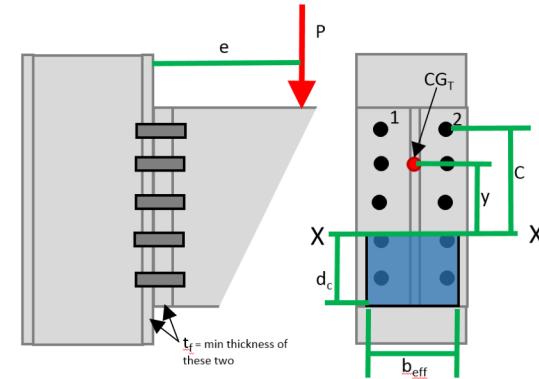


Bolted Connection Design – AISC Spec J

Combined Shear & Tension in Bearing Connections: Bracket Case I

Method of Design



Combined Shear & Tension

- AISC Sec. J3.7 – Combined V&T in Bearing Connections
- AISC Sec. J3.9 – Combined V&T in Slip Crit. Connections

When the required stress in either shear or tension is less than or equal to 30% of the available stress, the effects of combined stress need not be investigated.

Available tensile strength of bolts w/ tension & shear (R_n)

$$R_n = F'_{nt} A_b \quad \phi R_n = 0.75 R_n \text{ for LRFD}$$

$$\text{Where: } F'_{nt} = 1.3 F_{nt} - \left(\frac{F_{nt}}{\phi F_{nv}} \right) f_{rv} \leq F_{nt}$$

F_{nt} = nominal tensile stress from AISC Table J3.2

F_{nv} = nominal shear stress from AISC Table J3.2

f_{rv} = required shear stress using LRFD

A_b = nominal unthreaded body area of bolt or threaded part

AISC Ch 7

Case I (N.A. not @ C.G.) or II (N.A. @ C.G.)

AISC Ch. 7 Case I

Shear force Per Bolt:

$$r_{uv} = \frac{P_u}{n}$$

Tension per Bolt:

1) Estimate NA @ $d_c = \text{depth}/6$

2) Establish horizontal size of compression block

$$b_{eff} = 8t_f \leq b_f$$

3) Verify by taking moment about proposed NA

$$(\sum A_b)y = b_{eff}d_c \left(\frac{d_c}{2} \right) \text{ Adjust NA accordingly}$$

4) Establish I_x

$$I_x = A_b (\sum d_y)^2 + \frac{b_{eff}(d_c)^3}{3} \text{ Where: } d_y = \text{bolt distance to NA}$$

5) Tensile force in farthest bolt

$$r_{ut} = \left(\frac{P_u e c}{I_x} \right) A_b$$

Shear & Tensile Stress in Bolts at 30% Threshold?:

$$f_{rv} = \frac{r_{uv}}{A_b}$$

$$f_t = \frac{r_{ut}}{A_b}$$

Is $f_{rv} \geq 0.3 \phi F_{nv}$?

$\phi = 0.75$ for both cases

Is $f_t \geq 0.3 \phi F_{nt}$?

If either exceed 30% → Consider combined V&T.

If not → Done.

Available Tensile Strength (Revised for Combined V&T):

$$F'_{nt} = 1.3 F_{nt} - \left(\frac{F_{nt}}{\phi F_{nv}} \right) f_{rv} \leq F_{nt}$$

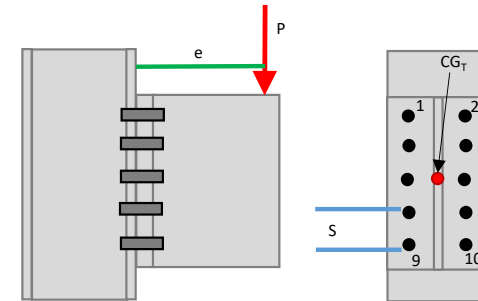
New Design Tension Strength:

$$\phi R_n = 0.75 F'_{nt} A_b$$

$$f_t(A_b) < \phi R_n \rightarrow \text{Good}$$

Bolted Connection Design – AISC Spec J

Combined Shear & Tension in Bearing Connections: Bracket Case II Method of Design



Combined Shear & Tension

- AISC Sec. J3.7 – Combined V&T in Bearing Connections
- AISC Sec. J3.9 – Combined V&T in Slip Crit. Connections

When the required stress in either shear or tension is less than or equal to 30% of the available stress, the effects of combined stress need not be investigated.

