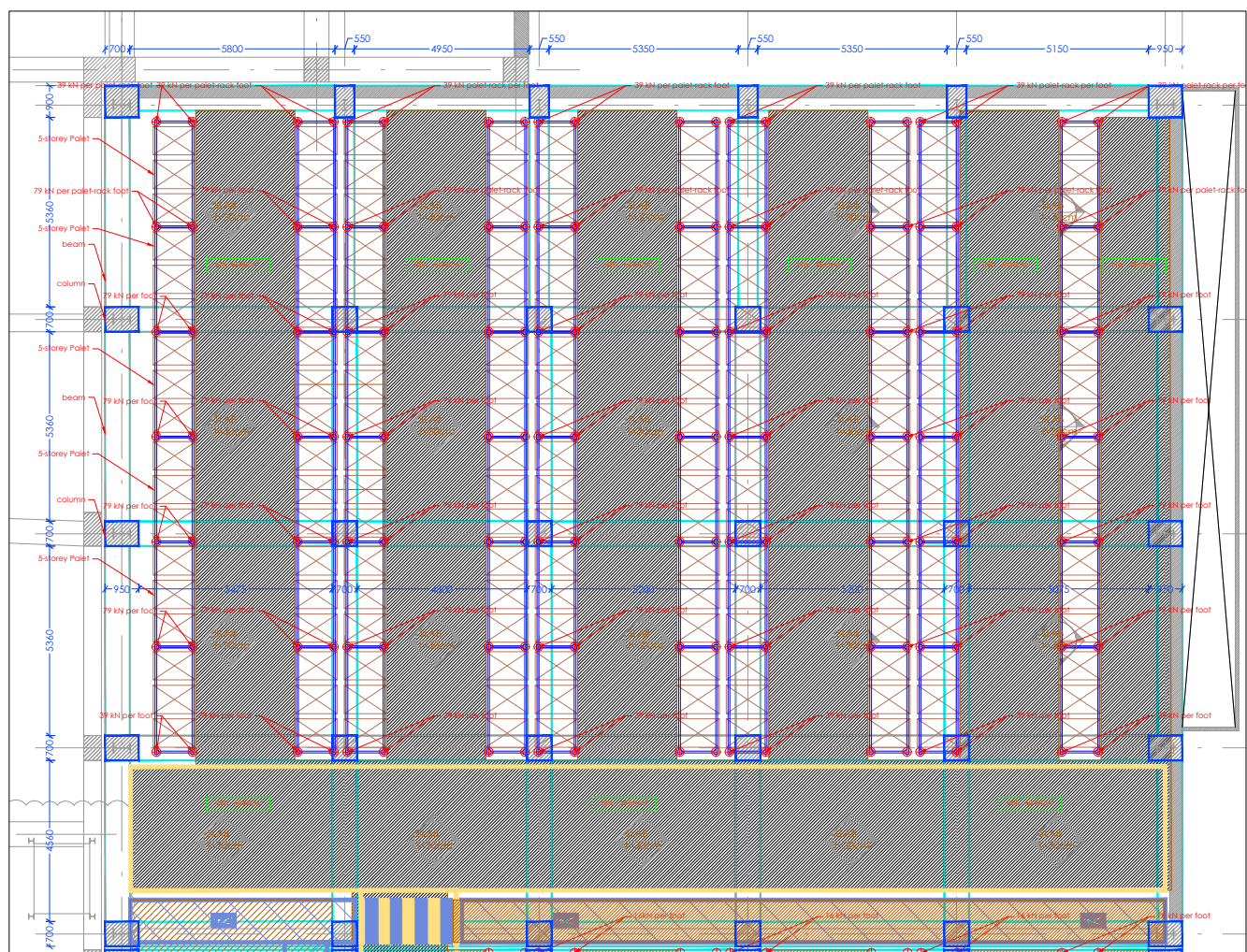
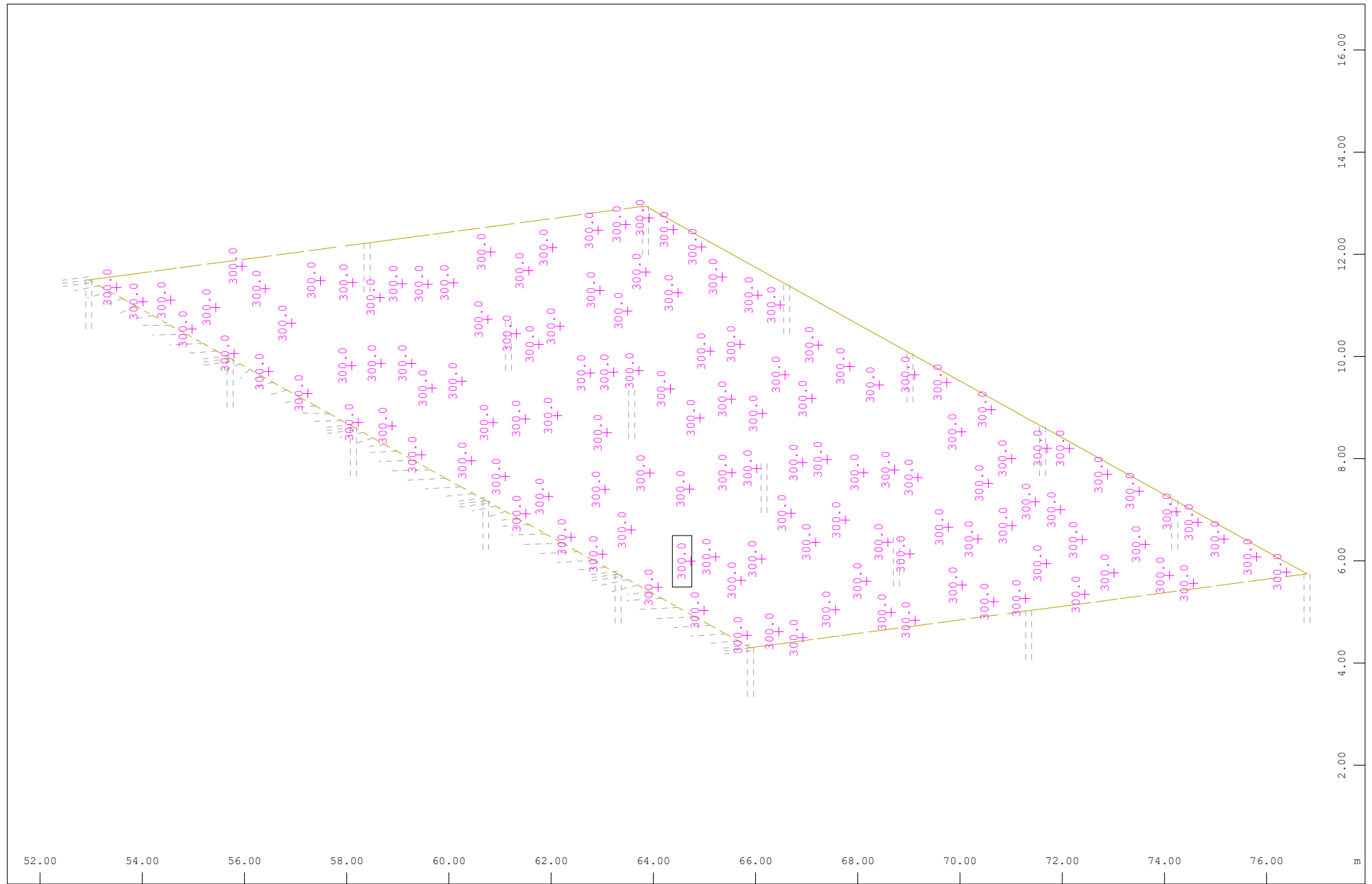


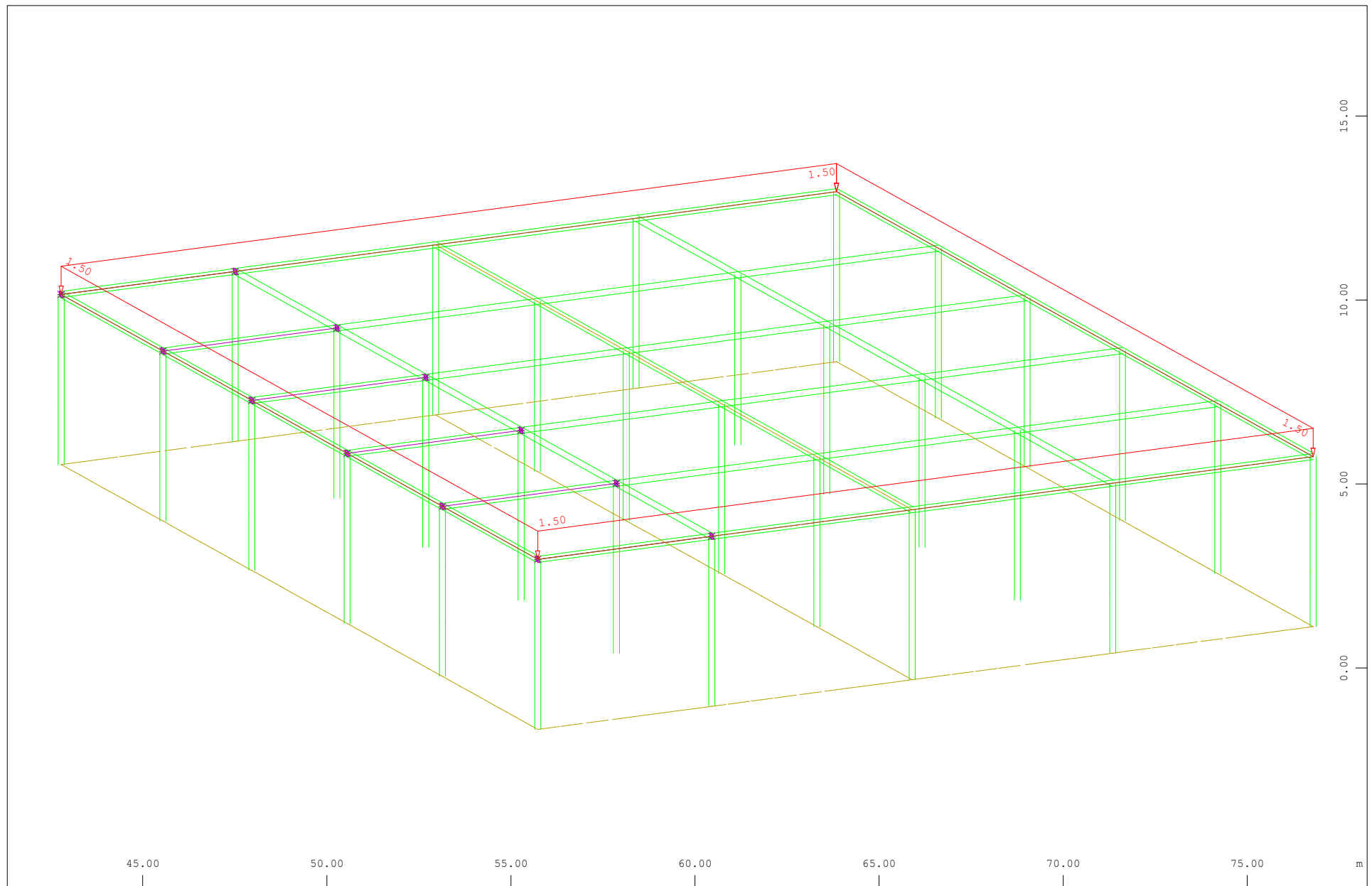




Z
 X Y

Sector of system Group 6
 Average plate thickness in Element in mm (Max=300.0)

