

7 TANK ANCHORAGE (OVERTURNING STABILITY)

7.1 Design Code

EN14015:2004, Section 12

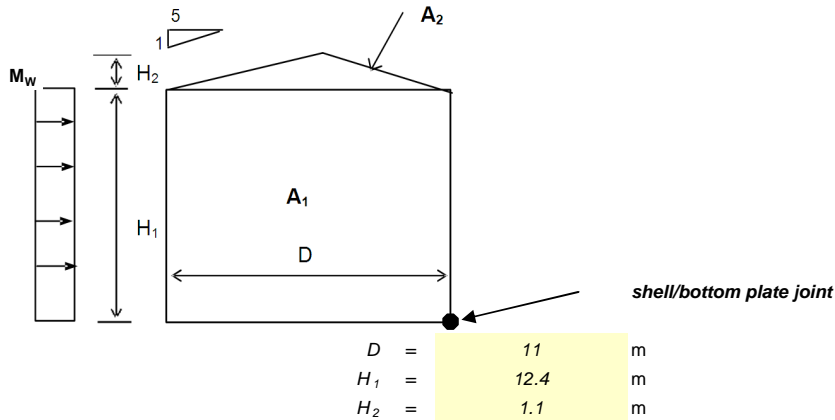
7.2 Fluid

Demineralised Water

7.3 Concept

7.3.1 Uplift due to Wind (Horizontal & Vertical)

7.3.1.1 Free Body Diagram of tank



7.3.1.2 Wind Pressure

Design windspeed, V	=	162	km/h
	=	45	m/s
Maximum windpressure, $P_{w,max}$	=	$\frac{1.48V^2}{190^2}$	
	=	1.08	kPa

7.3.1.3 Calculation

(a) Note:

For the shell, its projected area A_1	=	$H_1 D$	
	=	136.4	m^2

For the roof, its projected area A_2	=	$\frac{1}{2} H_2 D$	
	=	6.05	m^2

(b) Overturning Moment about Shell and Bottom Joint, M_W

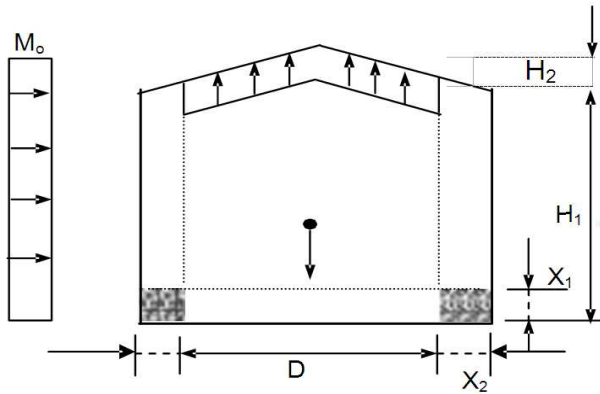
Moment due to wind on Shell, M_1	=	$A_1 P_w (H_1/2)$	
	=	909.9	kN m

Moment due to wind on Roof, M_2	=	$A_2 P_w C_d (H_1 + H_2/3)$	
	=	83.1	kN m

M_W	=	$M_1 + M_2$	
	=	<u>993.0</u>	kN m

7.3.2 Uplift due to Internal Pressure

7.3.2.1 Free Body Diagram of tank



7.3.2.2 Calculation

$$\begin{aligned} \text{Internal Cross Sectional Area, } A_{\text{int}} &= \frac{\pi D^2}{4} \text{ m}^2 \\ &= 95.0 \text{ m}^2 \\ \text{Internal Design Pressure, } P &= 50 \text{ kPa} \\ \text{Uplift Force, } F_{\text{uplift}} &= 4751.7 \text{ kN} \\ \text{Thus, the uplift moment, } M_{\text{pi}} &= \frac{1}{2} D F_{\text{uplift}} \\ &= \underline{\underline{26134.1}} \text{ kN m} \end{aligned}$$

