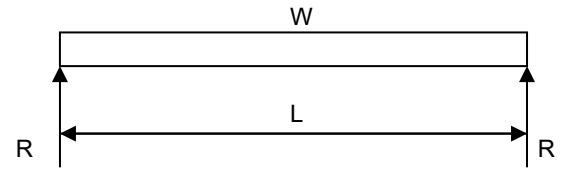


## Wood Joists and Simple Beams:

UNIT LOADS (See Design Criteria):

RDL	=	12	PSF	Roof Dead Load					
RLL	=	16	PSF	Roof Live Load	CD	=	1.25		
FDL	=	10	PSF	Floor Dead Load					
FLL	=	40	PSF	Floor Live Load	CD	=	1.00		
ODL	=	11	PSF	Other Dead Load					
OLL	=	-24	PSF	Other Live Load	CD	=	1.60	(Wind Uplift)	



Mark	FJ01	FJ02	FJ03	FJ04	RB05	RB06	RB07	RB08	RB09
Span (ft).	12.00	15.00	18.00	20.00	12.00	10.00	9.00	8.00	20.00
Ld. Type	Floor	Floor	Floor	Floor	Roof	Roof	Roof	Roof	Roof
Trib. Width (ft.)	1.33	1.33	1.33	1.33	10.00	10.00	10.00	10.00	20.00
C <sub>R</sub>	1.15	1.15	1.15	1.15	1.00	1.00	1.00	1.00	1.00
Size	2x8	2x10	2x12	2x14	(2)-2x12	(2)-2x10	(2)-2x8	(2)-2x8	8x14
Grade	#2DFL-D	#2DFL-D	#1DFL-D	#1DFL-D	#2DFL-D	#2DFL-D	#2DFL-D	#2DFL-D	#1DFL-B
E (psi)	1,600,000	1,600,000	1,600,000	1,600,000	1,600,000	1,600,000	1,600,000	1,600,000	1,600,000
Density (PCF)	32	32	32	32	32	32	32	32	32
F <sub>cperp</sub> (psi)	625	625	625	625	625	625	625	625	625
b (in.)	1.5	1.5	1.5	1.5	3.0	3.0	3.0	3.0	7.5
d (in.)	7.25	9.25	11.25	13.25	11.25	9.25	7.25	7.25	13.50
A (in <sup>2</sup> )	10.875	13.875	16.875	19.875	33.75	27.75	21.75	21.75	101.25
S (in <sup>3</sup> )	13.141	21.391	31.641	43.891	63.281	42.781	26.281	26.281	227.813
I (in <sup>4</sup> )	47.63	98.93	177.98	290.78	355.96	197.86	95.27	95.27	1,537.73
C <sub>F</sub>	1.20	1.10	1.00	0.90	1.00	1.10	1.20	1.20	0.99
Beam Wt. (PLF)	2.4	3.1	3.8	4.4	7.5	6.2	4.8	4.8	22.5
Trib. Area (ft <sup>2</sup> )	16	20	24	27	120	100	90	80	400
LL Reduction (R)	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	12.5%
W <sub>DL</sub> (PLF)	16	16	17	18	128	126	125	125	263
W <sub>LL</sub> (PLF) <sup>(1)</sup>	53	53	53	53	160	160	160	160	280
R <sub>DL</sub> (lbs.)	95	123	154	178	765	631	562	499	2,625
R <sub>LL</sub> (lbs.) <sup>(2)</sup>	320	400	480	533	960	800	720	640	3,200
R <sub>TL</sub> (lbs.)	415	523	634	711	1,725	1,431	1,282	1,139	5,825
L <sub>brng</sub> (in.)	0.44	0.56	0.68	0.76	0.92	0.76	0.68	0.61	1.24
V <sub>DL</sub> /C <sub>D</sub> (lbs.)	94	123	153	175	717	593	540	471	2,589
V <sub>L+D</sub> /C <sub>D</sub> (lbs.)	373	469	568	632	1,164	968	888	774	3,852
f <sub>v</sub> (psi)	51	51	50	48	52	52	61	53	57
F <sub>v</sub> (psi)	95	95	95	95	95	95	95	95	85
	54%	53%	53%	50%	54%	55%	64%	56%	67%
M <sub>D</sub> /C <sub>D</sub> (ft-lbs.)	315	513	769	986	2,550	1,752	1,404	1,110	14,583
M <sub>L+D</sub> /C <sub>D</sub> (ft-lbs.)	1,244	1,962	2,852	3,554	4,140	2,862	2,307	1,823	21,700
f <sub>b</sub> (psi)	1,136	1,101	1,082	972	785	803	1,053	832	1,143
F <sub>b</sub> (psi)	1,242	1,139	1,150	1,035	900	990	1,080	1,080	1,332
	91%	97%	94%	94%	87%	81%	98%	77%	86%
Δ <sub>DL</sub> (in.)	0.096	0.118	0.142	0.137	0.104	0.090	0.121	0.075	0.384
=L /	>1,000	>1,000	>1,000	>1,000	>1,000	>1,000	893	>1,000	625
Δ <sub>TL</sub> (in.)	0.423	0.502	0.584	0.550	0.236	0.203	0.276	0.172	0.794

