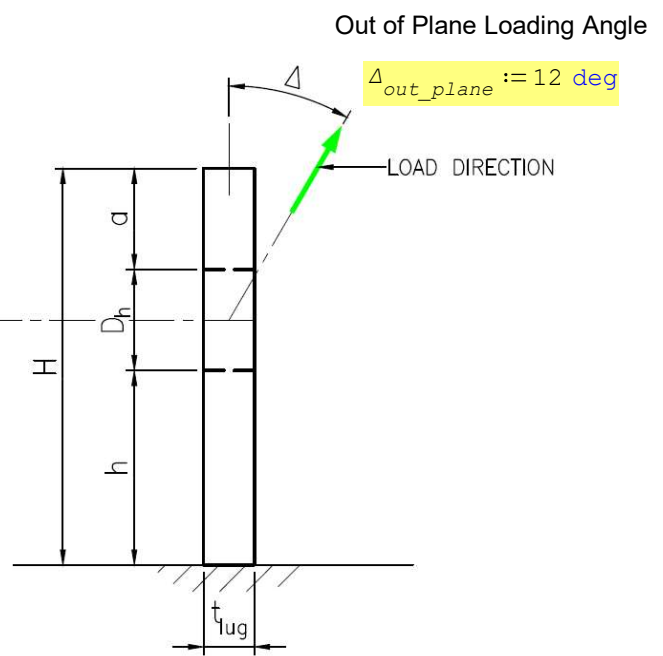
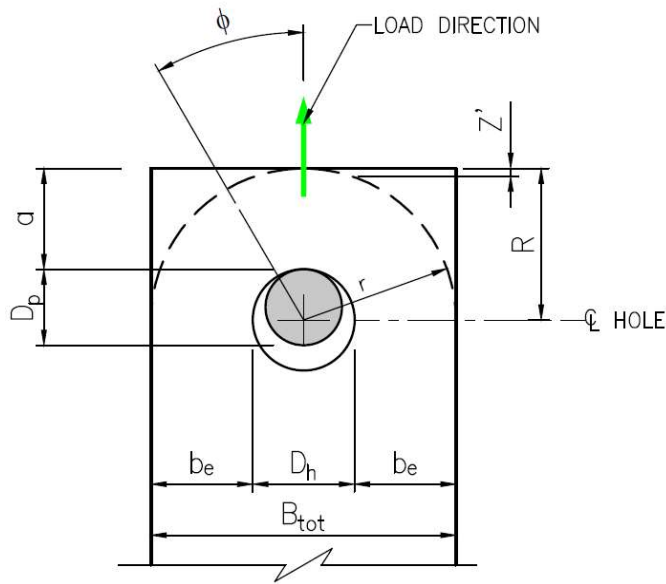


Big Important Client

Lifting Lug Design- Static Design Basis

09/29/2024

Client Code: 1501-1704



Material Properties

Lug Properties

$$E := 29000 \text{ ksi}$$

$$F_{y_lug} := 36 \text{ ksi}$$

$$F_{u_lug} := 58 \text{ ksi}$$

Base Metal Properties

$$F_{y_base} := 36 \text{ ksi}$$

$$F_{u_base} := 58 \text{ ksi}$$

Enter Nominal Design Factor for Max Allowable Stress

3-1.3.1 Nominal Design Factors. The static strength design of a below-the-hook lifting device shall be based on the allowable stresses defined in sections 3-2 and 3-3. The minimum values of the nominal design factor, N_d , in the allowable stress equations shall be as follows:

- $N_d = 2.00$ for Design Category A lifters
- $N_d = 3.00$ for Design Category B lifters
- $N_d = 6.00$ for Design Category C lifters

Design Category: **A**

Nominal Design Factor: $N_d := 2$

See ASME B30
Sec. 2-2

See ASME B30
Eq. C-2

Lug Section Geometry

Material Above Pin: $a := 2 \text{ in}$

Hole Diameter: $D_h := 2 \text{ in}$

Pin Diameter: $D_{pin} := \frac{1}{2} \text{ in}$

Radius: $r := \frac{1}{2} \cdot D_h + a = 3 \text{ in}$

Side Material: $b_e := r - \frac{1}{2} \cdot D_h = 2 \text{ in}$

Base Total: $B_{tot} := 2 \cdot b_e + D_h = 6 \text{ in}$

Material Below Hole: $h := 2.5 \text{ in}$

Total Height: $H := h + D_h + a = 6.5 \text{ in}$

Lug Thickness: $t_{lug} := \frac{1}{2} \text{ in}$

ϕ = Shear Plane Loading Angle

$$\phi_{loading_angle} := 15 \text{ deg}$$

Shear Plane Reduction Due to Curve

$$Z_{prime} := r - \sqrt{r^2 - \left(\frac{D_{pin}}{2} \cdot \left(\sin(\phi_{loading_angle}) \right) \right)^2} = 0 \text{ in}$$

Calculate Allowable Stresses for Static Design

We are going to calculate the allowable stresses per ASME - BTH-1, and then convert those stresses into equivalent safety factors. Then, we will use AISC Allowable Stress Equations for design.