M 1 : 105
 X * 0.502
 Y * 0.906
 Z * 0.962

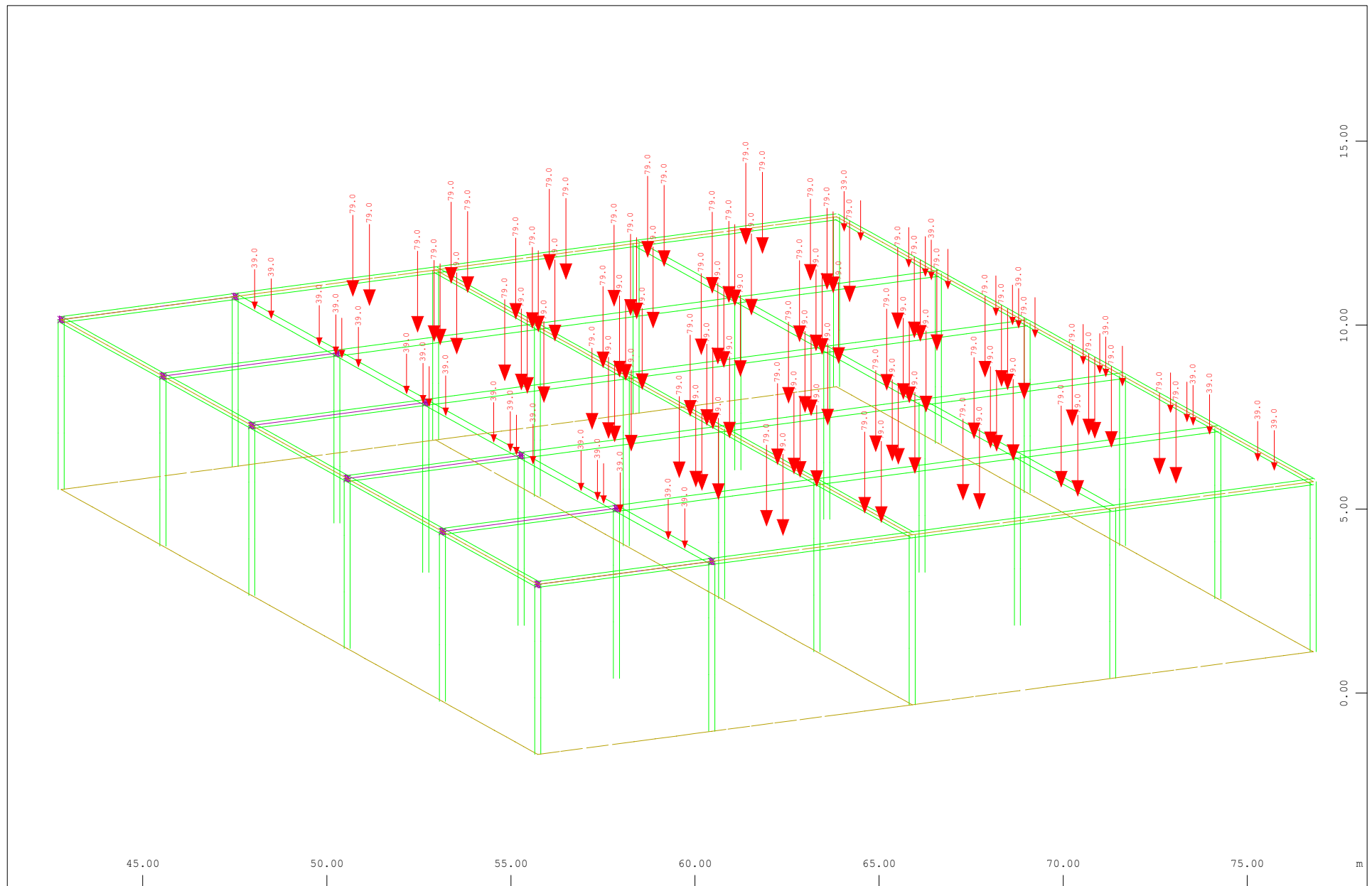


Z
Y
X

All loads, Loadcase 2 dead weight , (1 cm 3D = unit) Free area load (force) in global Z (Unit=2.75 kN/m2)

(Min=-1.50) (Max=-1.50)

M 1 : 146
X * 0.502
Y * 0.906
Z * 0.962

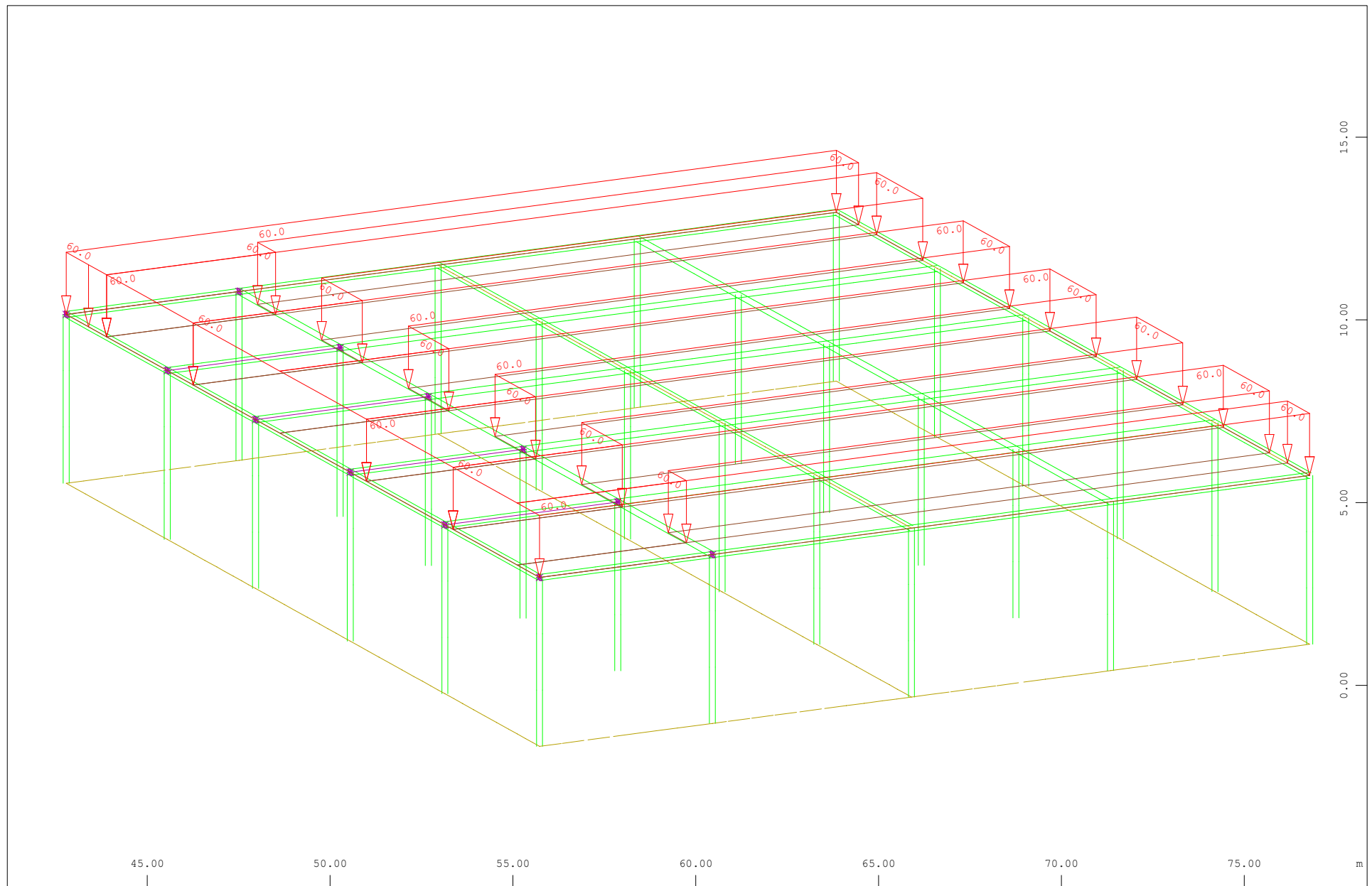


Z
Y

All loads, Loadcase 3 pallet general , (1 cm 3D = unit) Free single load (force) in global Z (Unit=50.0 kN
(Max=-39.0)

▶ (Min=-79.0)

M 1 : 146
X * 0.502
Y * 0.906
Z * 0.962

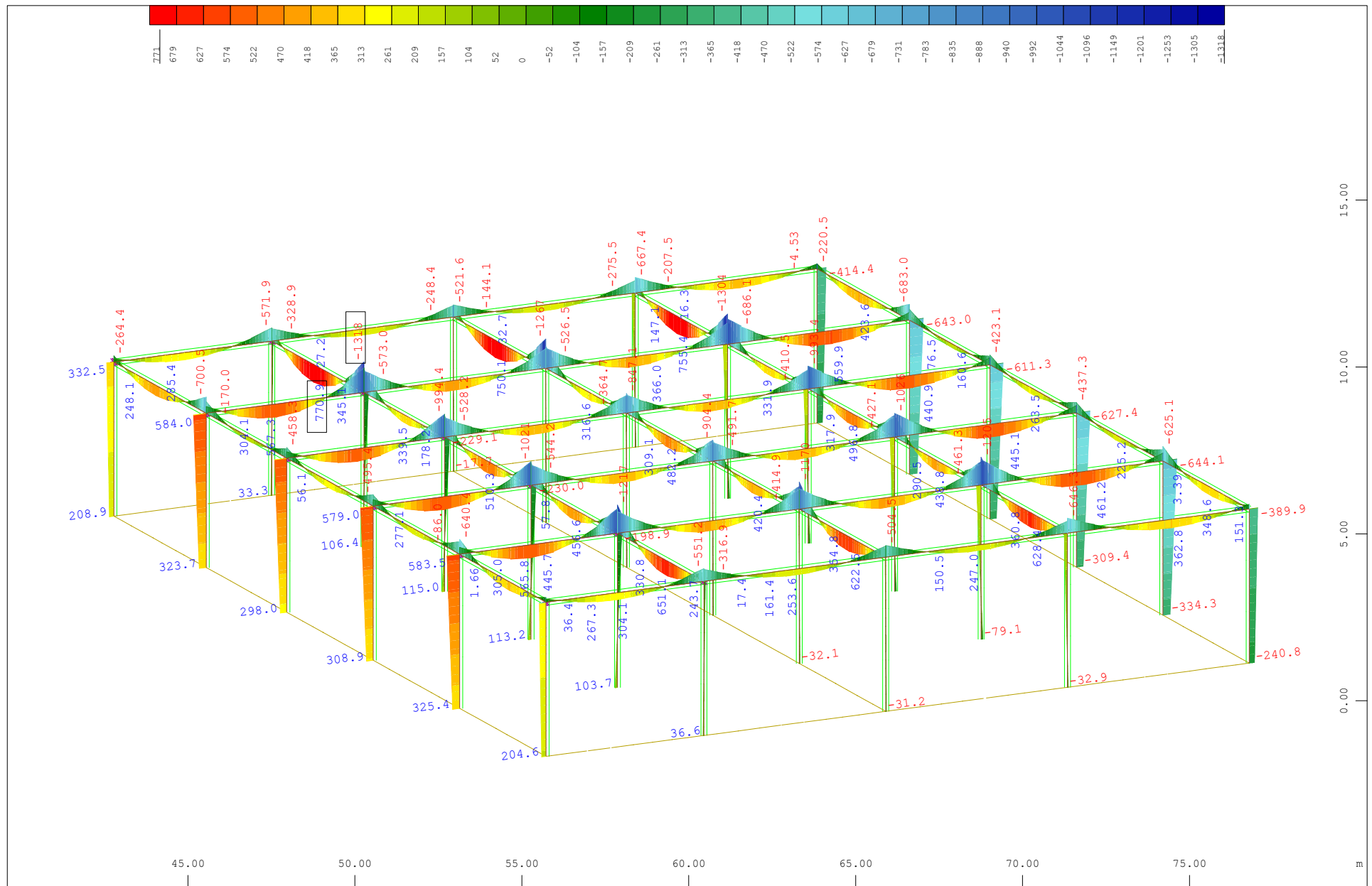


Z
Y

All loads, Loadcase 15 live load , (1 cm 3D = unit) Free area load (force) in global Z (Unit=50.0 kN/m2

