

Proj. No.: 13-120085-03

Engr: Chandra

Date: 5/29/2008

Checker:

Date:

32 - 2 3/4 dia Anchor A307 Type C with 28in
proj @ 5.625 deg (Typ) on 25.313 ft BCD

POS/TOG EL. 101.00'

GR. EL. 100.00'

2 # 3

Ties @

18"

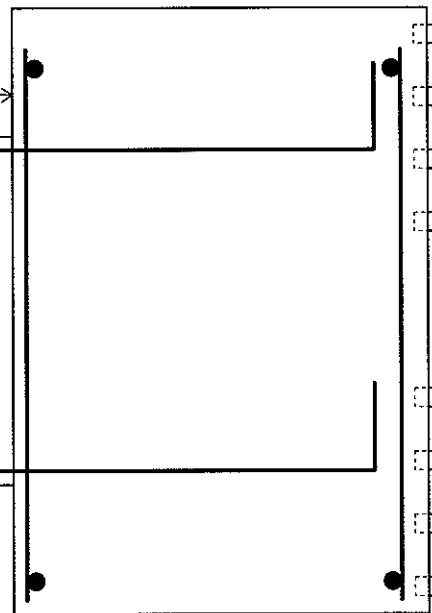
O.C.

V
A

3.25'

V
A

4.00'



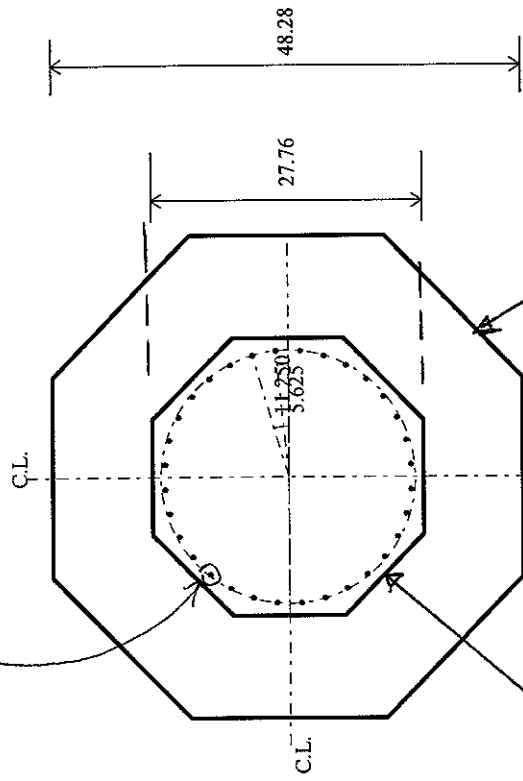
82 # 9 @ 7.00" O.C. (bot)

64 # 6 @ 9.00" O.C. (top)

82 # 9 @ 7.00" O.C. (bot)

64 # 6 @ 9.00" O.C. (top)

N



PILE CAP

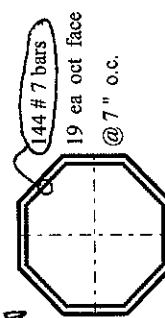
PIER

11.50'
20.00'

$f'_c = 5000 \text{ psi}$
 $f_y = 60,000 \text{ psi}$

(All dimensions in ft)

PLAN (1 REQD.)



SECT A-A

PIER

MATERIAL QUANTITIES	
CONCRETE	384.6 (CY)
FORMWORK	1030.9 (SF)
REINF. STEEL	35094.6 (LB)
EXCAVATION	832.7 (CY)