Gross Tension Yield ASME BTH-1 Eq. 3-1 & 3-2

$$F_{t_gross} := \frac{F_{y_lug}}{N_d} = 18 \text{ ksi}$$

$$F_{t_effect} := \frac{F_{y_lug}}{1.2 \cdot N_d} = 24.2 \text{ ksi}$$

$$\Omega_{tension_gross} := \frac{F_{y_lug}}{F_{t_gross}} = 2$$

Bearing Stress ASME BTH-1 Eq. 3-53

$$F_p := \frac{1.25 \cdot F_{y_lug}}{N_d} = 22.5 \text{ ksi}$$

$$\Omega_{bearing} := \frac{F_{y_lug}}{F_p} = 1.6$$

Minor Axis Bending - Solid Bars ASME BTH-1 Eq. 3-25

$$F_b := \frac{1.25 \cdot F_{y_lug}}{N_d} = 22.5 \text{ ksi}$$

$$\Omega_{bending} := \frac{F_{y_lug}}{F_b} = 1.6$$

Shear on Bars, Pins & Plates See ASME B30 Sec. 3-2.3.6

$$F_v := \text{if } \frac{a}{t_{lug}} \leq 2.45 \cdot \sqrt{\frac{E}{F_{y_lug}}} = 10.4 \text{ ksi}$$

$$\frac{F_{y_lug}}{N_d \cdot \sqrt{3}}$$

else

$$\frac{F_y}{N_d}$$

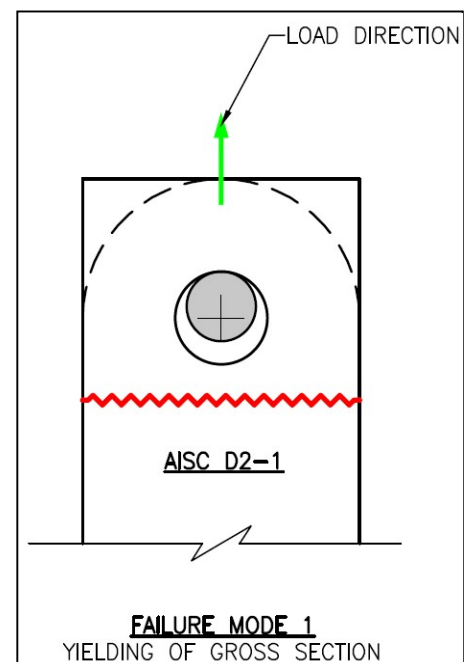
$$\Omega_{shear} := \frac{F_{y_lug}}{F_v} = 3.5$$

Failure Mode 1 - Yielding of Gross Section

(Previously Calc'd)

$$\Omega_{tension_gross} = 2$$

$$P_{allow_gross_yield} := \frac{t_{lug} \cdot B_{tot} \cdot F_{y_lug}}{\Omega_{tension_gross}} = 54 \text{ kip}$$



Failure Mode 2 - Tension Rupture on Effective Area

(Previously Calc'd)

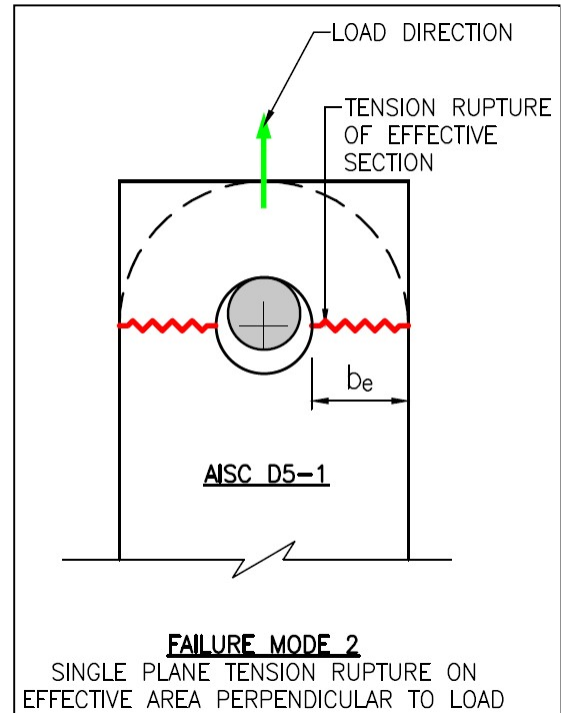
$$b_e = 2 \text{ in} \quad t_{lug} = 0.5 \text{ in}$$

$$\Omega_{tens_rupt} := 5 \quad (\text{treat at OSHA Rigging Component})$$

Apply AISC Dimension limitations Sect. D5-1b

$$b_e := \min \left(\left[\begin{array}{c} b_e \\ 2 \cdot t_{lug} + .63 \text{ in} \end{array} \right] \right) = 1.63 \text{ in}$$

$$P_{allow_tens_rupt} := \frac{F_{u_lug} \cdot (2 \cdot t_{lug} \cdot b_e)}{\Omega_{tens_rupt}} = 18.9 \text{ kip}$$