➤ (Min=-60.0) (Max=-60.0)

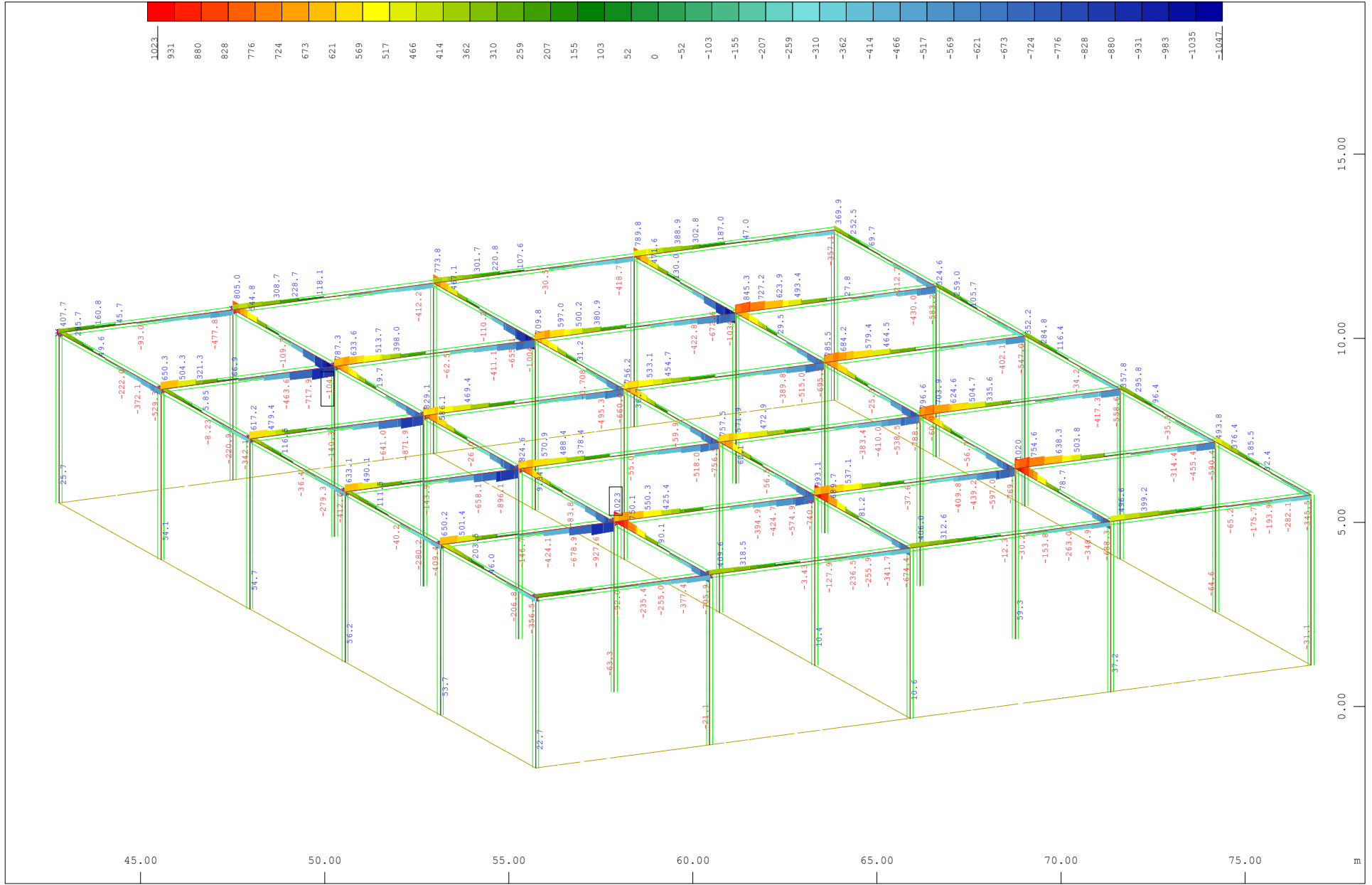
M 1 : 147
X * 0.502
Y * 0.906
Z * 0.962



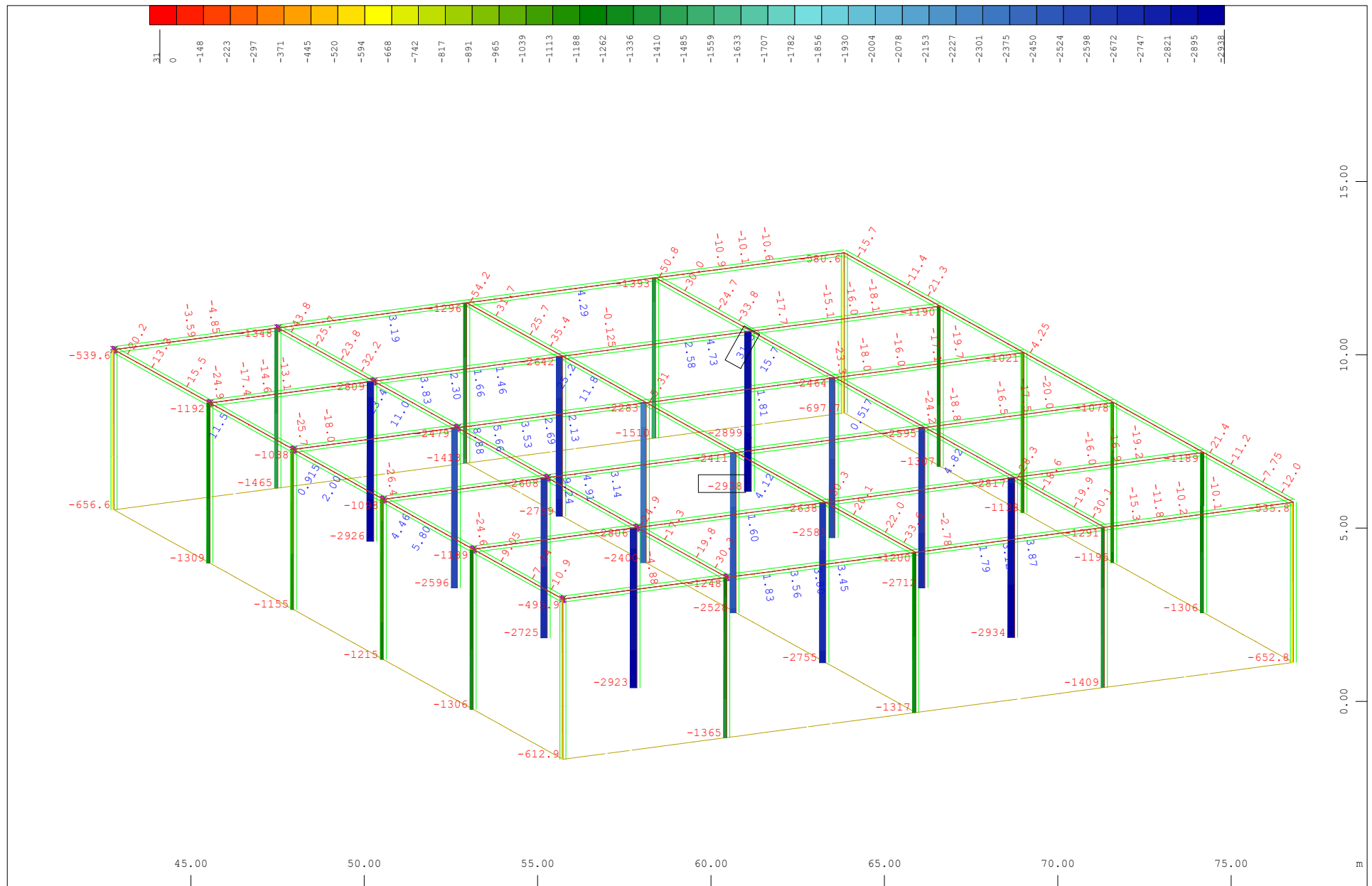
Z
X Y

Beam Elements , Bending moment M_y , nonlinear Loadcase 1002 1.35G+1.35G+Q_E+1.5Q , 1 cm 3D = 2242. kNm (Min=-1318.) (Max=770.9)

