

deflection due to girder & diaphragm weights

deflections due to concrete deadload (slab) and superimposed dead loads

typo; should be 0.9

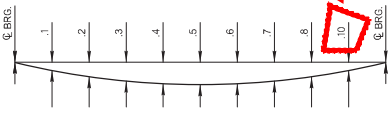
DEFLECTION SCHEDULE - GIRDERS 1 - 6												
BEAM AND DIAPHRAGM DEFLECTION					SLAB, HAUNCH, S.I.P. STEEL DECK FORMS AND TRAFFIC RAIL DEFLECTION							
SPAN	CL. BRG.	1 & 9	2 & 8	3 & 7	4 & 6	0.5	CL. BRG.	1 & 9	2 & 8	3 & 7	4 & 6	0.5
1	0.00"	0.02"	0.03"	0.04"	0.06"	0.06"	0.00"	0.06"	0.12"	0.16"	0.19"	0.20"

DEFLECTION SCHEDULE - GIRDERS 1 - 6										
BEAM AND DIAPHRAGM DEFLECTION										
SPAN	CL. BRG.	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9
2 & 6	0.00"	0.14"	0.25"	0.33"	0.37"	0.37"	0.32"	0.25"	0.15"	0.06"
3 & 5	0.00"	0.02"	0.00"	0.03"	0.05"	0.07"	0.07"	0.05"	0.02"	0.00"
4	0.00"	0.03"	0.08"	0.13"	0.17"	0.18"	SYM. ABOUT C BRIDGE			

DEFLECTION SCHEDULE - GIRDERS 2 - 5										
BEAM AND DIAPHRAGM DEFLECTION										
SPAN	CL. BRG.	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9
2 & 6	0.00"	0.14"	0.25"	0.34"	0.38"	0.38"	0.33"	0.25"	0.15"	0.06"
3 & 5	0.00"	0.02"	0.00"	0.03"	0.05"	0.07"	0.07"	0.05"	0.02"	0.00"
4	0.00"	0.03"	0.08"	0.13"	0.17"	0.18"	SYM. ABOUT C BRIDGE			

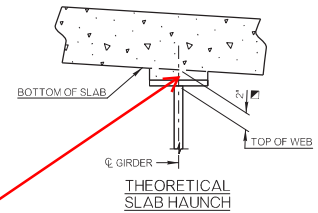
DEFLECTION SCHEDULE - GIRDERS 1 - 6										
DECK SLAB, HAUNCH, S.I.P. STEEL DECK FORMS AND TRAFFIC RAIL DEFLECTION										
SPAN	CL. BRG.	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9
2 & 6	0.00"	0.33"	0.61"	0.81"	0.90"	0.89"	0.78"	0.59"	0.36"	0.15"
3 & 5	0.00"	0.03"	0.01"	0.09"	0.16"	0.20"	0.20"	0.14"	0.07"	0.01"
4	0.00"	0.08"	0.21"	0.35"	0.45"	0.49"	SYM. ABOUT C BRIDGE			

the girder self weight deflections are broken out separately because the contractor only uses the concrete and superimposed dead load deflections to set the haunch heights.



