

## Wood Beam calculation

Using Southern Pine No. 2 , therefore properties common to all Southern Pine

$$G := 0.55$$

$$\gamma := G \cdot 62.4 \frac{\text{lb}}{\text{ft}} = 34.32 \frac{\text{lb}}{\text{ft}}$$

$$L_b := 16 \text{ ft}$$

$$Trib := 1 \text{ ft}$$

Use **2x10** for Joist application

$$b := 1.5 \text{ in}$$

$$d := 9.25 \text{ in}$$

$$F_b := 800 \frac{\text{lb}}{\text{in}^2}$$

$$F_v := 175 \frac{\text{lb}}{\text{in}^2}$$

$$F_t := 475 \frac{\text{lb}}{\text{in}^2}$$

$$F_c := 1300 \frac{\text{lb}}{\text{in}^2}$$

$$E := 1400000 \frac{\text{lb}}{\text{in}^2}$$

$$S := \frac{b \cdot d^2}{6} = 21.3906 \text{ in}^3$$

$$I := \frac{b \cdot d^3}{12} = 98.93 \text{ in}^4$$

$$C_D := 1.0$$

$$C_{ME} := .9 \quad \text{Adjustment Factor for E calcs}$$

$$E' := E \cdot C_{ME} = 1260 \frac{\text{kip}}{\text{in}^2}$$

$$\frac{d}{b} = 6.1667$$

$$COV_E := .25 \quad \text{Appendix F NDS}$$

$$\text{since } \frac{d}{b} > 6 \quad CL < 1.0$$

$$E'_{min} := E' \cdot \left(1 - 1.645 \cdot COV_E\right) \cdot \frac{1.03}{1.66} = 460.289 \frac{\text{kip}}{\text{in}^2}$$

## Enercalc Calculations

Adjustment Factors for Bending:

$$\text{if } F_b \leq 1150 \frac{\text{lb}}{\text{in}^2} = 1$$

see Wet Service Factor Table Footnote

$$C_M := 1$$

else

$$C_M := .85$$

$$C_r := 1.15$$

Beam Stability Factor

Lateral blocking spacing for buckling in ft

$$\text{NumberOfIntermediateBlkgLocations} := 16$$

$$l_u := \frac{L_b}{\text{NumberOfIntermediateBlkgLocations} + 1} = 0.9412 \text{ ft}$$

Unbraced length that compression flange is not braced

$$l_u = 11.2941 \text{ in}$$

$$\frac{l_u}{d} = 1.221$$

$$\text{if } \frac{l_u}{d} < 7 \quad = 1.9388 \text{ ft}$$

$$l_e := 2.06 \cdot l_u$$

else

$$l_e := 1.63 \cdot l_u + 3 \cdot d$$

$$l_e = 1.9 \text{ ft}$$

$$R_B := \sqrt{\frac{l_e \cdot d}{b^2}} = 9.78$$

if  $R_B < 50 = \text{"PASS"}$  <<<<<CHECK

"PASS"

else

"FAIL"

$$E'_{min} := E' \cdot (1 - 1.645 \cdot COV_E) \cdot \frac{1.03}{1.66} = 460.3 \frac{\text{kip}}{\text{in}^2}$$

$$F_{bE} := \frac{1.2 \cdot E'_{min}}{R_B^2} = 5.7747 \frac{\text{kip}}{\text{in}^2}$$

$$F'_b := F_b \cdot C_D \cdot C_M \cdot C_r = 0.92 \frac{\text{kip}}{\text{in}^2}$$

