

# INTERACTION DIAGRAMS FOR SQUARE AND RECTANGULAR COLUMNS

## INPUT VALUES



Material Strengths =

$$f_y := 60$$

$$f_c := 3$$

$$\beta_1 := \text{if}[f_c \leq 4, 0.85, \text{if}[f_c \geq 8, 0.65, 0.85 - (f_c - 4) \cdot 0.05]] \quad \beta_1 = 0.85$$

Longitudinal column dimension =

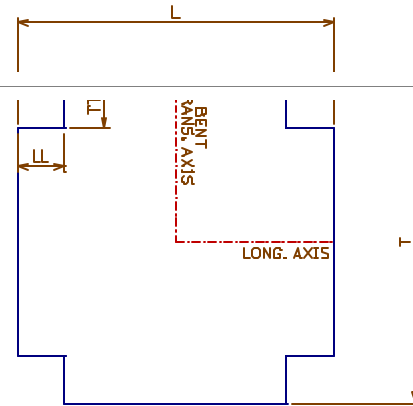
$$L := 60$$

$$LL := 9$$

Transverse column dimension =

$$T := 60$$

$$TT := 9$$



SIZE OF BAR; AREA OF ONE BAR; AND NUMBER OF EACH REQUIRED FOR 1% STEEL

$$EA := 0, 1..5$$

$$\begin{matrix} \text{BART} := \begin{pmatrix} 7 \\ 8 \\ 9 \\ 10 \\ 11 \\ 14 \end{pmatrix} & \text{AS\_EACH} := \begin{pmatrix} 0.60 \\ 0.79 \\ 1 \\ 1.27 \\ 1.56 \\ 2.25 \end{pmatrix} & N\_REQUIRED_{EA} := \text{floor} \left[ \frac{(L \cdot T - 4 \cdot LL \cdot TT) \cdot 0.01}{AS\_EACH_{EA}} + 1 \right] & N\_REQUIRED = \begin{pmatrix} 55 \\ 42 \\ 33 \\ 26 \\ 22 \\ 15 \end{pmatrix} \end{matrix}$$

Note: Program will assume that the bars are spaced evenly and determine the number of bars in each face. If you have a different configuration you may change the configuration manually. This change should be made on page 3.

Number of bars =

$$nbars := 36$$

Size of bars =

$$sbars := 11$$

$$in := sbars - 4$$

Diameter of bars used (size 4 to 11 supported)

$$dia := \text{info}_{in, 2}$$

$$dia = 1.27$$

Distance to CL. reinforcing =

$$cover := 3.0$$

$$cover = 3$$



As of one bar =

$$Asone := \text{info}_{in, 1}$$

$$Asone = 1.27$$

The user may manually change the number of bars in each face here.

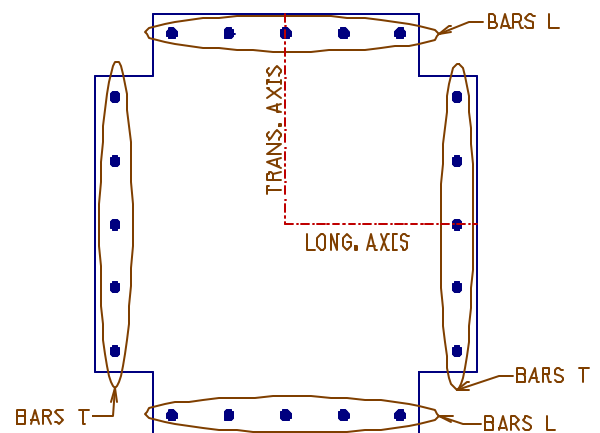
$$barsL = 9$$

$$bspaceL = 4.5$$

CHECK

$$barsT = 9$$

$$bspaceT = 4.5$$



Modulus for steel=

$$E_s := 29000$$

Max strain is steel =

$$\epsilon_{\max} := \frac{f_y}{E_s} \quad \epsilon_{\max} = 0.002069$$

Number of iteration points =  
(accuracy of the curve)

$$ip := 200$$

Capacity Reduction Factor =  
see 8.16.1.2.2

$$\phi_{\min} := 0.70$$

$$\phi_{\max} := 0.90$$

Capacity reduction factor for concrete (usually 0.85) =

$$\phi_{\text{concrete}} := 0.85$$

Capacity reduction factor for steel (usually 0.90) =

$$\phi_{\text{steel}} := 0.90$$

Theoretical max strain in concrete (ACI suggests 0.003) =

$$\epsilon_{\max \text{conc}} := 0.003$$

APPLIED LOADS

Pu (k)MuT (k\*ft)MuL (k\*ft)