DEAD LOAD DEFLECTION DIAGRAM

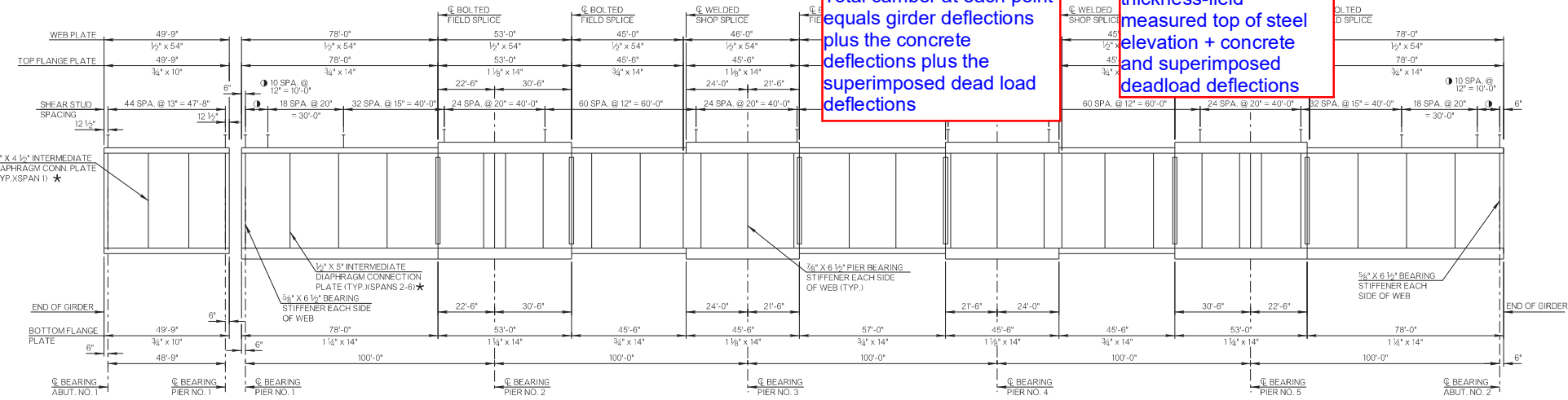
THE DEAD LOAD DEFLECTION SHOWN AT THE TENTHPOINTS ARE THE DEFLECTIONS DUE TO DECK SLAB + HAUNCH + SIP STEEL DECK FORM ALLOWANCE + CONCRETE TRAFFIC RAIL. IT DOES NOT INCLUDE THE BEAM WEIGHT, DIAPHRAGMS OR FUTURE WEAR SURFACE.



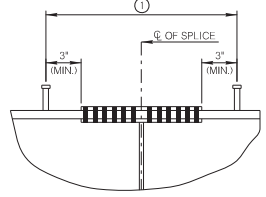
DIMENSION (BOTTOM OF SLAB TO TOP OF WEB) MAY VARY IF GIRDER CAMBER AFTER ERECTION DIFFERS FROM PLAN CAMBER BY MORE THAN 0.10". NO LOAD DEFLECTION DUE TO WEIGHT OF DECK. NO LOAD DEFLECTION DUE TO WEIGHT OF DECK. NO LOAD DEFLECTION DUE TO WEIGHT OF DECK. NO LOAD DEFLECTION DUE TO WEIGHT OF DECK.

haunch thickness = top of slab elevation - slab thickness - field elevation + concrete and superimposed dead load deflections

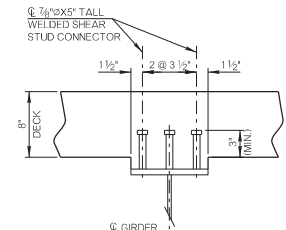
Total camber at each point equals girder deflections plus the concrete deflections plus the superimposed dead load deflections



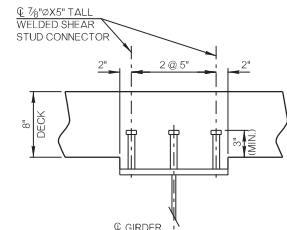
SUMMARY OF STRUCTURAL STEEL QUANTITIES	
ITEM	LBS.
AASHTO M270 GRADE 50W	
GIRDERS	602,700
TYPICAL WEB STIFFENER PLATE	9,910
BEARING STIFFENERS	6,710
SPLICE PLATES	16,080
FILL PLATES	1,620
INTERMEDIATE CROSS FRAMES	31,700
END CROSS FRAMES	10,870
TOTAL AASHTO M270 GRADE 50W	679,590
WELDED SHEAR STUD CONNECTORS	7,650



CLEARANCE AT SPLICE PLATE
 SHEAR CONNECTORS IN SPLICE PLATE REGION SHALL BE ADJUSTED BY FABRICATOR TO EACH SIDE OF CENTERLINE OF SPLICE.



10" FLANGE SHEAR STUD CONNECTOR DETAIL (SPAN 1)



14" FLANGE SHEAR STUD CONNECTOR DETAIL (SPANS 2-6)

* CONNECTION PLATES ARE ATTACHED ON BOTH SIDES OF THE WEB. OMIT CONNECTION PLATES ON EXTERIOR FACES OF GIRDERS 1 & 6. REFER TO FRAMING PLAN ON SUPERSTRUCTURE DETAILS FOR DIAPHRAGM CONNECTION PLATE SPACING.