

## 10.4.4 Lateral Resistance

### 10.4.4.1

The factored lateral strength resistance of a bolted connection,  $P_u$ ,  $Q_u$ , or  $N_u$ , varies directly with the number of shear planes and shall be greater than or equal to the effect of the factored loads:

• For parallel-to-grain loading

$$= \phi P_u n_s n_p l_e$$

• For perpendicular-to-grain loading

$$= \phi Q_u n_s n_p l_e$$

for loads at angle,  $\theta$ , to grain

$$P_u Q_u$$

$$P_u \sin^2 \theta + Q_u \cos^2 \theta$$

where

- $\phi$  = 0.7
- $P_u$  ( $K_u$ ,  $K_s$ ,  $K_t$ )
- lateral strength resistance for parallel to grain loading (Clause 10.4.4.2), N
- number of shear planes
- $l_e$ , mm
- factor for two to maximum 12 bolts in a row

$$0.33 \left( \frac{l_e}{d} \right)^{0.5} \left( \frac{s}{d} \right)^{0.2} N^{0.3}, \text{ but is not greater than 1.0}$$

where

- $l_e$  = member thickness (see Clause 10.4.2), mm
- $s$  = bolt spacing in the row, mm
- $d$  = bolt diameter, mm
- $N$  = number of bolts in a row

**Note:**  $l_e = 1.0$  in cases with only one bolt per row, and in all wood-to-concrete connections.

- factor for loaded end distance
- 0.75 for end distance of  $7d$ , or 1.0 for  $10d$  (for intermediate values, interpolate linearly)
- factor for number of rows
- 1.0 for 1 row, or for 1 bolt per row
- 0.8 for 2 rows (2 or more bolts in a row)
- 0.6 for 3 rows (2 or more bolts in a row)

$$Q_u (K_u, K_s, K_t)$$

lateral strength resistance for perpendicular to grain loading (Clause 10.4.4.2), N

### 10.4.2

Unit lateral strength resistances,  $p_u$  or  $q_u$ , (N per shear plane) shall be taken as the smallest value obtained from formulae (a) to (g) as follows:

Two-member connections, only formulae (a), (b), (d), (e), (f), and (g), are valid. For three member connections, only formulae (a), (c), (d), and (g) are valid.

$$F_t d' \frac{l_2}{l_1} \frac{l_2}{d}$$

$$F_t d' \frac{1}{2} \frac{l_2}{l_1} \frac{l_2}{d}$$

$$F_t d' \left[ \sqrt{\frac{1}{6} \frac{l_2}{(l_1 + l_2)} \frac{f_y}{f_1} + \frac{1}{5} \frac{l_1}{d}} \right]$$

$$F_t d' \left[ \sqrt{\frac{1}{6} \frac{f_2}{(f_1 + f_2)} \frac{f_y}{f_1} + \frac{1}{5} \frac{l_2}{d}} \right]$$

$$F_t d' \frac{1}{5} \left( \frac{l_1}{d} + \frac{f_2}{f_1} \frac{l_2}{d} \right)$$

$$F_t d' \left[ \frac{2}{3} \frac{f_2}{(f_1 + f_2)} \frac{f_y}{f_1} \right]$$

0.8  $F_t$

bolt diameter, mm

side member thickness, mm

embedding strength of main member, MPa

main member thickness, mm

embedding strength of side member, MPa

bolt yield strength, MPa

310 MPa for ASTM A 307 bolts

Wood member embedding strength:

63G (1–0.01d), for parallel-to-grain loading

27.4G (1–0.01d), for perpendicular-to-grain loading

mean relative density (Table A10.1)