7.3.3 Seismic Loading

From Annex G.2 of EN14015:2004,

The overturning moment applied to the bottom of the tank is calculated as:

$$\begin{aligned} M_{\text{SL}} &= 0.009803[Z(T_t X_s + T_r H_L + T_1 X_1) + Z' T_2 X_2] \\ M_{\text{SL}} &= \underline{\underline{3949.1}} \text{ kN m} \end{aligned}$$

where,

- Z = Seismic Factor (in accordance to UBC 1997)
- Z' = Seismic Factor due to sloshing of contents and soil conditions
- T_t = weight of tank shell, kg
- T_r = weight of tank roof, kg
- T_1 = effective weight of tank contents, kg
- T_2 = effective weight of tank contents when sloshing, kg
- X_s = height from shell bottom to centre of gravity of tank shell, m
- X_1 = height from shell bottom to centroid of seismic force applied to T_1 , m
- X_2 = height from shell bottom to centroid of seismic force applied to T_2 , m
- H_L = height of tank shell, m

7.3.4 Resistive Moments due to Nominal Shell and Roof Structure Supported by the Shell, M_{DL}

(excluding roof plates and structures attached to roof plates, and in corroded condition)

$$\begin{aligned} M_{\text{DL}} &= W_2 D/2 \\ M_{\text{DL}} &= \underline{\underline{1123.3}} \text{ kN m} \end{aligned}$$

where,

- W_2 = total weight of the nominal shell and roof structures

7.3.5 Resistive Moments due to Roof Plates and Appurtenances, M_{DLR}

(excluding roof structures supported by the shell, and in corroded condition)

$$M_{DLR} = W_3 D/2$$

$$M_{DLR} = \underline{\underline{292.1}} \text{ kN m}$$

where,

$$W_3 = \text{total weight of the roof's plates and appurtenances}$$

7.3.6 Resistive Moments due to Fluid Weight

$$W_f = 0.1 t_a \sqrt{F_{by} H W_s} \text{ , except that } w_f < 0.2 W_s \text{ HD}$$

$$= 41.8 \text{ kN/m of shell circumference}$$

except that $W_f < 25.6 \text{ kN/m}$

Hence, w_f is taken as 25.6 kN/m

where,

$$W_f = \text{moment of the force that resists uplift in the annular region (kN/m)}$$

$$t_a = \text{annular plate thickness (mm)}$$

$$F_{by} = \text{min specified yield strength of bottom annulus (MPa)}$$

$$H = \text{height of liquid till overflow (m)}$$

$$W_s = \text{maximum density of liquid, not less than } 1\text{g/cm}^3$$

$$M_f = \frac{1}{2} A_{int} w_f$$

$$= \underline{\underline{1215.0}} \text{ kN m}$$

7.3.7 Conclusion

$$M_w = 993.0 \text{ kN m}$$

$$M_{Pi} = 26134.1 \text{ kN m}$$

$$M_{SL} = 3949.1 \text{ kN m}$$

$$M_{DL} = 1123.3 \text{ kN m}$$

$$M_{DLR} = 292.1 \text{ kN m}$$

$$M_f = 1215.0 \text{ kN m}$$

In accordance with Clause 12.1, Unanchored Tanks shall satisfy these criteria:

- (i) Uplift of an empty tank due to internal design pressure is lower than the effective weight of the corroded roof, shell and permanent attachments.
- (ii) Uplift due to internal design pressure plus wind loads is lower than the effective weight of the corroded roof, shell and permanent attachments plus effective weight of products in tank
- (iii) Uplift of an empty tank due to wind loads is lower than the effective weight of the corroded roof, shell and permanent attachments.
- (iv) If required by Annex G (Overturning Moments due to Seismic Forces)

(i) $M_{Pi} < M_{DL} + M_{DLR}$	=	26134.1	<	1415.4
Safety factor	=	0.1		
(ii) $M_w + M_{Pi} < M_{DL} + M_{DLR} + M_f$	=	27127.1	<	2630.4
Safety factor	=	0.1		
(iii) $M_w < M_{DL} + M_{DLR}$	=	993.0	<	1415.4
Safety factor	=	1.4		
(iv) $M_{SL} + M_{Pi} < M_{DL} + M_{DLR} + M_f$	=	30083.2	>	2630.4
Safety factor	=	0.1		

**Criteria are not met, therefore,
Anchor bolts are required.**