Count the number of data points

```
G:=
| for j3 ∈ 0.. 100
|   F ← 0 on error
|   B ← j3 if Zj3,0 > 0
|   break otherwise
| B
```

G = 4

n := 0.. G

Z =

	0	1	2
0	1415	1189	4023
1	1300	951.9	3103
2	910.6	1612	548.9
3	1043	164.4	0
4	1300	1737	3400
5	0	0	0
6	0	0	0
7	0	0	0
8	0	0	0
9	0	0	0
10	0	0	0
11	0	0	0
12	0	0	0
13	0	0	0
14	0	0	0
15	0	0	0

PL := Z<sup><0></sup>

MT := Z<sup><1></sup>

ML := Z<sup><2></sup>

PT := Z<sup><0></sup>



Capacity reduction factors

$\phi_{min} = 0.7$                        $\phi_{max} = 0.9$

Max tension force =

$P_t := -P_{ten} \cdot \phi_{max}$                        $P_t = -2221.992$

$barsL = 9$

Max compression force =

$P_c := P_{comp} \cdot \phi_{min}$                        $P_c = 7494.266$

$barsT = 9$

$L = 60$

$f_c = 3$

$sbars = 11$

$T = 60$

$A_{st} = 45.72$

$nbars = 36$

$cover = 3$

$A_g = 3276$

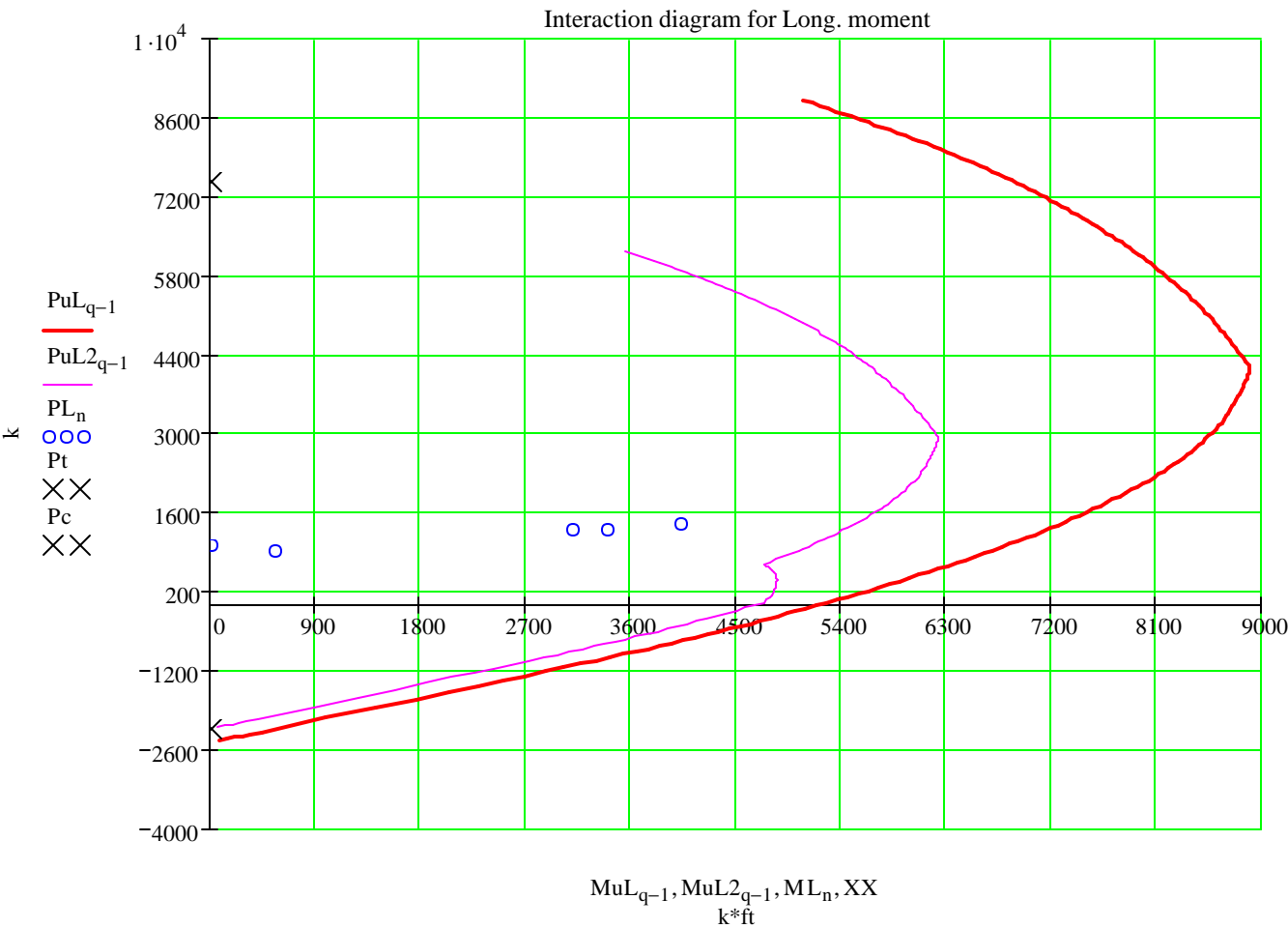
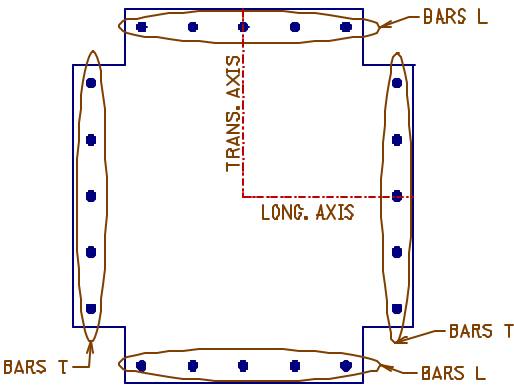
$f_y = 60$

$barsL = 9$

$barsT = 9$

Percent steel =

$\%steel := 100 \cdot \frac{A_{st}}{A_g}$                        $\%steel = 1.396$



**SAFETY FACTOR**

$\min(\text{SF}) = 1.374$

$$\min \left( \begin{pmatrix} \text{SF1}_n \\ \text{SF2}_n \end{pmatrix} \right) =$$