M 1 : 161
X * 0.502
Y * 0.906
Z * 0.962



M 1 : 146
X * 0.502
Y * 0.906
Z * 0.962



Z
Y

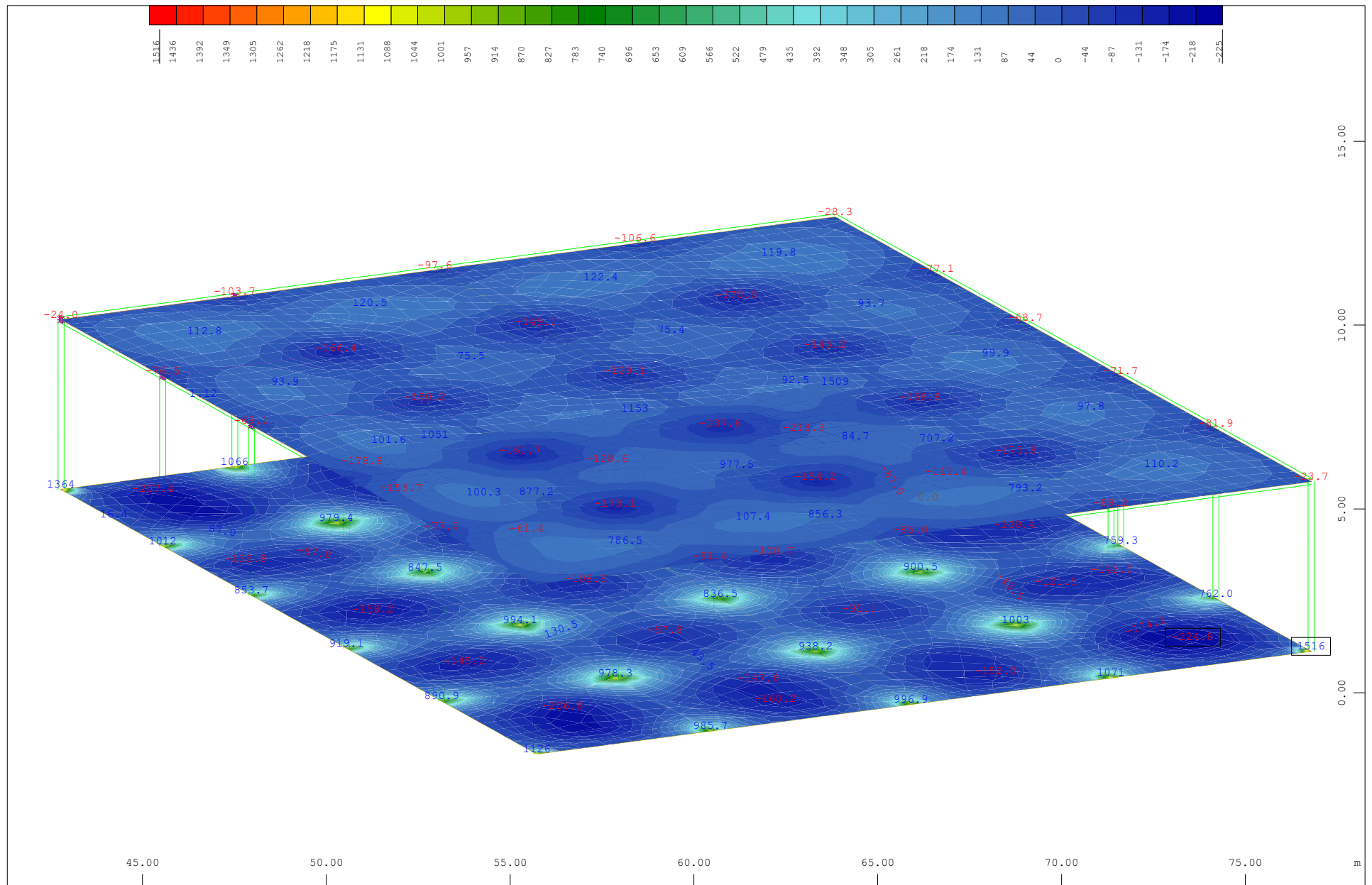
Beam Elements , Normal force Nx, nonlinear Loadcase 1003 1.35G+1.35Q+E+1.05Q , 1 cm 3D = 22859. kN (Min=-2938.) (Max=31.3)

M 1 : 155

X * 0.502

Y * 0.906

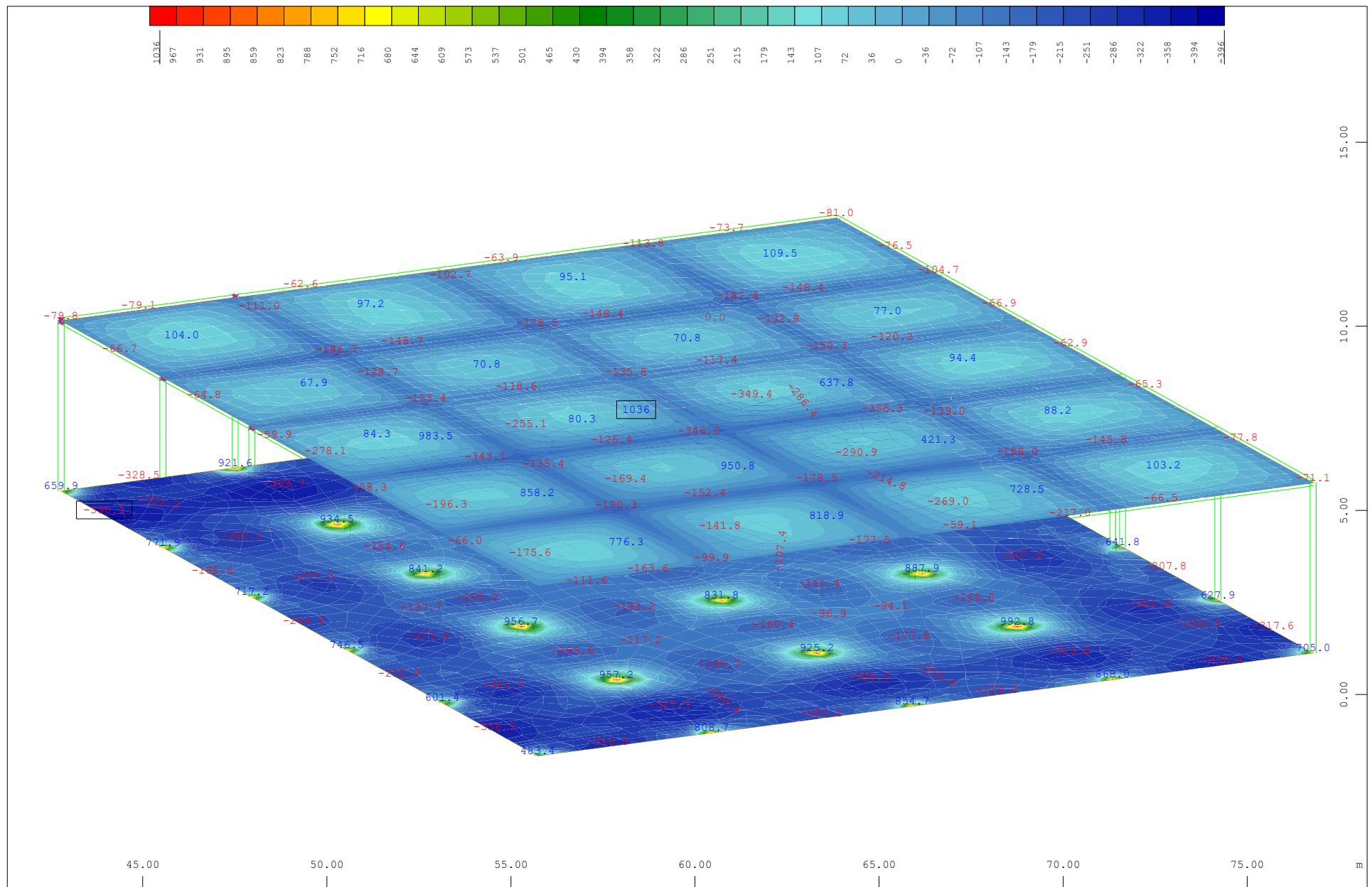
Z * 0.962



Z
Y
X

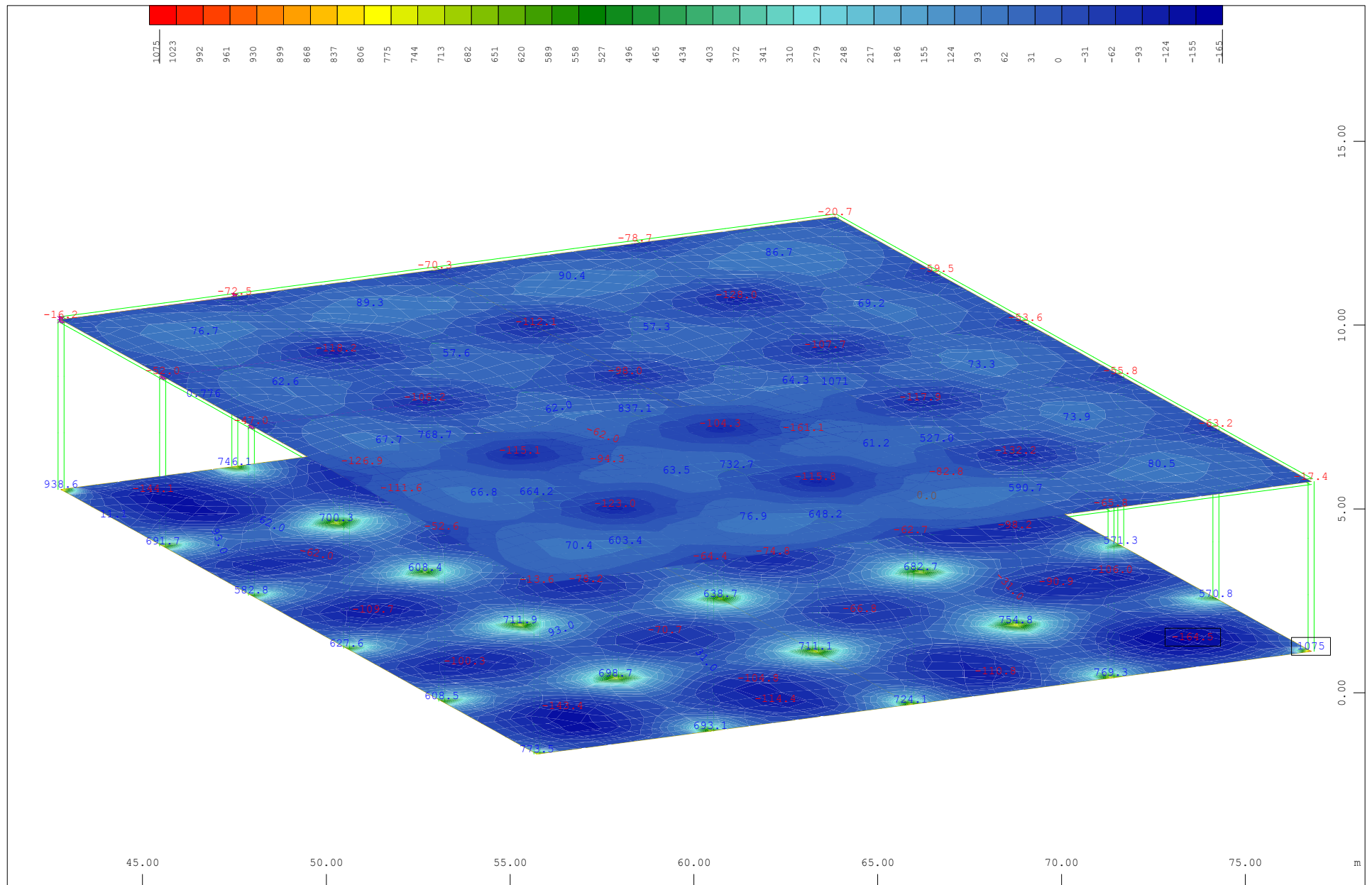
Principal bending moment I in Node, nonlinear Loadcase 1002 1.35G+1.35G+Q_E+1.5Q , from -224.8 to 1516. step 43.5 kNm/m