FOUNDATION PLAN & ELEV FOR

38-T-201

ELEVATION LOOKING NORTH

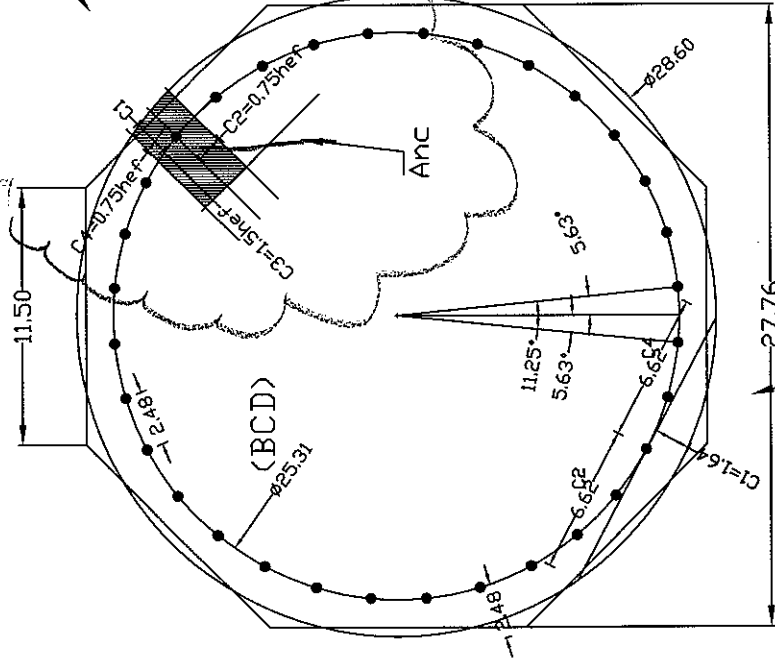
Signature

10F9

38-T-201

ANCHOR BOLT DESIGN

EDGE DIMENSIONS FOR TENSION
ASSUMED # OF ANCHORS IN TENSION = 1



EDGE DIMENSIONS FOR SHEAR
(PIPSTE-0251 /2003 p A-6

Pier

hef = embedment length = ~~36.3'~~
46"

$$C1 = (W * BCD) / 2 =$$

$$C2 = C4 = (PI) * BCD / n = 0$$

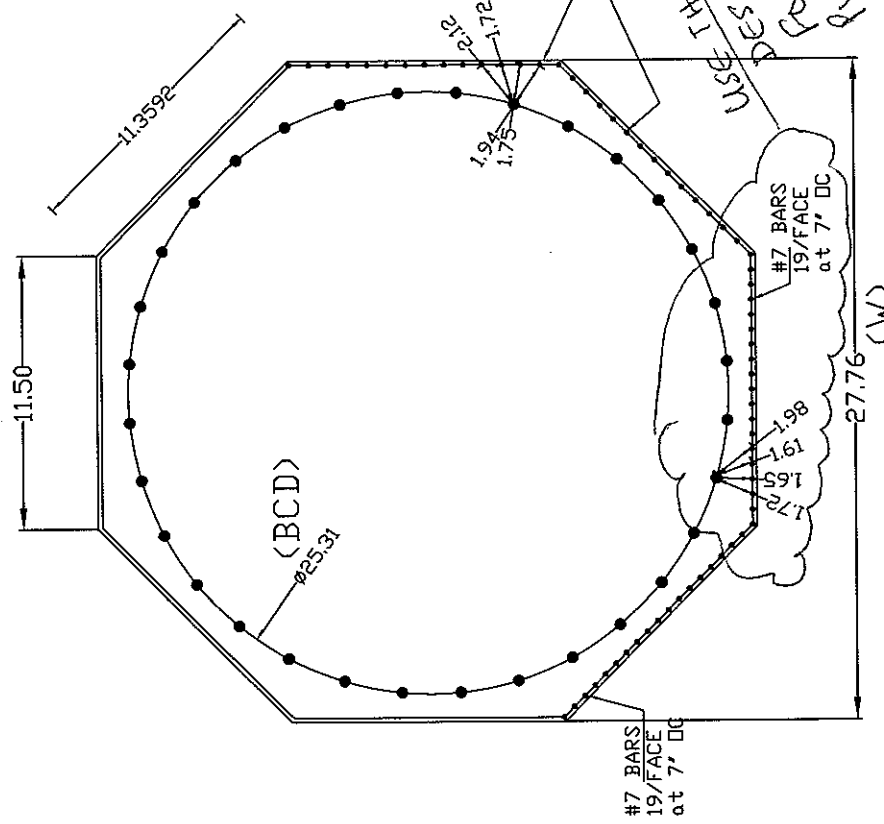
$$Anc = (1.5 * hef + C1) * (C2 + SE + C4) =$$

Conservatively use bolt spacings
of 8" or 6"

BY : CHANDRA 03-03-2008
CHK:

38-T-201

ANCHOR BOLT DESIGN



$F'_c = 5000 \text{ psi}$
 $f_y = 60 \text{ ksi}$
 ANCHOR BOLT: ASTM A307 TYPE -C

$f'_c = 5000 \text{ psi}$

BY : CHANDRA 03-03-2008

CHK:

3 OF 9

Handwritten signature/initials