Does not matche Enercalc due to CM factor footnote, see above

$$C_L := \frac{1 + \left( \frac{F_{bE}}{F'_b} \right)}{1.9} - \sqrt{\left( \frac{1 + \left( \frac{F_{bE}}{F'_b} \right)}{1.9} \right)^2 - \left( \frac{F_{bE}}{F'_b} \right)} = 0.9907$$

$$F''_b := F'_b \cdot C_L = 0.9115 \frac{\text{kip}}{\text{in}^2}$$

## Load Calcs

$$Wght := \frac{b \cdot d \cdot 12 \text{ in} \cdot \gamma}{\text{ft}} = 3.3069 \frac{\text{lbft}}{\text{ft}}$$

$$c := \frac{d}{2} = 4.625 \text{ in}$$

## Loading

$$DL := 10 \frac{\text{lbft}}{\text{ft}^2} - 3.3069 \frac{\text{lbft}}{\text{ft}^2}$$

$$L := 40 \frac{\text{lbft}}{\text{ft}^2}$$

$$L_r := 20 \frac{\text{lbft}}{\text{ft}^2}$$

$$SL := 40 \frac{\text{lbft}}{\text{ft}^2}$$

$$WL := 10 \frac{\text{lbft}}{\text{ft}^2}$$

$$D := DL \cdot Trib + Wght = 10 \frac{\text{lbft}}{\text{ft}}$$

$$L := L \cdot Trib = 40 \frac{\text{lbft}}{\text{ft}}$$

$$L_r := L_r \cdot Trib = 20 \frac{\text{lbft}}{\text{ft}}$$

$$SL := SL \cdot Trib = 40 \frac{\text{lbft}}{\text{ft}}$$

$$W := WL \cdot Trib = 10 \frac{\text{lbft}}{\text{ft}}$$

$$L_{LorR} := \max \left( \left[ L_r \quad L \right] \right) = 40 \frac{\text{lbf}}{\text{ft}}$$

$$W_1 := D + L_{LorR} = 50 \frac{\text{lbf}}{\text{ft}}$$

$$W_2 := D + SL = 50 \frac{\text{lbf}}{\text{ft}}$$

$$W_3 := D = 10 \frac{\text{lbf}}{\text{ft}}$$

$$W_4 := D + W = 20 \frac{\text{lbf}}{\text{ft}}$$

$$W_5 := D + .75 \cdot L_{LorR} + .75 \cdot SL = 70 \frac{\text{lbf}}{\text{ft}}$$

$$W_6 := D + .75 \cdot L_{LorR} + .75 \cdot SL + .45 \cdot W = 74.5 \frac{\text{lbf}}{\text{ft}}$$

Based on Live Load Combination Loading

Bending

$$w_{max} := W_1 = 50 \frac{\text{lbf}}{\text{ft}}$$

$$M := \frac{w_{max} \cdot L_b^2}{8} = 1.6 \text{ ft kip}$$

$$f_b := \frac{M \cdot C}{I} = 897.589 \frac{\text{lbf}}{\text{in}^2}$$

$$UC_{bend} := \frac{f_b}{F'_{t'}} = 0.9848$$

if  $UC_{bend} < 1 = \text{"PASS"}$

**"PASS"**

else

**"FAIL"**

<<<<<CHECK

Shear (Matches Enercalc)

$$C_M := .97$$

$$V_{at\_d\_dist} := w_{max} \cdot (L_b - d) - \frac{w_{max} \cdot L_b}{2} = 0.3615 \text{ kip}$$

$$f_v := \frac{3 \cdot V_{at\_d\_dist}}{2 \cdot b \cdot d} = 0.0391 \frac{\text{kip}}{\text{in}^2}$$

$$F'_{v} := F_v \cdot C_D \cdot C_M = 0.1698 \frac{\text{kip}}{\text{in}^2}$$

$$UC_{shear} := \frac{f_v}{F'_{v}} = 0.2302$$

if  $UC_{shear} < 1 = \text{"PASS"}$

**"PASS"**

else

**"FAIL"**

<<<<<CHECK

## Based on Snow Combination Loading

$$C_D := 1.15$$

$$F'_b := F_b \cdot C_D \cdot C_M \cdot C_r = 1026.26 \frac{\text{lbf}}{\text{in}^2}$$