M 1 : 146
X * 0.502
Y * 0.906
Z * 0.962



Z
Y

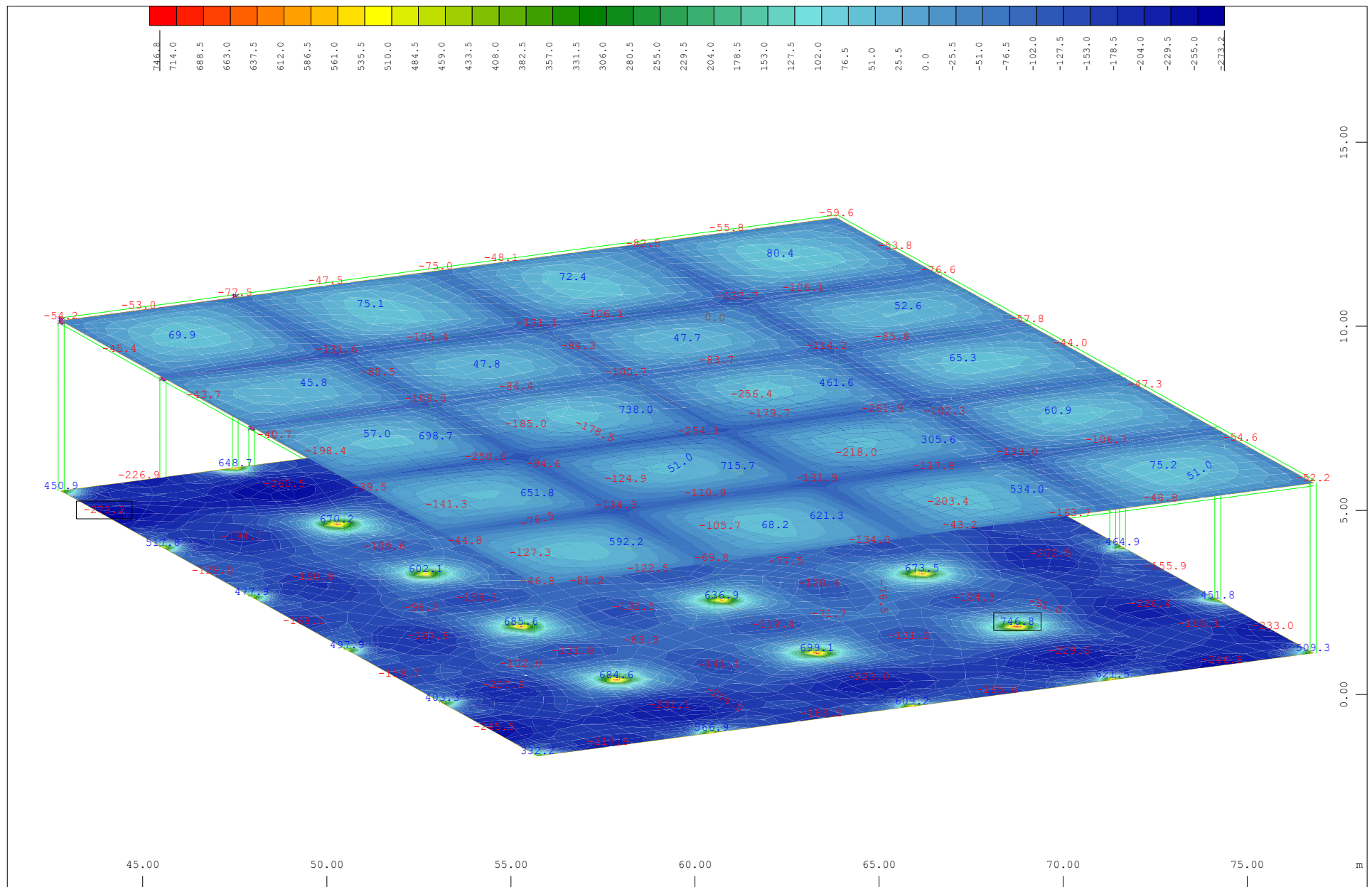
M 1 : 146
X * 0.502
Y * 0.906
Z * 0.962



Z
Y
X

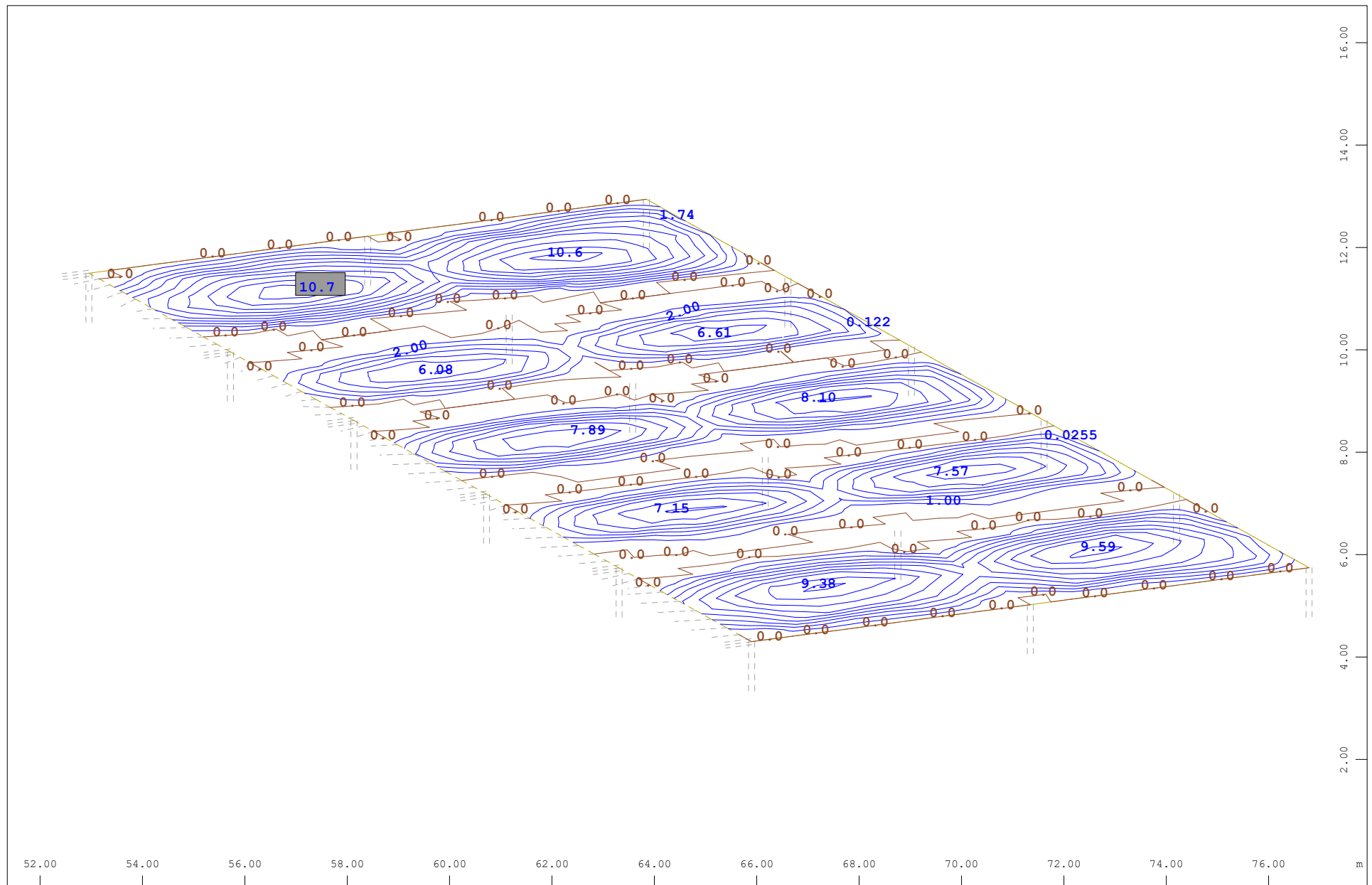
Principal bending moment I in Node, nonlinear Loadcase 2002 G+G+Q_E+Q , from -164.5 to 1075. step 31.0 kNm/m

M 1 : 146
X * 0.502
Y * 0.906
Z * 0.962



Z
X
Y

M 1 : 146
X * 0.502
Y * 0.906
Z * 0.962

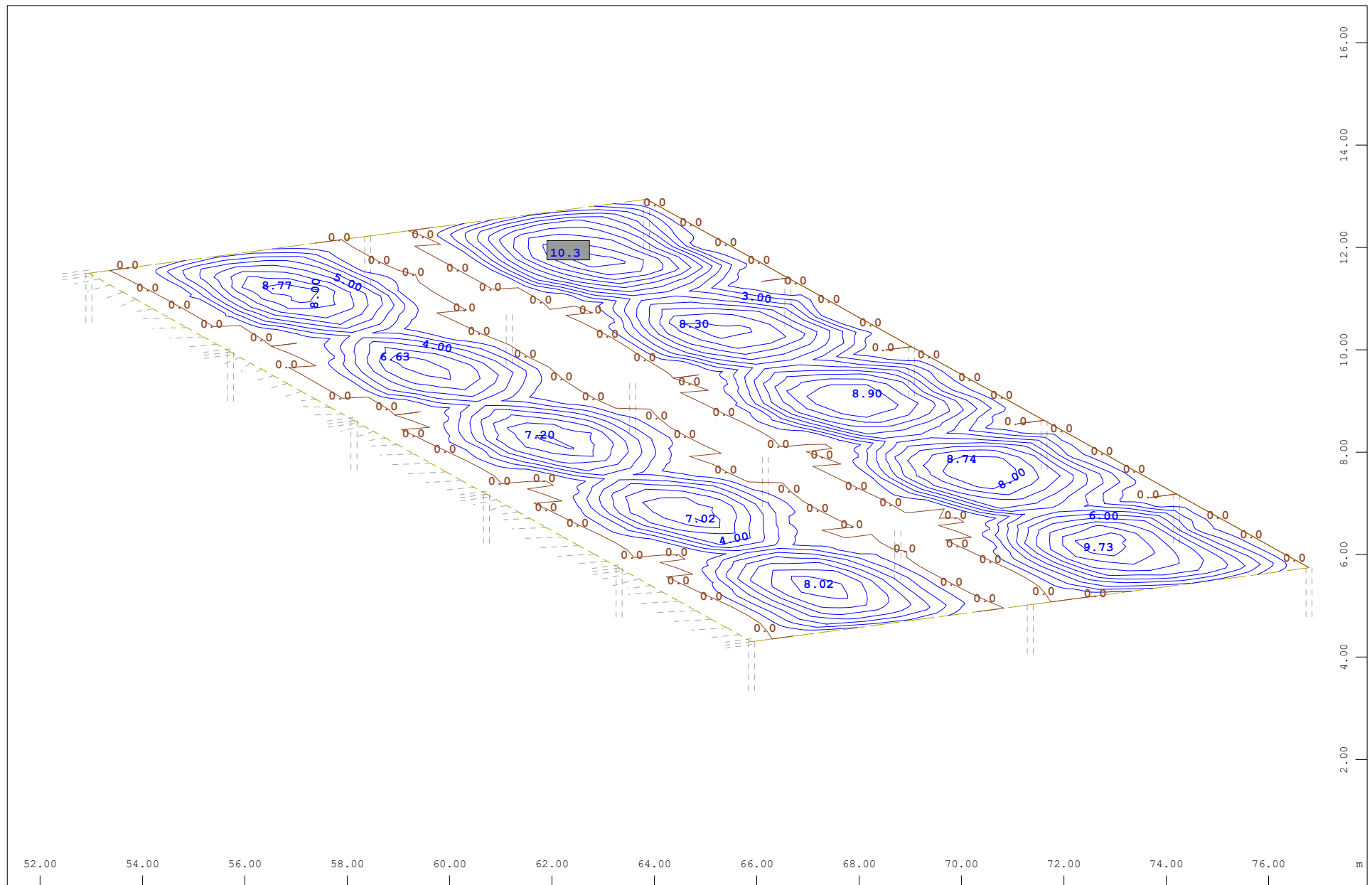


z
 Y
 X

Sector of system Group 6
 Quadrilateral Elements , lower Principal reinforcements (1st layer) in Node
 cm²/m

