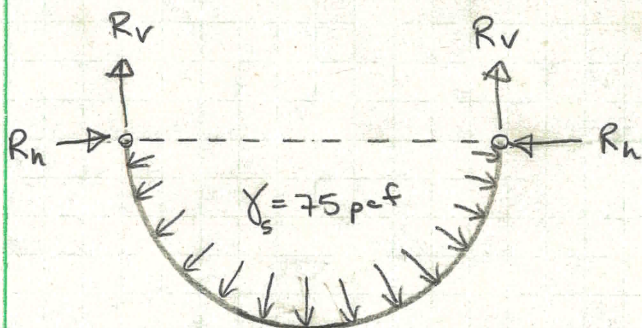


07/13/23

1

- COMPUTE FLUID PRESSURE ON 5 ft RADIUS FLUME
- COMPUTE TENSILE STRENGTH OF FLUME CORRUGATED STRUCTURAL PLATE.

- ASSUME FULL DEPTH SLURRY,  $\gamma_s = 75 \text{ lb/ft}^3$

CORRUGATED STRUCTURAL PLATE

5 ft RADIUS (CSP)  
 2" DEEP x 6" SPACING CORRUGATION  
 12 GAGE (PRELIM.  
 = 0.111 in ASSUMPTION)  
 CARBON STEEL:  $F_y = 33 \text{ ksi}$   
 $F_u = 45 \text{ ksi}$

- CENTROID,  $C = \left( \frac{4r}{3\pi}, \frac{4r}{3\pi} \right)$   
 $(2.1 \text{ ft}, 2.1 \text{ ft})$

- QUARTER-CIRCLE AREA,  $A$   
 $= \frac{\pi r^2}{4} = 19.63 \text{ ft}^2$

- FLUID WEIGHT,  $W_f$   
 $= \gamma_s A = 75 \text{ pcf} (19.63 \text{ ft}^2)$

$$W_f = 1472.25 \text{ plf}$$

- HORIZONTAL FLUID PRESSURE,  $F_h$

$$P_h = \gamma_s z = 75 \text{ pcf} (5 \text{ ft}) = 375 \text{ psf}$$

$$F_h = \frac{1}{2} \gamma_s z^2 = \frac{1}{2} (75 \text{ pcf}) (5 \text{ ft})^2$$

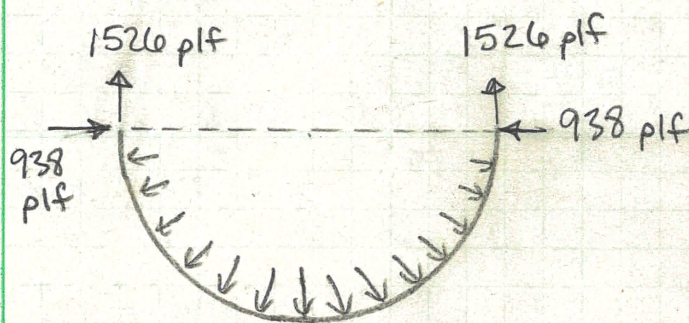
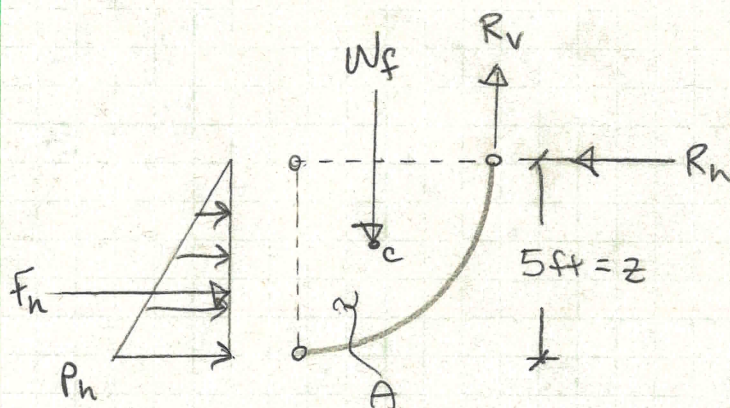
$$F_h = 937.5 \text{ plf}$$