$$C_L := \frac{1 + \left( \frac{F_{bE}}{F'_b} \right)}{1.9} - \sqrt{\left( \frac{1 + \left( \frac{F_{bE}}{F'_b} \right)}{1.9} \right)^2 - \frac{\left( \frac{F_{bE}}{F'_b} \right)}{.95}} = 0.9894$$

$$F''_b := F'_b \cdot C_L = 1015.4276 \frac{\text{lbf}}{\text{in}^2}$$

$$M := \frac{W_2 \cdot L_b^2}{8} = 1.6 \text{ ft kip}$$

$$f_b := \frac{M \cdot c}{I} = 897.589 \frac{\text{lbf}}{\text{in}^2}$$

$$UC_{bending} := \frac{f_b}{F''_b} = 0.884$$

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if UCbending < 1 = "PASS"
    "PASS"
else
    "FAIL"
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<<<<<CHECK

$$V := \frac{W_2 \cdot L_b}{2} = 0.4 \text{ kip}$$

$$f_v := \frac{3 \cdot V}{2 \cdot b \cdot d} = 0.0432 \frac{\text{kip}}{\text{in}^2}$$

$$F'_v := F_v \cdot C_D \cdot C_M = 0.1952 \frac{\text{kip}}{\text{in}^2}$$

$$UC_{shear} := \frac{f_v}{F'_v} = 0.2215$$

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if UCshear < 1 = "PASS"
    "PASS"
else
    "FAIL"
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<<<<<CHECK

## Base on Wind Combinations

$$C_D := 1.6$$

$$F'_h := F_h \cdot C_D \cdot C_M \cdot C_r = 1427.84 \frac{\text{lbf}}{\text{in}^2}$$

$$C_L := \frac{1 + \left( \frac{F_{bE}}{F'_{bE}} \right)}{1.9} - \sqrt{\left( \frac{1 + \left( \frac{F_{bE}}{F'_{bE}} \right)}{1.9} \right)^2 - \left( \frac{F_{bE}}{F'_{bE}} \right)} = 0.9842$$

$$F'_{bE} := F'_{bE} \cdot C_L = 1405.2435 \frac{\text{lbf}}{\text{in}^2}$$

$$w_{max} := \max \left( \begin{bmatrix} 0 & 0 & 0 \\ W_4 & 0 & W_6 \end{bmatrix} \right) = 74.5 \frac{\text{lbf}}{\text{ft}}$$

$$M := \frac{w_{max} \cdot L_b^2}{8} = 2.384 \text{ ft kip}$$

$$f_b := \frac{M \cdot c}{I} = 1337.4079 \frac{\text{lbf}}{\text{in}^2}$$

$$UC_{bending} := \frac{f_b}{F'_{bE}} = 0.9517$$

if  $UC_{bending} < 1 = \text{"PASS"}$

<<<<<CHECK

"PASS"

else

"FAIL"

$$V := \frac{w_{max} \cdot L_b}{2} = 0.596 \text{ kip}$$

$$f_v := \frac{3 \cdot V}{2 \cdot b \cdot d} = 0.0644 \frac{\text{kip}}{\text{in}^2}$$

$$F'_{vE} := F_{vE} \cdot C_D \cdot C_M = 0.2716 \frac{\text{kip}}{\text{in}^2}$$

$$UC_{shear} := \frac{f_v}{F'_{vE}} = 0.2372$$

if  $UC_{shear} < 1 = \text{"PASS"}$

<<<<<CHECK

"PASS"

else

"FAIL"

## Actual Deflection and Ratio Checks

Based on Floor or Roof Live Loads only

$$\Delta_{ST} := \frac{5 \cdot L_{LorR} \cdot L_b^4}{I \cdot E' \cdot 384} = 0.4732 \text{ in} \quad \text{Ratio} := \frac{L_b}{\Delta_{ST}} = 405.7743 \quad \text{if Ratio} > 360 = \text{"PASS"} \quad \text{<<<<<CHECK}$$