↗, Design Case 1 ULS design , from 0 to 10.7 step 1.00

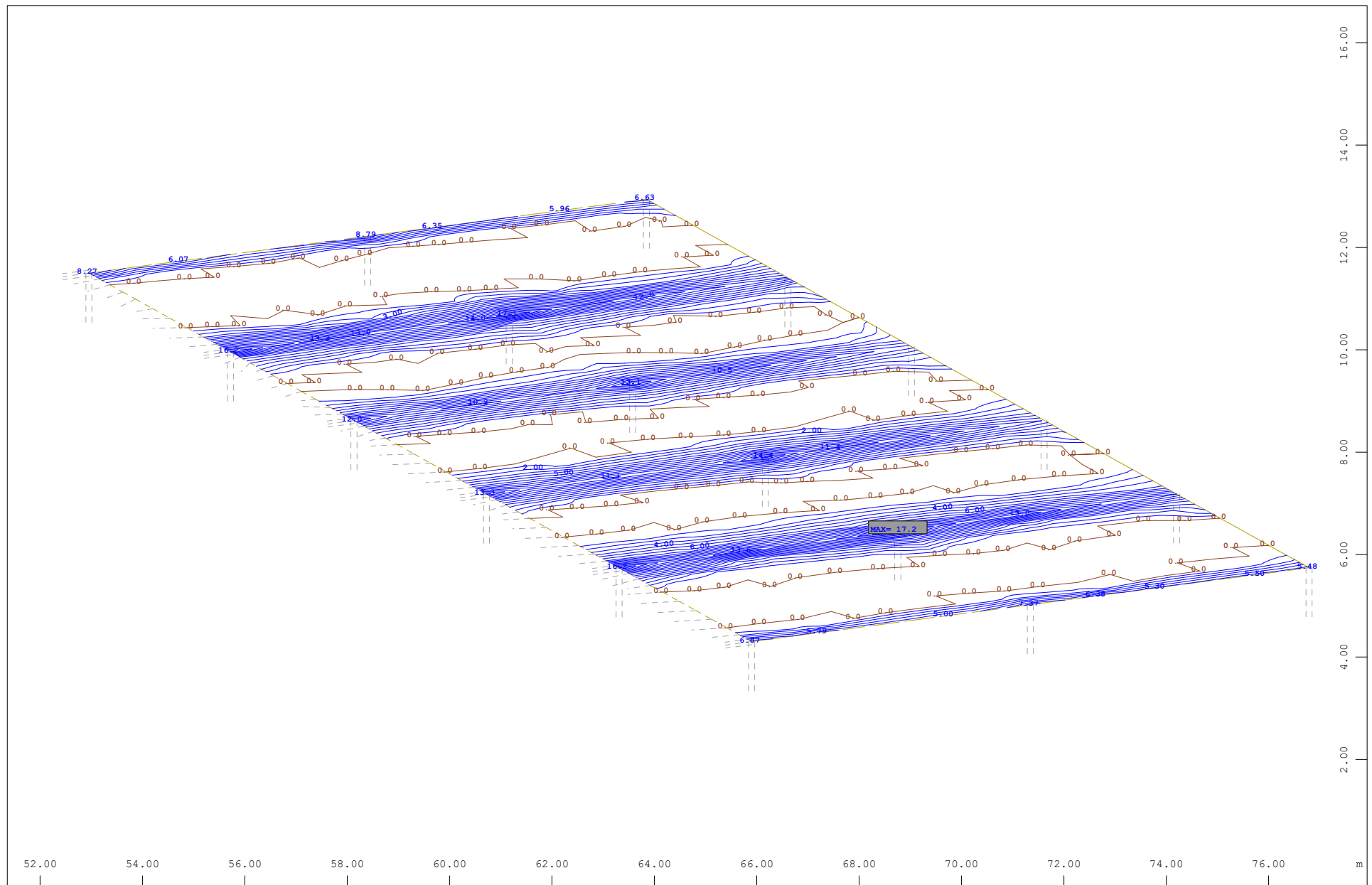
M 1 : 105
 X * 0.502
 Y * 0.906
 Z * 0.962



Z
 Y
 Sector of system Group 6
 Quadrilateral Elements, lower Cross reinforcements (2nd layer) in Node

↗, Design Case 1 ULS design, from 0 to 10.3 step 1.00 cm²/m

M 1 : 105
 X * 0.502
 Y * 0.906
 Z * 0.962

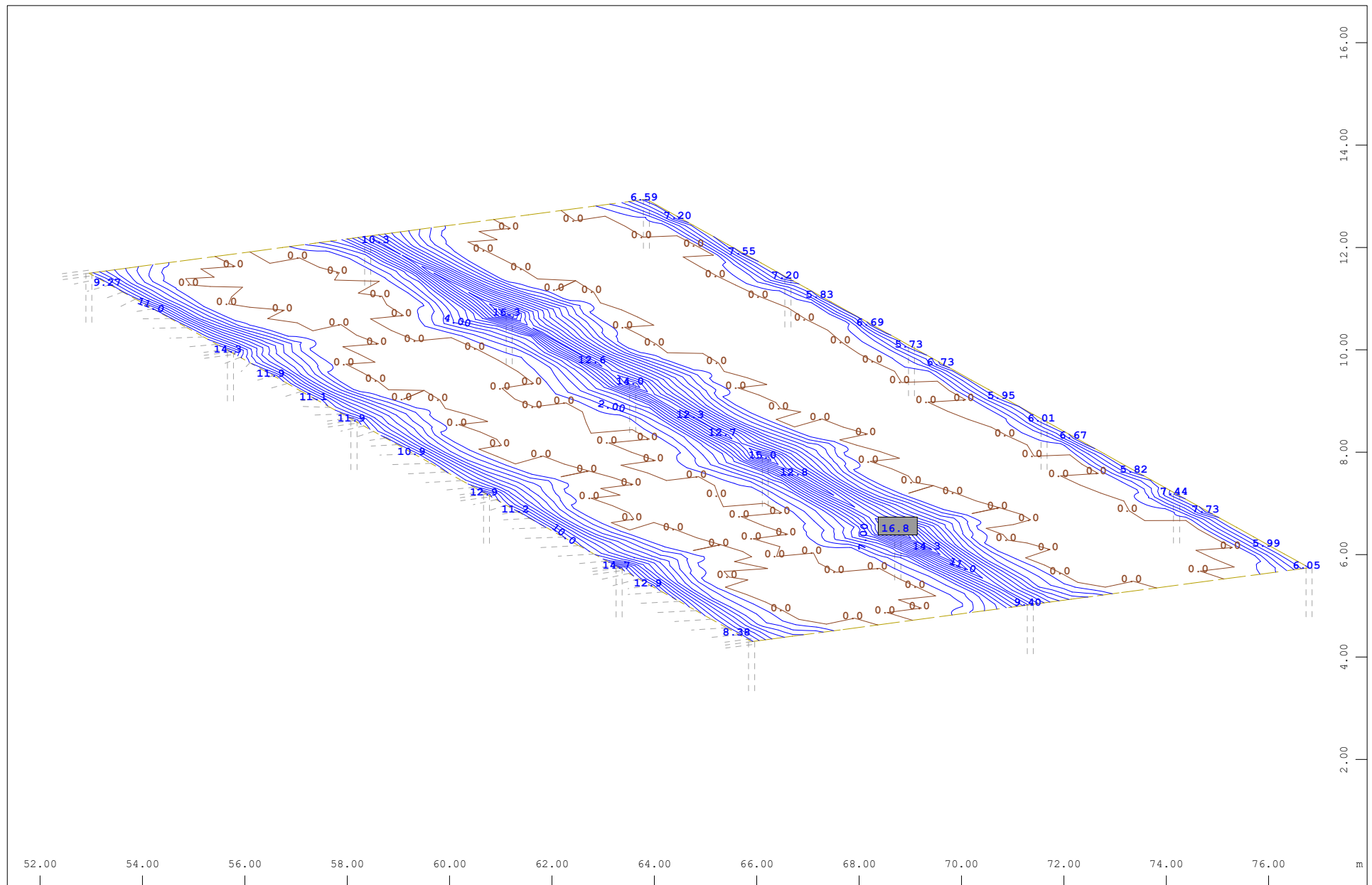


Z
 Y
 X

Sector of system Group 6
 Quadrilateral Elements , upper Principal reinforcements (1st layer) in Node
 cm2/m

N, Design Case 1 ULS design , from 0 to 17.2 step 1.00

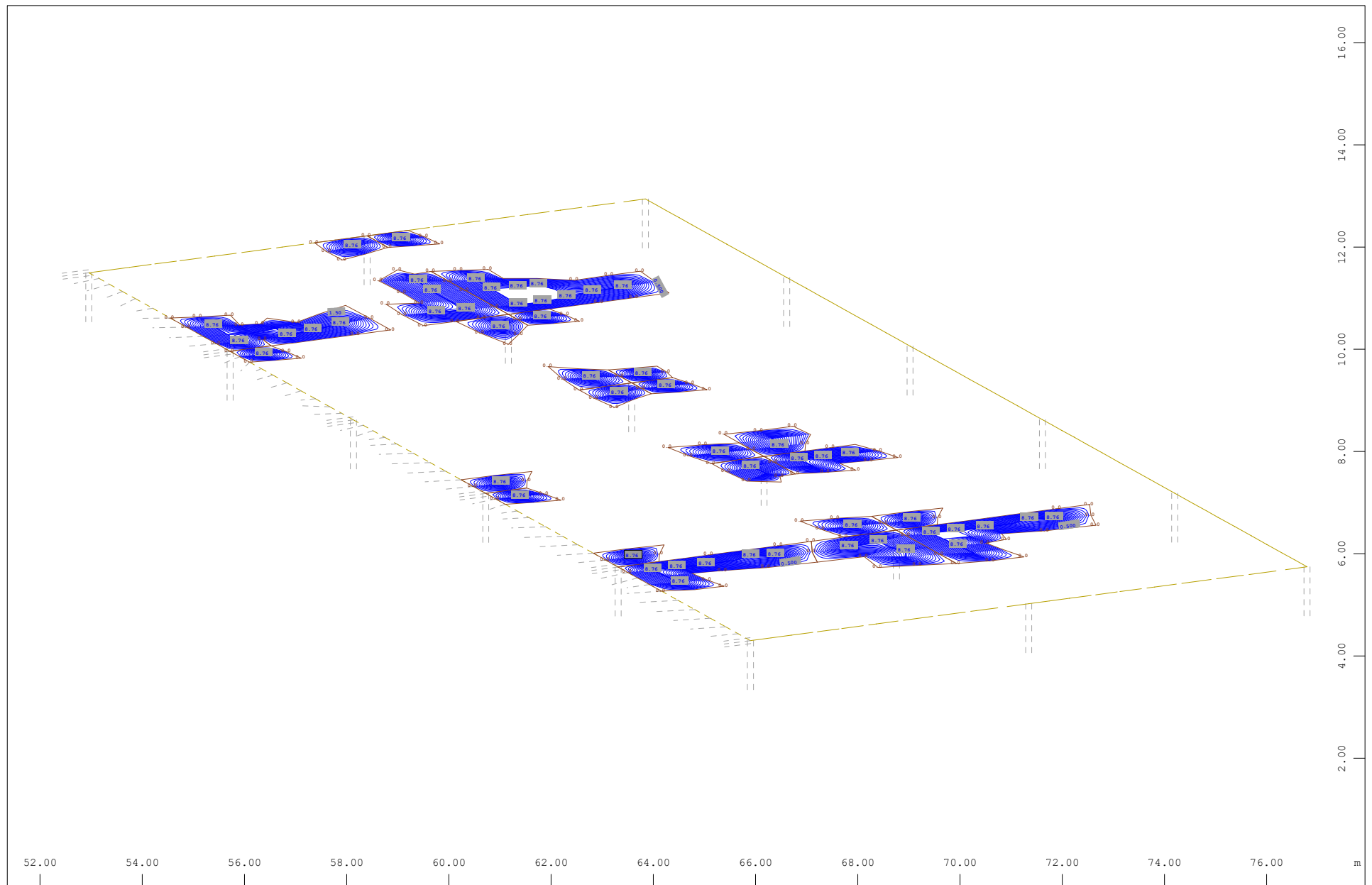
M 1 : 105
 X * 0.502
 Y * 0.906
 Z * 0.962