Failure Mode 3 - Shear Rupture on Planes Parallel to Line of Action

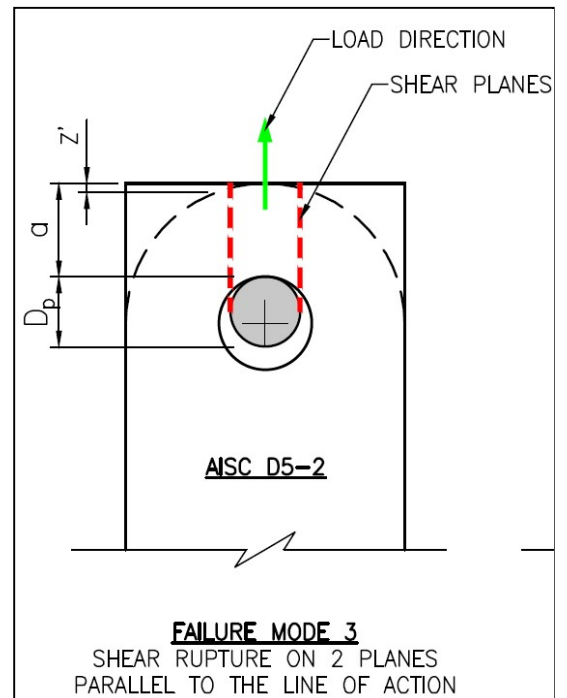
(Previously Calc'd)

$$\Omega_{shear} = 3.5$$

$$A_{sf} := 2 \cdot \left(a - z_{prime} + \frac{D_{pin}}{2} \right) \cdot t_{lug} = 2.25 \text{ in}^2$$

$$P_{allow_shear} := \frac{0.6 \cdot F_{u_lug} \cdot A_{sf}}{\Omega_{shear}} = 22.6 \text{ kip}$$

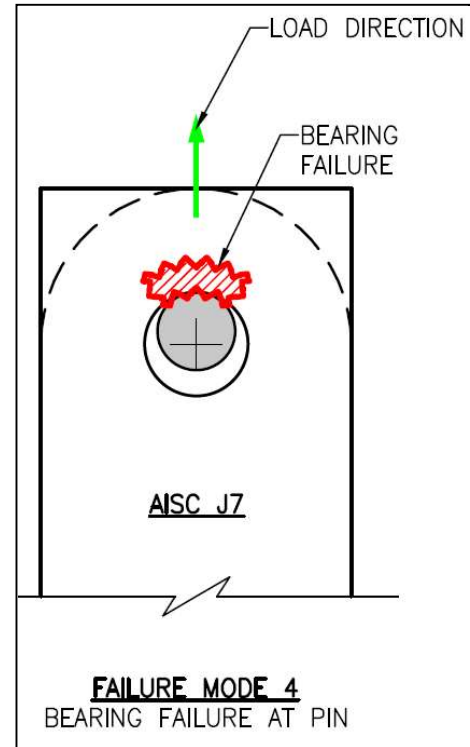
AISC EQ. D5-2



Failure Mode 4 - Bearing Failure @ Pin

$$P_{allow_bearing} := \frac{.9 \cdot F_{y_lug} \cdot t_{lug} \cdot D_{pin}}{\Omega_{bearing}} = 5.1 \text{ kip}$$

Note: We are deviating from AISC equation J7-1. Equation J7-1 applies a 1.8 coefficient to the yield stress. We have replaced that with a 0.9 coefficient based upon David Ricker's 1991 Engineering Journal recommendations. AISC assumes a tight fitting pin. For lifting operations we will have a loose fitting pin, and Ricker reports capacity is reduced.