"PASS"

else

"FAIL"

$$\Delta_{TOT} := \frac{5 \cdot (W_1) \cdot L_b^4}{\tau \cdot E' \cdot 384} = 0.5915 \text{ in} \quad \text{Ratio} := \frac{L_b}{\Delta} = 324.6196 \quad \text{if Ratio} > 240 = \text{"PASS"} \quad \text{<<<<<CHECK}$$

"PASS"

Ratio = 360

$\Delta_{TOT}$

"PASS"  
else  
"FAIL"

Based on Snow or Wind Loads only

$$SW_{max} := \max \left( \left[ \begin{array}{c} SL \\ W \end{array} \right] \right) = 40 \frac{\text{lbf}}{\text{ft}}$$

$$\Delta_{SW} := \frac{5 \cdot SW_{max} \cdot L_b^4}{I \cdot E' \cdot 384} = 0.4732 \text{ in} \quad \text{Ratio} := \frac{L_b}{\Delta_{SW}} = 405.7743 \quad \text{if Ratio} > 360 = \text{"PASS"} \quad \text{<<<<<CHECK}$$

"PASS"

else  
"FAIL"

Total Deflection Based on outdoor deck use with snow and wind

$$SW_{max} := \max \left( \left[ \begin{array}{c} W_5 \\ W_6 \end{array} \right] \right) = 74.5 \frac{\text{lbf}}{\text{ft}}$$

$$\Delta_{TOT} := \frac{5 \cdot SW_{max} \cdot L_b^4}{I \cdot E' \cdot 384} = 0.8813 \text{ in} \quad \text{Ratio} := \frac{L_b}{\Delta_{SW}} = 405.7743 \quad \text{if Ratio} > 240 = \text{"PASS"} \quad \text{<<<<<CHECK}$$

"PASS"

else  
"FAIL"

For better floor performance and for seasoned wood used indoors:

$$D := 0.5 \cdot D = 5 \frac{\text{lbf}}{\text{ft}} \quad \text{Ratio of deflection from D+L to be less than L/360 (not 240)}$$

$$w_{L,D} := D + L = 45 \frac{\text{lbf}}{\text{ft}}$$

$$\Delta_{totL,D} := \frac{5 \cdot (w_{L,D}) \cdot L_b^4}{I \cdot E' \cdot 384} = 0.5323 \text{ in} \quad \text{Ratio} := \frac{L_b}{\Delta_{totL,D}} = 360.69 \quad \text{if Ratio} > 360 = \text{"PASS"} \quad \text{<<<<<CHECK}$$

"PASS"

else  
"FAIL"

Creep - Deflection including creep

$$K_{cr} := 2 \quad (2.0 \text{ for wet service conditions}) \text{ --Ref NDS Specifications 3.5 Bending Members}$$

Outdoor based on snow and wind

$$\Delta_{LT} := \frac{5 \cdot (W_3) \cdot L_b^4}{I \cdot E' \cdot 384} = 0.12 \text{ in}$$

$$W_5 := W_5 - W_3 \quad W_6 := W_5 - W_3$$

$$W_{MAX} := \max \left( \left[ \begin{array}{c} W_5 \\ W_6 \end{array} \right] \right) = 60 \frac{\text{lbf}}{\text{ft}}$$

$$\Delta_{ST} := \frac{5 \cdot W_{MAX} \cdot L_b^4}{I \cdot E' \cdot 384} = 0.7098 \text{ in}$$

$$\Delta_{tot} := K_{cr} \cdot \Delta_{LT} + \Delta_{ST} = 0.95 \text{ in} \quad \text{Ratio} := \frac{L_b}{\Delta_{tot}} = 202.8873 \quad \text{if Ratio} > 240 = \text{"FAIL"} \quad \text{<<<<<CHECK}$$

"PASS"

else  
"FAIL"