Z
 Y
 Sector of system Group 6
 Quadrilateral Elements , upper Cross reinforcements (2nd layer) in Node

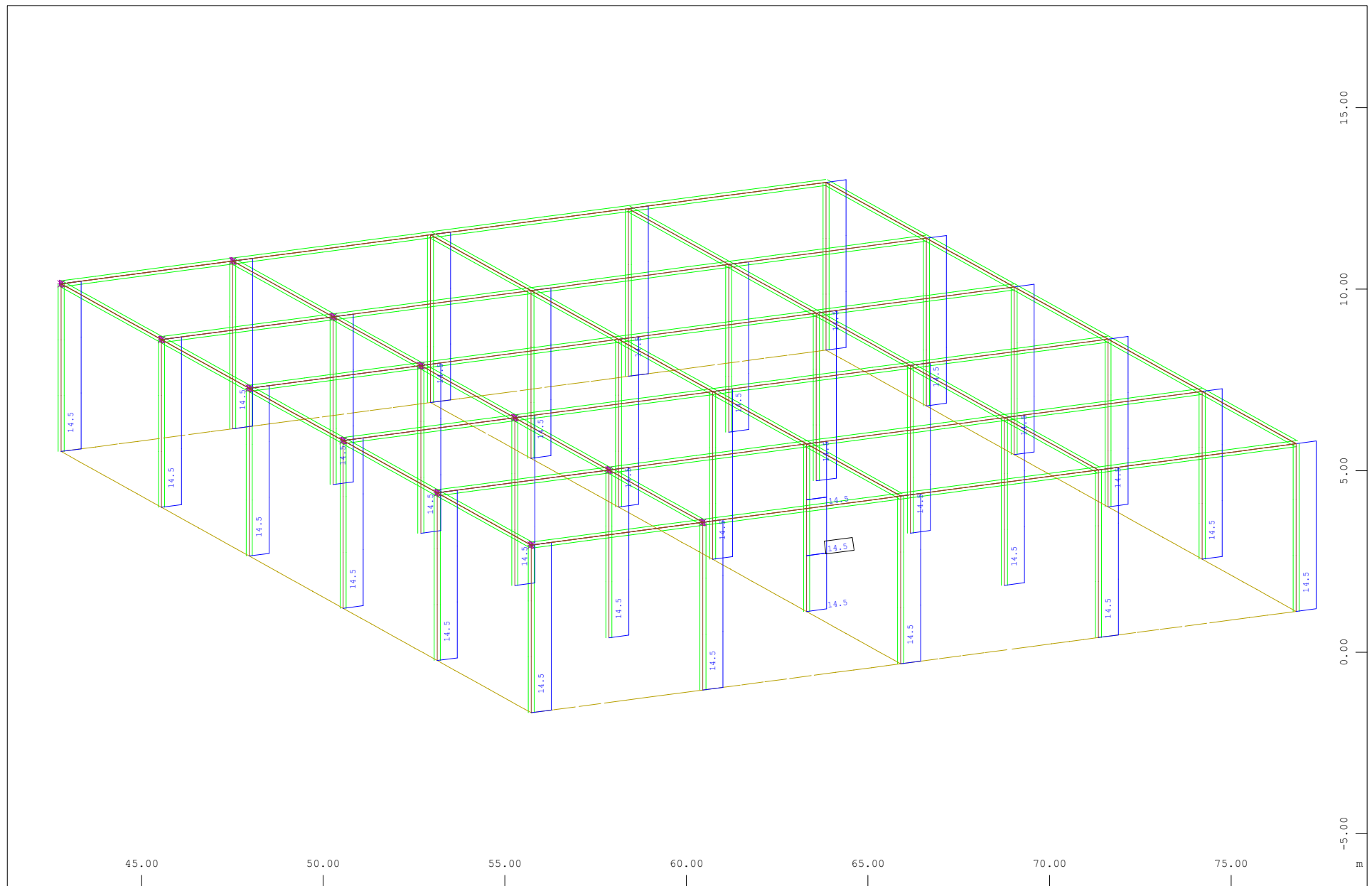
↗ , Design Case 1 ULS design , from 0 to 16.8 step 1.00 cm²/m

M 1 : 105
 X * 0.502
 Y * 0.906
 Z * 0.962



Sector of system Group 6
 Quadrilateral Elements , Shear reinforcement in Node 1 , Design Case 1 ULS design , from 0 to 8.76 step 0.500 cm²/m²

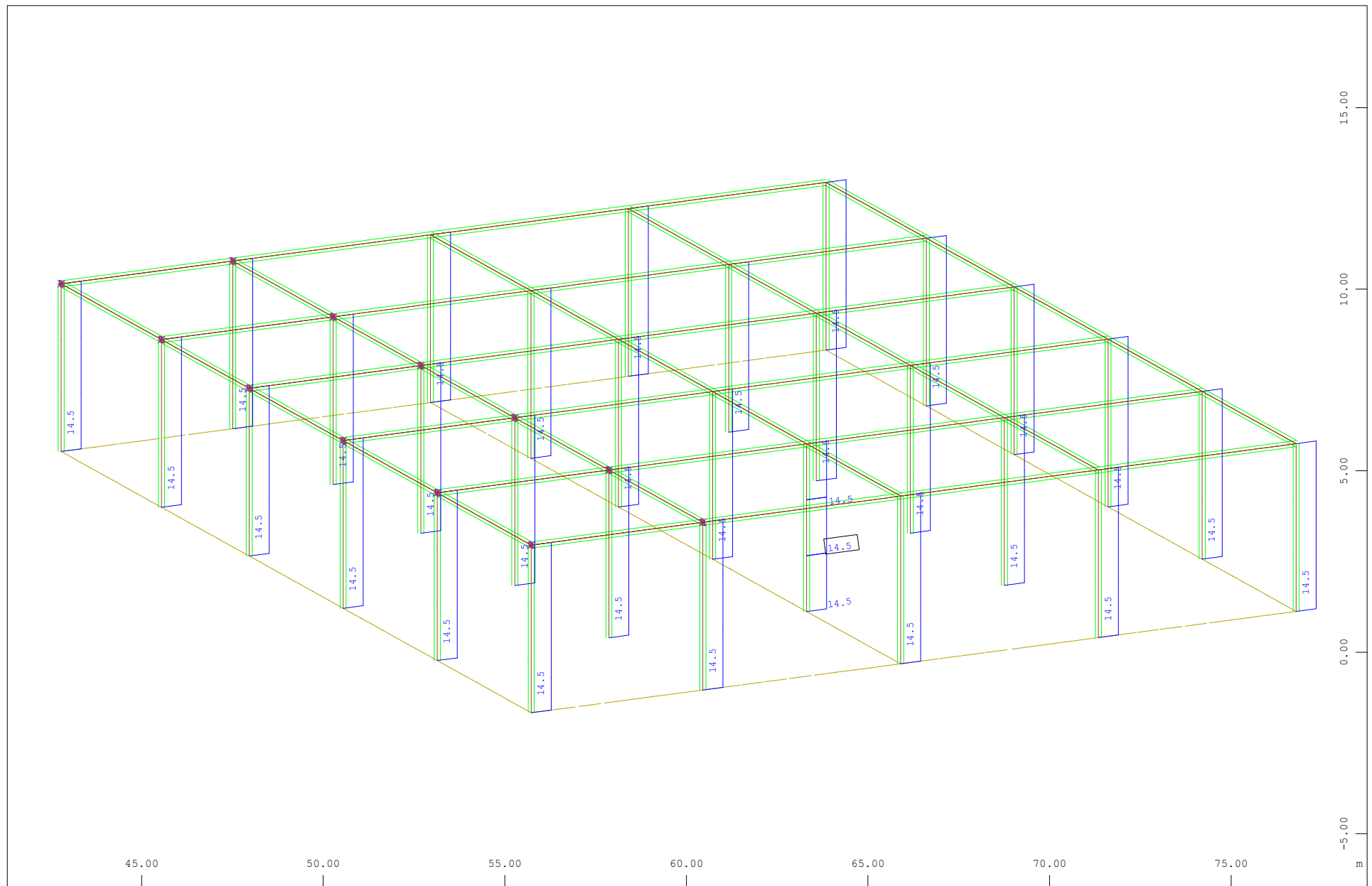
M 1 : 105
 X * 0.502
 Y * 0.906
 Z * 0.962



Z
Y
X

Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case 1 , 1 cm 3D = 34.8 cm2 (Max=14.5)