1.374
1.747
8.23
7.185
1.595



Capacity reduction factors

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$P_t = -2221.992$

$barsT = 9$

Max compression force =

$P_c := P_{comp} \cdot \phi_{min}$

$P_c = 7494.266$

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$L = 60$

$T = 60$

$cover = 3$

$barsL = 9$

$f_c = 3$

$A_{st} = 45.72$

$A_g = 3276$

$barsT = 9$

$sbars = 11$

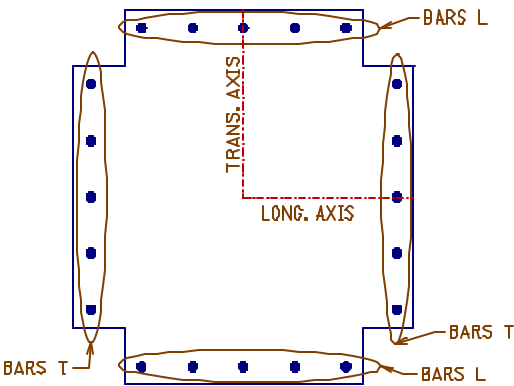
$nbars = 36$

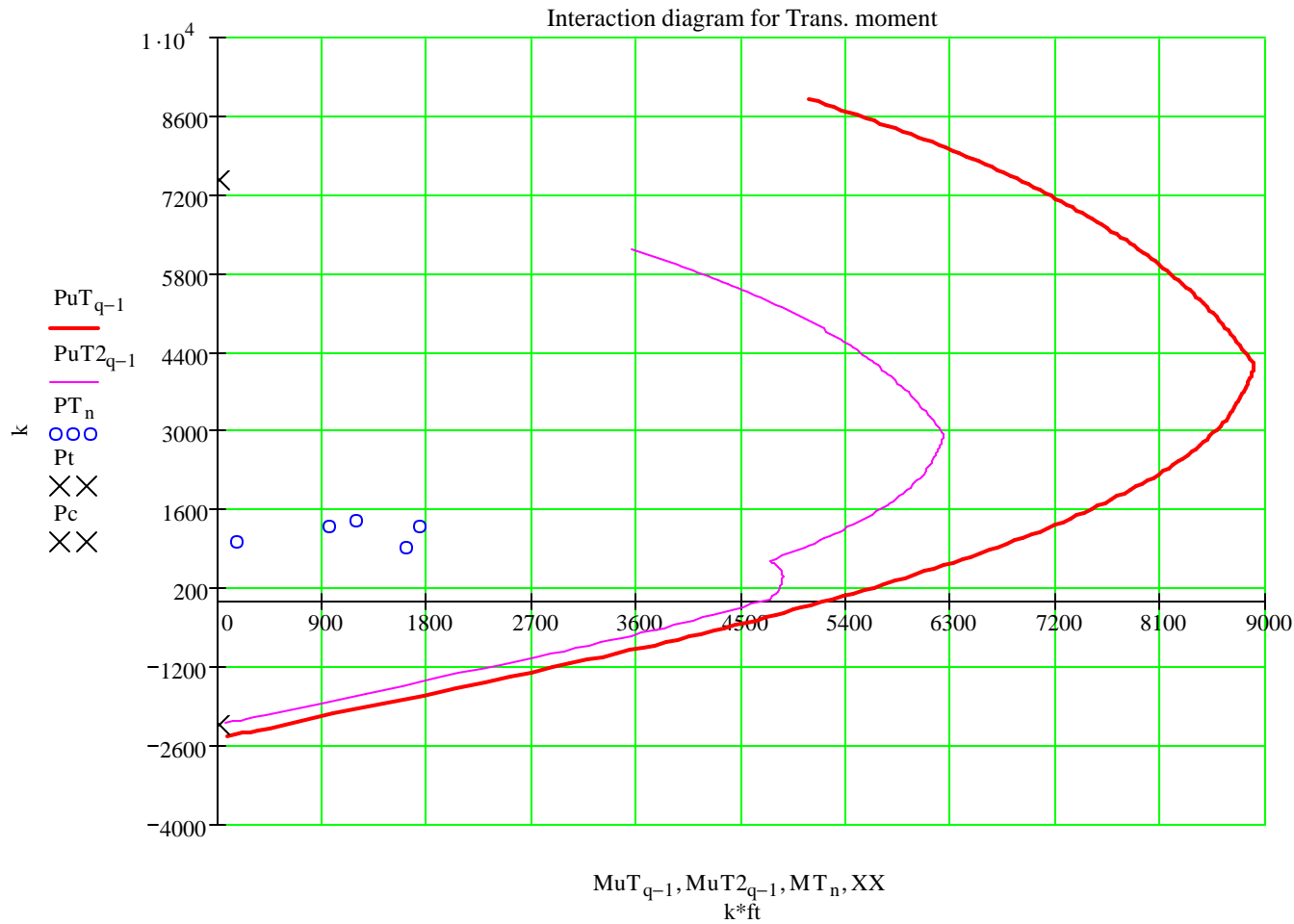
$f_y = 60$

Percent steel =

$\%steel := 100 \cdot \frac{A_{st}}{A_g}$

$\%steel = 1.396$





**SAFETY FACTOR**

$\min(SF) = 3.107$

$$\min \left( \begin{pmatrix} SF1_n \\ SF2_n \end{pmatrix} \right) =$$

4.649
5.696
3.107
7.185
3.121