- WEIGHT OF CSP (FROM TABLE)  
 [STRUCTURAL PLATE DESIGN GUIDE, 9th ED. BY CONTECH]

HALF-CIRCLE: 100 plf

QUARTER-CIRCLE: 53 plf

Self-weight



GROSS  
 • AREA OF CSP IN TENSION ( $\text{in}^2/\text{ft}$ )  
 $= 1.556 \text{ in}^2/\text{ft}$

TENSILE YIELDING:

$$\frac{P_h}{L} = \frac{A_g F_y}{L} = \frac{1.556 \text{ in}^2/\text{ft} (33 \text{ ksi})}{1.67}$$

$$P_{y/L} = 30.7 \text{ klf}$$



• NET AREA OF CSP IN TENSION ( $\text{in}^2/\text{ft}$ )

(2)  $3/4"$  BOLTS PER FT  $\rightarrow$  (2)  $7/8"$  HOLES PER FT

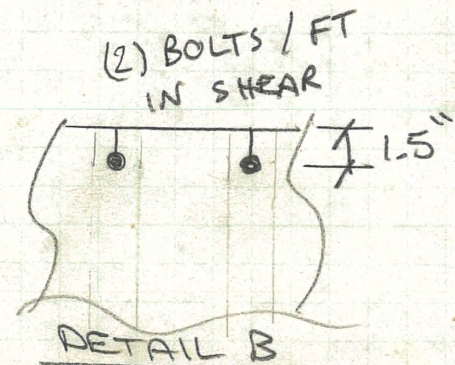
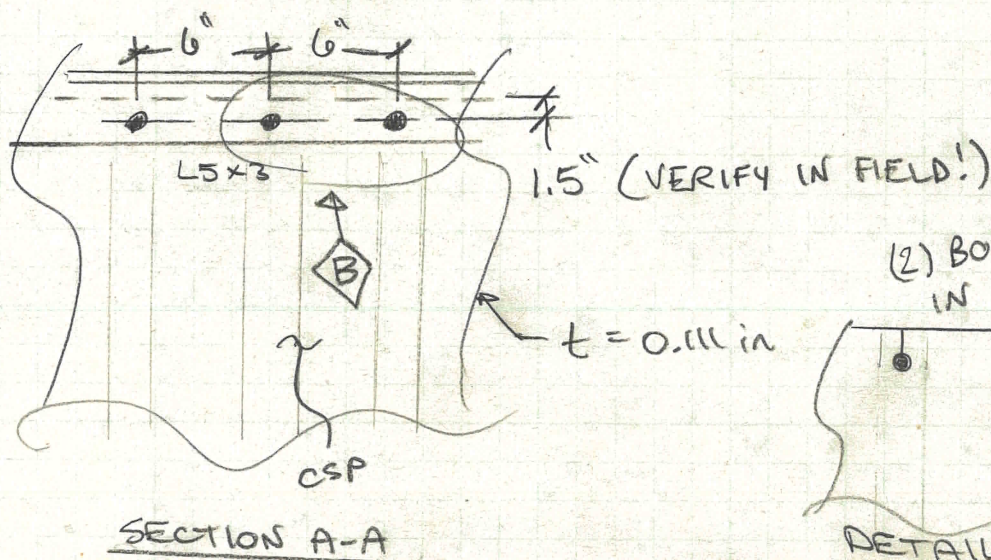
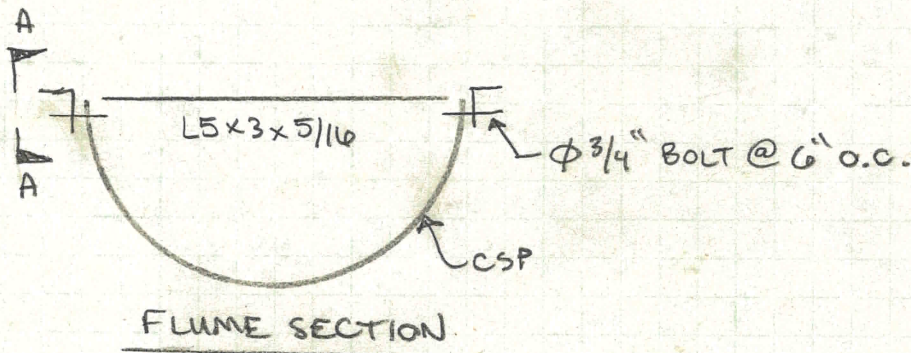
$$A_{\text{hole}} = 2(7/8") (0.111 \text{ in thick}) = 0.194 \text{ in}^2/\text{ft}$$