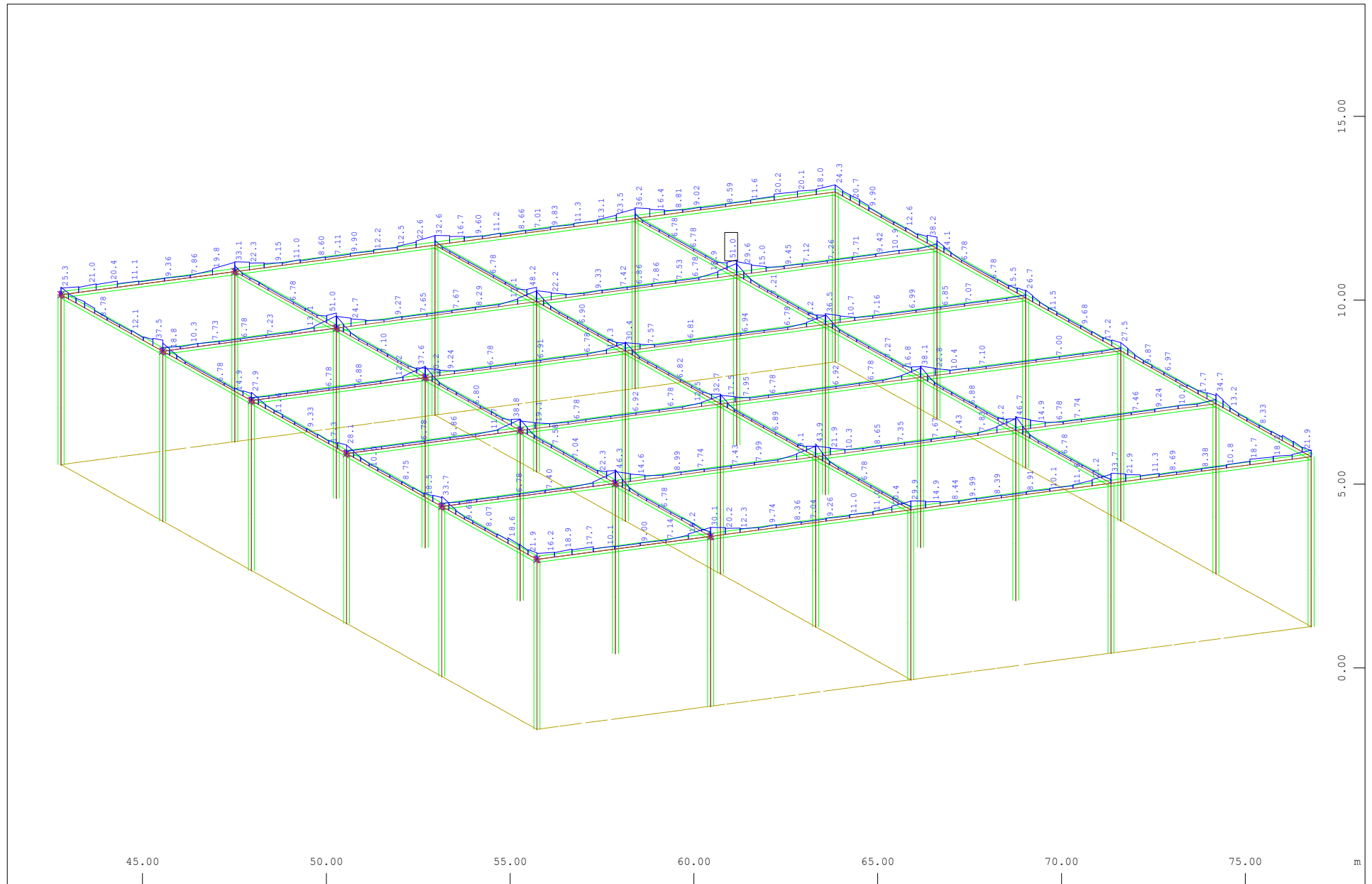
M 1 : 148
X * 0.502
Y * 0.906
Z * 0.962



Z
Y
X

Beam Elements , Longitudinal Reinforcements Lay. 0, Design Case 1 , 1 cm 3D = 34.8 cm2 (Max=14.5)

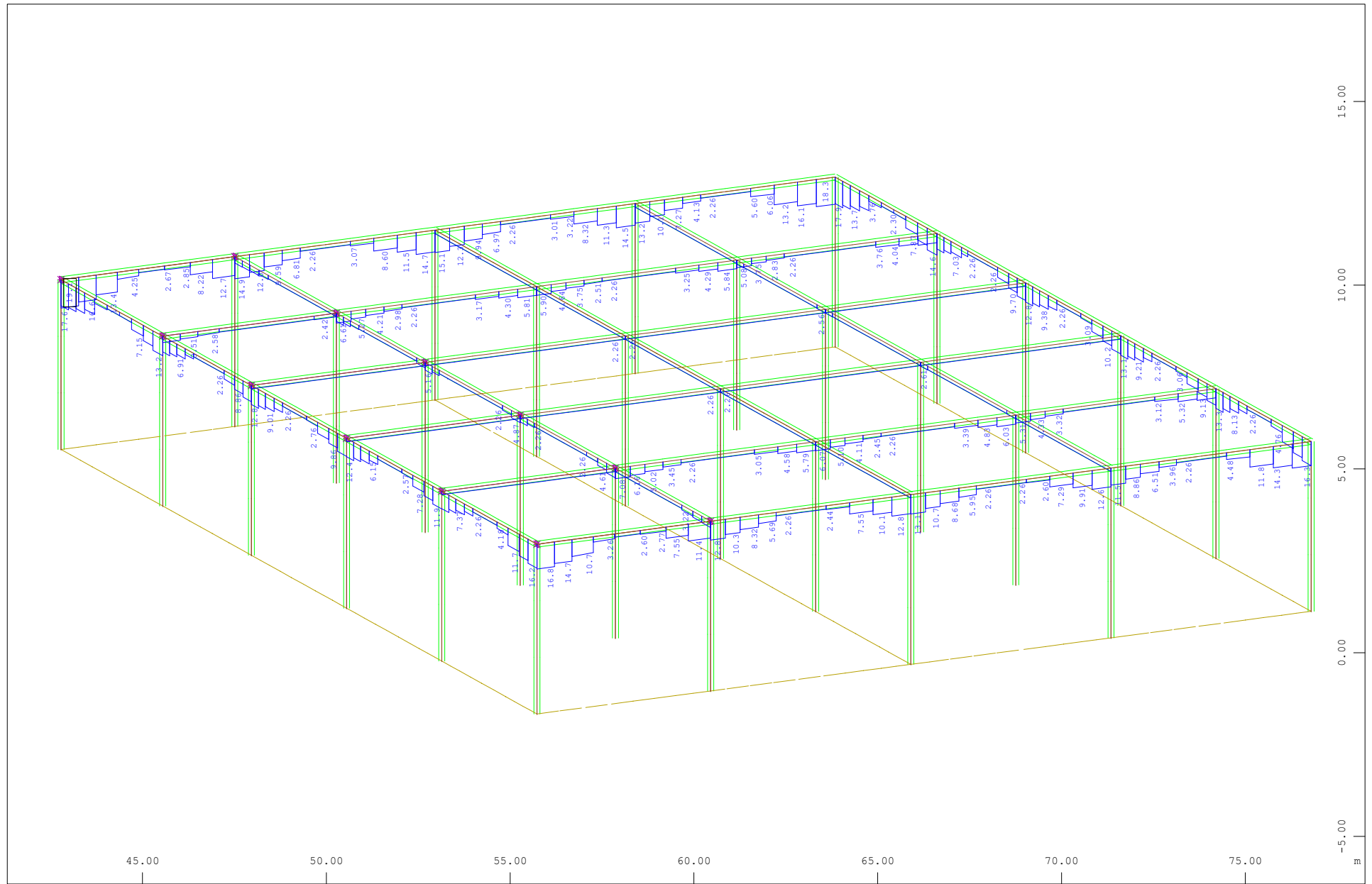
M 1 : 148
X * 0.502
Y * 0.906
Z * 0.962



Z
X Y

Beam Elements , Longitudinal Reinforcements Lay. 2, Design Case 1 , 1 cm 3D = 174.2 cm2 (Max=51.0)

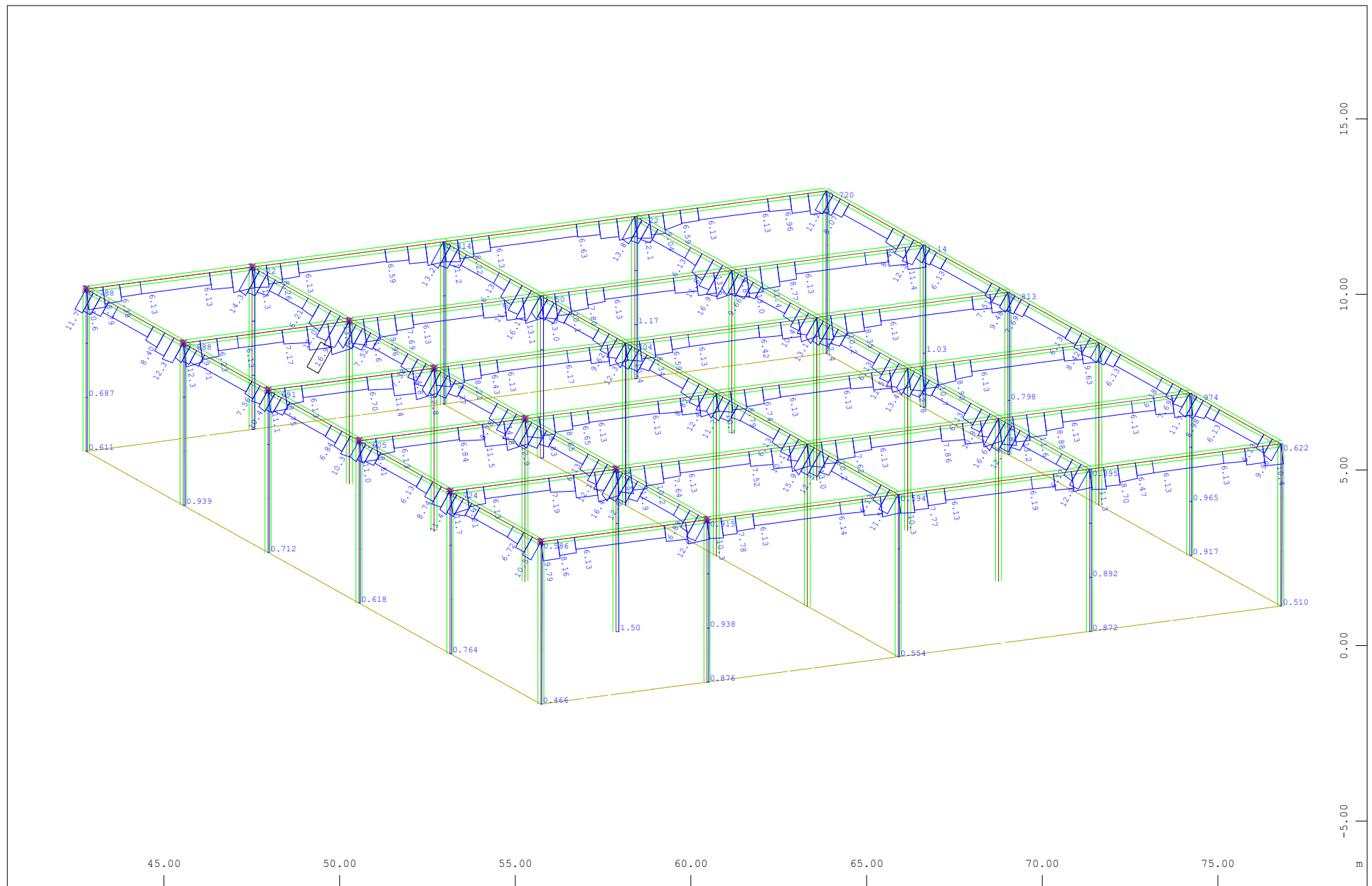
M 1 : 146
X * 0.502
Y * 0.906
Z * 0.962



Z
Y
X

Beam Elements , Longitudinal Reinforcements Lay. 3, Design Case 1 , 1 cm 3D = 34.8 cm2 (Max=19.1)

M 1 : 146
X * 0.502
Y * 0.906
Z * 0.962



Z
Y

Beam Elements , Stirrup Reinforcements Lay. 1, Design Case 1 , 1 cm 3D = 27.5 cm²/m (Max=16.9)

M 1 : 153
X * 0.502
Y * 0.906
Z * 0.962