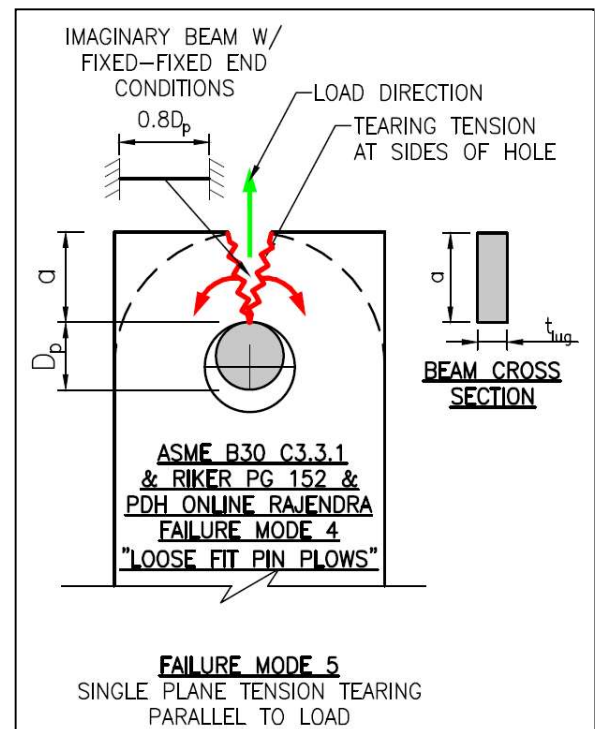


Failure Mode 5 - Tearing Tension Along Line of Action

$$P_{allow_Tear} = \frac{1.67 \cdot F_{y_lug} \cdot t_{lug} \cdot a^2}{\Omega_{bending} D_{pin}}$$

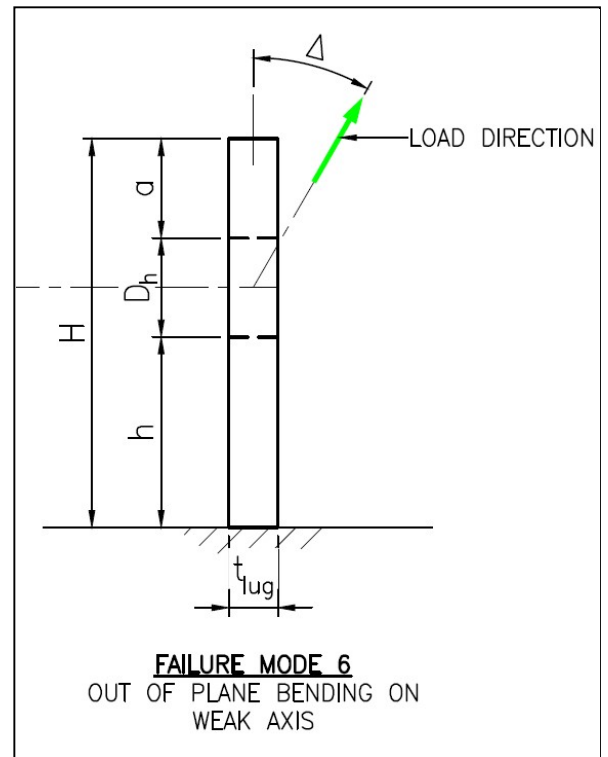
Flexural Tension on Fixed-Fixed Beam
Equation from Ricker Article pg. 152 and Figure 15
(also in Rajendra Failure Mode 4)

$$P_{allow_tear} := \frac{1.67 \cdot F_{y_lug} \cdot t_{lug} \cdot a^2}{\Omega_{bending} \cdot D_{pin}} = 150.3 \text{ kip}$$



Failure Mode 6 - Out of Plane Bending - Weak Axis

$$Z_{plastic} := \frac{1}{4} \cdot B_{tot} \cdot t_{lug}^2 = 0.375 \text{ in}^3$$



$$P_{allow_flexure} := \frac{F_{y_lug} \cdot Z_{plastic}}{\Omega_{bending} \cdot \sin(\Delta_{out_plane}) \cdot \left(h + \frac{D_h}{2} \right)} = 11.6 \text{ kip}$$

Summary

Failure Mode 1 - Yielding of Gross Section

Failure Mode 2 - Tension Rupture on Effective Area

Failure Mode 3 - Shear Rupture on Planes Parallel to Line of Action

Failure Mode 4 - Bearing Failure @ Pin

Failure Mode 5 - Tearing Tension Along Line of Action

Failure Mode 6 - Out of Plane Bending - Weak Axis

$$P_{allow_gross_yield} = 54 \text{ kip}$$

$$P_{allow_tens_rupt} = 18.9 \text{ kip}$$

$$P_{allow_shear} = 22.6 \text{ kip}$$

$$P_{allow_bearing} = 5.1 \text{ kip}$$

$$P_{allow_tear} = 150.3 \text{ kip}$$

$$P_{allow_flexure} = 11.6 \text{ kip}$$

$$P_{n_allow} := \min \left[\begin{array}{c} P_{allow_gross_yield} \\ P_{allow_tens_rupt} \\ P_{allow_shear} \\ P_{allow_bearing} \\ P_{allow_tear} \\ P_{allow_flexure} \end{array} \right] = 5.1 \text{ kip}$$