Shear in Reinforced Concrete Slabs under Concentrated Loads close to Supports

Eva Lantsoght



Overview

Introduction

- Overview of experiments
- Beams vs. slabs
- Modified Bond Model
- Code extension proposal
- Application to practice
- Conclusions





Motivation (1)

Bridges from 60s and 70s



Increased live loads



heavy and long truck (600 kN > perm. max = 50ton)

The Hague in 1959

End of service life + larger loads



Motivation (2)





Motivation (3)

- First checks since mid-2000s
 - 3715 structures to be studied
 - 600 slab bridges shear-critical
- But: checks according to design rules
- => Residual capacity???

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 Hidden reserves of the bearing capacity



Highways in the Netherlands

Project description (1)

- Capacity of existing bridges
 - TU Delft
 - Concrete Structures
 - Structural Mechanics
 - TNO
 - RWS

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- Concrete Structures
 - Long-term tensile strength
 - Beam shear sustained loads
 - Continuous girders shear
 - Prestressed slabs punching
 - Slab bridges shear/punching





Concrete bridges



Shear Failure (1)



Shear failure of the de la Concorde bridge, Laval

Shear failures of bridges: rare but brittle failures

Shear Failure (2) Beam shear vs. Punching shear



Beam shear, one-way shear



Punching shear, two-way shear





amount of shear experiments done

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Shear Failure (3) Beam shear

- Since 1899 (Ritter)1955: collapse of warehouse
- Most experiments:
 - Beams

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- Heavily reinforced
- Small size
- Slender ($a/d \ge 2,5$)
- Basis for design codes



Influence of shear span

Shear Failure (4) Punching shear

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Categories of methods for punching shear



Slabs under concentrated loads (1)

- Transverse load redistribution
- Additional dimension in slabs
- Expected higher capacity than beams
- First experiments: Regan (1982)





Slabs under concentrated loads (2)



- Shear stress over effective width
- Fixed width, eg. 1 m
- Load spreading method

Slabs under concentrated loads (2)



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Goals

Assess shear capacity of slabs under concentrated loads
Determine effective width in shear



Experiments (1)



Size: $5m \times 2,5m$ (variable) $\times 0,3m =$ scale 1:2

Continuous support, Line supports

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Concentrated load: vary a/d and position along width

Experiments (2)

• 2nd series experimental work:

- Slabs under combined loading
- Line load

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- Preloading
- 50% of strength from slab strips
- Concentrated load until failure
- Conclusions from 1st series valid when combining loads?
- 26 experiments, 8 slabs
- Overall: 156 experiments, 38 slabs



Experiments (3)





Slabs vs. beams (1)



- Transverse load redistribution
- Geometry governing in slabs
- Location of load
 - result of different load-carrying paths
- Mid support vs end support
 - influence of transverse moment
- Wheel size

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• more 3D action

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Slabs vs. beams (2)





Modified Bond Model (1)

- Based on Bond Model (Alexander and Simmonds, 1990)
- For slabs with concentrated load in middle





Modified Bond Model (2)





Modified Bond Model (3)

- Adapted for slabs with concentrated load close to support
- Geometry is governing as in experiments

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- Determine factor that reduces capacity of "radial" strip
- Maximum load: based on sum capacity of 4 strips





Modified Bond Model (4)

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Modified Bond Model (5)





Modified Bond Model (6)

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Modified Bond Model (7)





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Modified Bond Model (8)





Modified Bond Model (9)



Experiments vs Eurocode shear

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Experiments vs Modified Bond Model

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Code extension proposal Limit State function

 $P_{f} = P \quad R < R_{d}$

Experiment vs. Design value

- Experiment
 - mean values
 - Test/Predicted ratio



Design value

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characteristic values

Reliability analysis based on load and resistance

Code extension proposal Random variables

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Code extension proposal Slabs subjected to Wheel Loads

$$V_{Rd,c,prop} = C_{Rd,c} k \ 100 \rho_l f_{ck}^{-1/3} b_{eff,red} d_l \left(1.9 - \frac{f_{ck}}{225} \right)$$
$$b_{eff,red} = \left(0.52 \frac{l_{sup}}{b} + 0.48 \right) b_{eff}$$

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- Enhancement factor depends on f_{ck}
 - Experiments: f_{ck} not as in shear formula
- Effect of reduced support width
- Proposal for $a_{\nu} \leq 2.5 d_{\mu}$



Application to practice (1)

• Evaluating existing solid slab bridges:

- NEN-EN 1992-1-1:2005
- 25% reduction of contribution concentrated load close to support
- $\beta = a_v/2d$
- Combined: $\beta_{new} = a_v/2,5d$
- Effective width: French method and minimum 4*d*







Application to practice (3)



Checks at indicated sections

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9 existing Dutch solid slab bridges

Application to practice (4)

• Shear **stresses**: influence of recommendations

- QS-EC2: wheel loads at $a_v = 2,5d_1$
- QS-VBC: wheel loads at $a_v = d_l$
- QS-EC2 18% reduction in loads

• Shear **capacity**:

- QS-EC2: *ν_{Rd,c}* ~ *ρ*, *d*
- low reinforcement + deep section = small shear capacity
- QS-VBC: *τ*₁ ~ *f*_{ck} only

QS-EC2 improved selection ability



Impact on Sustainability

Example replacement of 3-span slab bridge (deck only)

- Economic cost: 500k 640k €
- Environmental cost: 136 ton CO₂
 - Blast furnace cement: 74 ton CO₂
 - Portland slag cement: 122 ton CO₂
- Social cost: case-dependent
 - can be 9 times economic cost
- Scope: 600 slab bridges







Summary & Conclusions

- Slabs under concentrated loads behave differently in shear than beams
- Beneficial effect of transverse load redistribution
- Modified Bond Model improvement as compared to Eurocode
- Code extension proposal for transverse load redistribution



 Application to practice: reduction in loads

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Contact:

Eva Lantsoght

E.O.L.Lantsoght@tudelft.nl

+31(0)152787449