$$A_g - A_{\text{hole}} = 1.556 - 0.194 = A_{\text{net}} = 1.362 \text{ in}^2/\text{ft}$$

TENSILE RUPTURE:

$$\frac{P_n}{\Omega} = \frac{A_n F_u}{\Omega} = \frac{1.362 \text{ in}^2/\text{ft} (45 \text{ ksi})}{2.0} = \frac{P_n}{\Omega} = 30.6 \text{ klf}$$

CONTROLS



SHEAR YIELDING:

$$\frac{R_n}{\Omega} = \frac{0.6 F_y A_{gv}}{\Omega} = \frac{0.6 (33 \text{ ksi}) (0.111 \text{ in}) (1.5 \text{ in}) (2 \text{ qty})}{1.5} = 4.4 \text{ klf}$$

SHEAR RUPTURE:

$$\frac{R_n}{\Omega} = \frac{0.6 F_u A_{nv}}{\Omega} = \frac{0.6 (45 \text{ ksi}) (0.111 \text{ in}) (1.5 \text{ in} - 0.4375 \text{ in}) (2 \text{ qty})}{2.0}$$

$\downarrow$   
 $1/2$  of  $7/8"$  hole

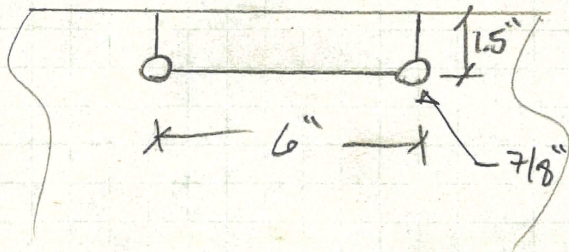
$$\frac{R_n}{\Omega} = 3.18 \text{ klf} \quad \text{CONTROLS}$$



BOLT SHEAR STRENGTH:

$\Phi 3/4"$  SINGLE SHEAR, THREADS INCLUDED:  $\frac{F_n}{\Sigma} = 11.9 \text{ k/bolt}$

(2) BOLTS / FT  $\rightarrow \frac{F_n}{\Sigma} = 23.8 \text{ klf}$

BLOCK SHEAR STRENGTH:

$$\text{SHEAR: } 2(1.5" - 0.4375") = 2.125"$$

$$\text{TENSION: } 6" - 0.875" = 5.125"$$

$$A_{nv} = 0.111" (2.125") = 0.236 \text{ in}^2$$

$$A_{nt} = 0.111" (5.125") = 0.569 \text{ in}^2$$

$$A_{gv} = 0.111" (3") = 0.333 \text{ in}^2$$

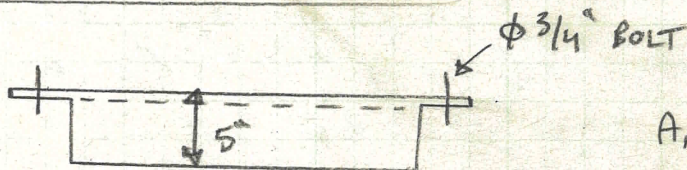
$$U_{bs} = 1.0$$

$$R_n = \min \left\{ \begin{array}{l} 0.6 F_u A_{nv} + U_{bs} F_u A_{nt} = 0.6(45 \text{ ksi})(0.236 \text{ in}^2) + 1(45 \text{ ksi})(0.569 \text{ in}^2) \\ 0.6 F_y A_{gv} + U_{bs} F_u A_{nt} = 0.6(33 \text{ ksi})(0.333 \text{ in}^2) + 1(45 \text{ ksi})(0.569 \text{ in}^2) \end{array} \right.$$

$$= \min \left\{ \begin{array}{l} 32 \text{ klf} \leftarrow \text{CONTROLS} \\ 32.2 \text{ klf} \end{array} \right.$$

$$\frac{R_n}{\Sigma} = \frac{32 \text{ klf}}{2.0} \quad \frac{R_n}{\Sigma} = 16 \text{ klf}$$