Available tensile strength of bolts w/ tension & shear (R_n)
 $R_n = F'_{nt} A_b$ $\phi R_n = 0.75 R_n$ for LRFD
 Where: $F'_{nt} = 1.3 F_{nt} - \left(\frac{F_{nt}}{\phi F_{nv}} \right) f_{rv} \leq F_{nt}$
 F_{nt} = nominal tensile stress from AISC Table J3.2
 F_{nv} = nominal shear stress from AISC Table J3.2
 f_{rv} = required shear stress using LRFD
 A_b = nominal unthreaded body area of bolt or threaded part

AISC Ch 7
 Case I (N.A. not @ C.G) or II (N.A. @ C.G.)

AISC Ch. 7 Case II

Shear force Per Bolt:
 $r_{uv} = \frac{P_u}{n}$

Moment Effect per Bolt:

- $M_u = P_u e$
- There is no compression block. Bolt get “compressive load”. Not exceeding clamping force.
- Establish I_x
 $I_x = A_b (\sum d_y)^2$ Where: d_y = bolt distance to NA
- Tensile force in worst case bolts
 $r_{ut} = \left(\frac{P_u e c}{I_x} \right) A_b$

Shear & Tensile Stress in Bolts at 30% Threshold?:
 $f_{rv} = \frac{r_{uv}}{A_b}$ $f_t = \frac{r_{ut}}{A_b}$
 Is $f_{rv} \geq 0.3 \phi F_{nv}$? $\phi = 0.75$ for both cases
 Is $f_t \geq 0.3 \phi F_{nt}$?
 If either exceed 30% → Consider combined V&T.
 If not → Done.

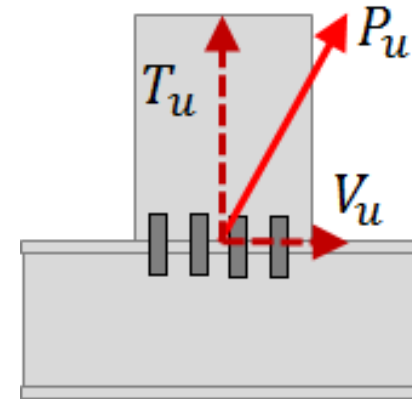
Available Tensile Strength (Revised for Combined V&T):
 $F'_{nt} = 1.3 F_{nt} - \left(\frac{F_{nt}}{\phi F_{nv}} \right) f_{rv} \leq F_{nt}$

New Design Tension Strength:
 $\phi R_n = 0.75 F'_{nt} A_b$

$f_t(A_b) < \phi R_n \rightarrow$ Good

Bolted Connection Design – AISC Spec J

Combined Shear & Tension in Slip Critical Connections



Combined Shear & Tension

- AISC Sec. J3.7 – Combined V&T in Bearing Connections
- AISC Sec. J3.9 – Combined V&T in Slip Crit. Connections

When a slip-critical connection is subject to tension that reduces the net clamping force, the available slip resistance per bolt shall be multiplied by the factor k_{sc}

- $k_{sc} = 1 - \frac{T_u}{D_u T_b n_b}$ (LRFD)
 - Where: T_u = required tension force using LRFD
 - n_b = number of bolts carrying the applied tension
 - $D_u = 1.13$
 - T_b = min. fastener tension AISC Table J3.1

The Reduction Factor Depends on the Holes

- For Standard & short-slotted holes perpendicular to load direction
 - $\phi = 1.00 R_n$
- For Oversized & short-slotted holes parallel to load direction
 - $\phi = 0.85 R_n$
- For Long-Slotted Holes
 - $\phi = 0.70 R_n$
- Finger shims up to 1/4" are allowed per AISC J3.2.

Part 1: Identify Design Loads

Identify Initial Parameters

$\mu, D_u, h_f, T_b, n_s, \phi$ & n_b

Convert P_u Into Shear (T_u) & Tension (V_u) Vectors

Available Tensile Bolt Strength (AISC J3.6)

$R_n = F_{nt} A_b$ with $\phi = 0.75$ (J3.6) F_{nt} (Table J3.2)

Just use AISC Table 7-2

Check: $\phi R_n > \frac{T_u}{\#bolts}$

Available Slip Resistance Per Bolt (if T_u were 0)

$\phi R_{n1bolt} = \phi \mu D_u h_f T_b n_s$ (see previous slide)

Available Slip Resistance of the Connection

- Include Reduction Factor k_{sc} w/ $P_{ut} = T_u$

$$k_{sc} = 1 - \frac{T_u}{D_u T_b n_b}$$

$$\phi R_{n(connection)} = (\phi R_{n1bolt})(k_{sc})(n_b)$$

Check: $(\phi R_{n1bolt})(k_{sc})(n_b) > V_u$

CHECK ALL ADDITIONAL LIMIT STATES FROM PREVIOUS SLIDE!!!!