

# ETABS Steel Frame Design

## AISC 360-16 Steel Section Check (Strength Summary)

### Element Details

Level	Element	Unique Name	Location (in)	Combo	Element Type	Section	Classification
Story2	D146	331	211.2569	UDStIS32	Ordinary Concentrically Braced Frame	HSS5X5X5/16	Non-Compact

### LLRF and Demand/Capacity Ratio

L (in)	LLRF	Stress Ratio Limit
422.3276	1	0.95

### Analysis and Design Parameters

Provision	Analysis	2nd Order	Reduction
ASD	Direct Analysis	General 2nd Order	Tau-b Fixed

### Stiffness Reduction Factors

$\alpha P_r / P_y$	$\alpha P_r / P_e$	$\tau_b$	EA factor	EI factor
0.003	0.005	1	0.8	0.8

### Design Code Parameters

$\Omega_b$	$\Omega_c$	$\Omega_{TY}$	$\Omega_{TF}$	$\Omega_v$	$\Omega_{V-RI}$	$\Omega_{VT}$
1.67	1.67	1.67	2	1.67	1.5	1.5

### Section Properties

A (in <sup>2</sup> )	J (in <sup>4</sup> )	I <sub>33</sub> (in <sup>4</sup> )	I <sub>22</sub> (in <sup>4</sup> )	A <sub>v3</sub> (in <sup>2</sup> )	A <sub>v2</sub> (in <sup>2</sup> )
5.26	31.2	19	19	2.4	2.4

### Design Properties

S <sub>33</sub> (in <sup>3</sup> )	S <sub>22</sub> (in <sup>3</sup> )	Z <sub>33</sub> (in <sup>3</sup> )	Z <sub>22</sub> (in <sup>3</sup> )	r <sub>33</sub> (in)	r <sub>22</sub> (in)	C <sub>w</sub> (in <sup>6</sup> )
7.6	7.6	9.16	9.16	1.9006	1.9006	

### Material Properties

E (lb/in <sup>2</sup> )	f <sub>y</sub> (lb/in <sup>2</sup> )	R <sub>y</sub>	C <sub>pr</sub>	$\alpha$
29000000	46000	1.1	1.4	NA

### HSS Section Parameters

HSS Welding	Reduce HSS Thickness?
ERW	No

### Stress Check forces and Moments

Location (in)	P <sub>r</sub> (kip)	M <sub>r33</sub> (kip-ft)	M <sub>r22</sub> (kip-ft)	V <sub>r2</sub> (kip)	V <sub>r3</sub> (kip)	T <sub>r</sub> (kip-ft)
211.2569	-0.392	-0.0247	-0.0002	3.568	4.499E-05	-0.0066

### Axial Force & Biaxial Moment Design Factors (H1-1b)

	L Factor	K <sub>1</sub>	K <sub>2</sub>	B <sub>1</sub>	B <sub>2</sub>	C <sub>m</sub>
Major Bending	0.5	1	1	1	1	1
Minor Bending	0.5	1	1	1	1	1

### Parameters for Lateral Torsion Buckling

L <sub>ltb</sub>	K <sub>ltb</sub>	C <sub>b</sub>
0.5	1	1.313

### Demand/Capacity (D/C) Ratio Eqn.(H1-1b)

D/C Ratio =	$(P_r/2P_c) + (M_{r33}/M_{c33}) + (M_{r22}/M_{c22})$
0.004 =	0.003 + 0.001 + 9.234E-06

### Axial Force and Capacities

P <sub>r</sub> Force (kip)	P <sub>nc</sub> /Ω (kip)	P <sub>nt</sub> /Ω (kip)
0.392	63.153	144.886

### Moments and Capacities

	M <sub>r</sub> Moment (kip-ft)	M <sub>n</sub> /Ω (kip-ft)	M <sub>n</sub> /Ω No LTB (kip-ft)	M <sub>n</sub> /Ω Cb=1 (kip-ft)
Major Bending	0.0247	21.0259	21.0259	21.0259
Minor Bending	0.0002	21.0259		

### Torsion Moment and Capacities

T <sub>r</sub> Moment (kip-ft)	T <sub>n</sub> Capacity (kip-ft)	T <sub>n</sub> /Ω Capacity (kip-ft)
-0.0066	29.4641	17.6432

### Shear Design

	V <sub>r</sub> Force (kip)	V <sub>n</sub> /Ω (kip)	Stress Ratio
Major Shear	3.568	39.696	0.09
Minor Shear	4.499E-05	39.696	0

### End Reaction Axial Forces

Left End Reaction (kip)	Load Combo	Right End Reaction (kip)	Load Combo
-13.526	UDStIS48	-13.526	UDStIS32