
- 11) Bearing at joint(s) B, N considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
  - 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 305 lb uplift at joint B and 305 lb uplift at joint N.
  - 13) Non Standard bearing condition. Review required.
  - 14) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
  - 15) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

**LOAD CASE(S)** Standard