★ SHEAR RUPTURE CONTROLS

L5x3x5/16 TIE ANGLE:

$$A_g = 2.41 \text{ in}^2 \quad F_y = 36 \text{ ksi}$$

$$A_n = (0.3125 \text{ in}) \times (3 \text{ in} - 0.875 \text{ in}) = 0.664 \text{ in}^2$$

$$F_u = 58 \text{ ksi}$$

TENSILE YIELDING:

$$\frac{P_n}{\Sigma} = \frac{F_y A_g}{\Sigma} = \frac{36 \text{ ksi} (2.41 \text{ in}^2)}{1.67} = 52 \text{ k}$$

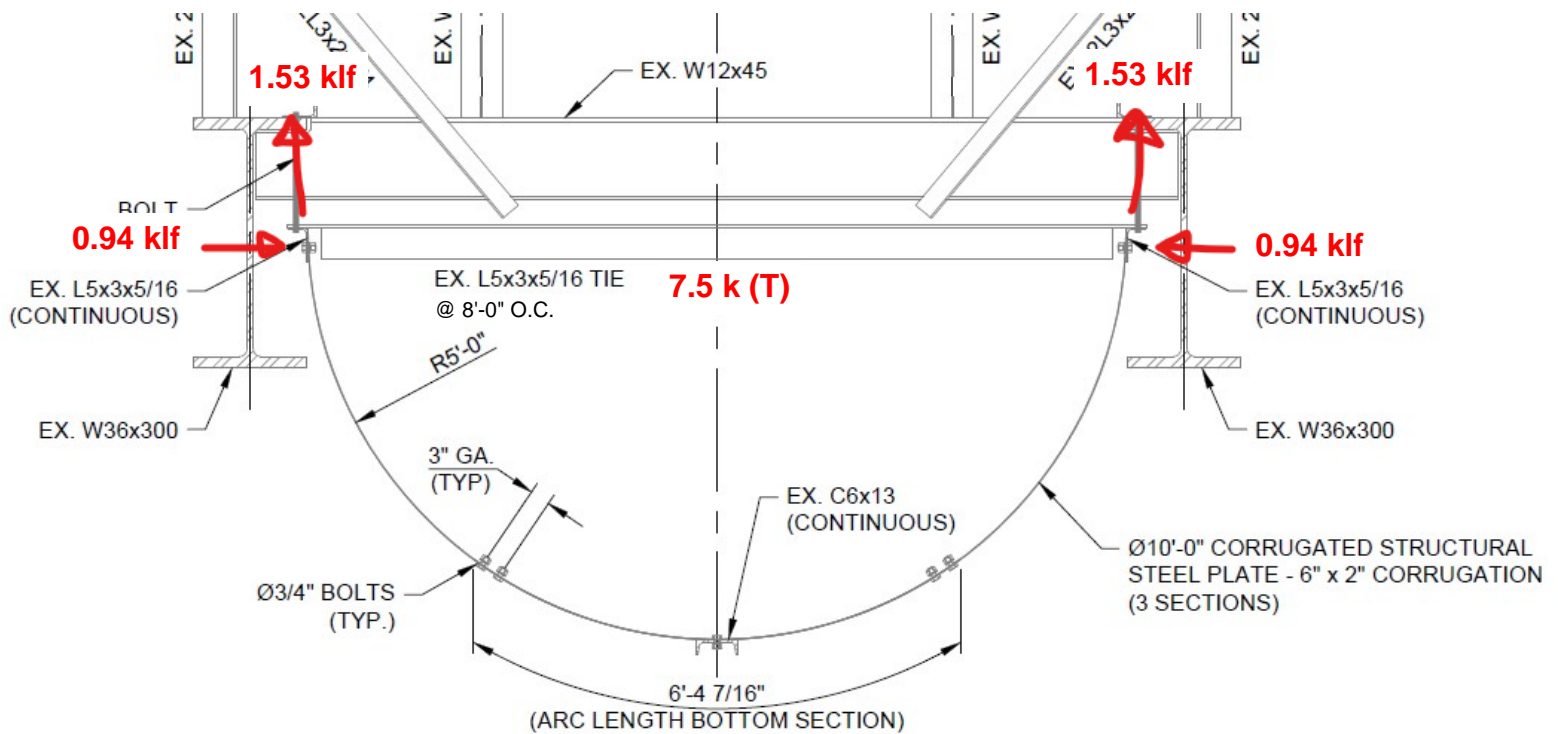
TENSILE RUPTURE:

$$\frac{P_n}{\Sigma} = \frac{F_u A_n}{\Sigma} = \frac{58 \text{ ksi} (0.664 \text{ in}^2)}{2.0} = 19.3 \text{ k} \leftarrow \text{CONTROLS}$$

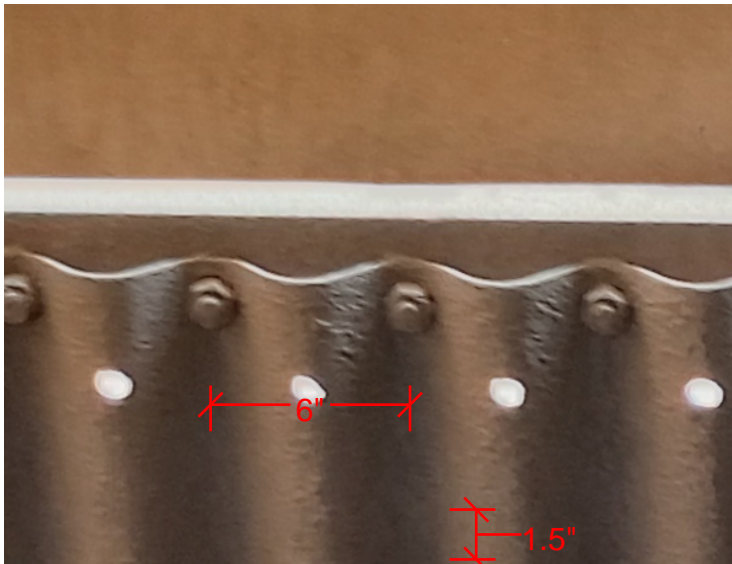
LOAD ON ANGLE:  $938 \text{ plf} (8 \text{ ft spacing}) = 7.5 \text{ k} < 19.3 \text{ k} \text{ OK}$

VERTICAL DEMAND:  $1.5 \text{ klf} < 31 \text{ klf}$  SHEAR RUPTURE OK





Top connection (flume to continuous angle)  
- shear rupture:



#### Section Properties

Gage	Thickness (Inches)	6" x 2" Corrugations			
		Area of Section $A_s$ Sq. In./Ft.	Radius of Gyration $r$ (Inches)	Section Modulus $S$ In. <sup>3</sup> /In.	Moment of Inertia $I$ In. <sup>4</sup> /In.
12	0.111	1.556	0.682	0.0574	0.0604
10	0.140	2.003	0.684	0.0733	0.0781
8	0.170	2.449	0.686	0.0888	0.0962
7	0.188	2.739	0.688	0.0989	0.1080
5	0.218	3.199	0.690	0.1147	0.1269
3	0.249	3.650	0.692	0.1302	0.1462
1	0.280	4.119	0.695	0.1458	0.1658
5/16	0.318	4.671	0.698	0.1640	0.1900
3/8	0.380	5.613	0.704	0.1950	0.2320

$f_u$ Minimum Tensile Strength (psi)	$f_y$ Minimum Yield Point (psi)	$E_m$ Modulus of Elasticity (psi)
45,000	33,000	$29 \times 10^6$