## Wood Joists and Simple Beams:

=L /	341	359	370	436	611	590	392	557	302
$\Delta_{LL}$ (in.)	0.326	0.384	0.442	0.413	0.131	0.114	0.155	0.097	0.410
=L /	441	469	488	582	>1,000	>1,000	697	992	586
Unbraced Length:									
$L_u$ (ft.)	12.00	15.00	18.00	20.00	12.00	10.00	9.00	8.00	20.00
$L_u/d$	19.86	19.46	19.20	18.11	12.80	12.97	14.90	13.24	17.78
$L_e$ (ft.)	22.08	27.60	33.12	36.80	22.37	18.61	16.56	14.85	36.80
$K_{BE}$	0.61	0.61	0.61	0.61	0.61	0.61	0.61	0.61	0.61
$R_B$	29.22	36.90	44.58	51.00	18.32	15.15	12.65	11.98	10.29
$F_{bE}$ (psi)	1,143	717	491	375	2,908	4,252	6,097	6,798	9,209
$C_D$	1.00	1.00	1.00	1.00	1.25	1.25	1.25	1.25	1.25
$F_b^*$ (psi)	1,714	1,440	1,323	1,071	1,125	1,361	1,620	1,620	1,644
$C_L$	0.617	0.476	0.361	0.341	0.971	0.978	0.983	0.985	0.989
$F'_b$ (psi)	1,058	686	478	366	1,092	1,331	1,592	1,596	1,626
$f_b$ (psi)	1,136	1,101	1,082	972	981	1,003	1,317	1,040	1,429
	107%	161%	226%	266%	90%	75%	83%	65%	88%

<sup>(1)</sup> Reduced for T.A.

<sup>(2)</sup> Not Reduced for T.A.

Alternate TJI:	22" L65	18" L65	14" L65	11.875" L65	11.875" L65	11.875" L65	11.875" L65	11.875" L65	28" H90
Quantity	1	1	1	1	1	1	1	1	1
$M_{cap}$ (ft.-lbs.)	12,245	9,835	7,655	6,260	6,260	6,260	6,260	6,260	24,580
$V_{cap}$ (lbs.)	2,935	2,535	2,125	1,925	1,925	1,925	1,925	1,925	2,900
$EI$ (lb-in <sup>2</sup> ) x 10 <sup>6</sup>	1,967	1,227	678	459	459	459	459	459	5,279
$\Delta_{DL}$ (in.)	0.004	0.015	0.060	0.139	0.130	0.062	0.040	0.025	0.179
=L /	>1,000	>1,000	>1,000	>1,000	>1,000	>1,000	>1,000	>1,000	>1,000
$\Delta_{TL}$ (in.)	0.016	0.065	0.245	0.558	0.292	0.140	0.092	0.057	0.370
=L /	>1,000	>1,000	825	428	>1,000	>1,000	>1,000	>1,000	>1,000
$\Delta_{LL}$ (in.)	0.013	0.050	0.186	0.418	0.163	0.078	0.051	0.032	0.191
=L /	>1,000	>1,000	>1,000	570	>1,000	>1,000	>1,000	>1,000	>1,000