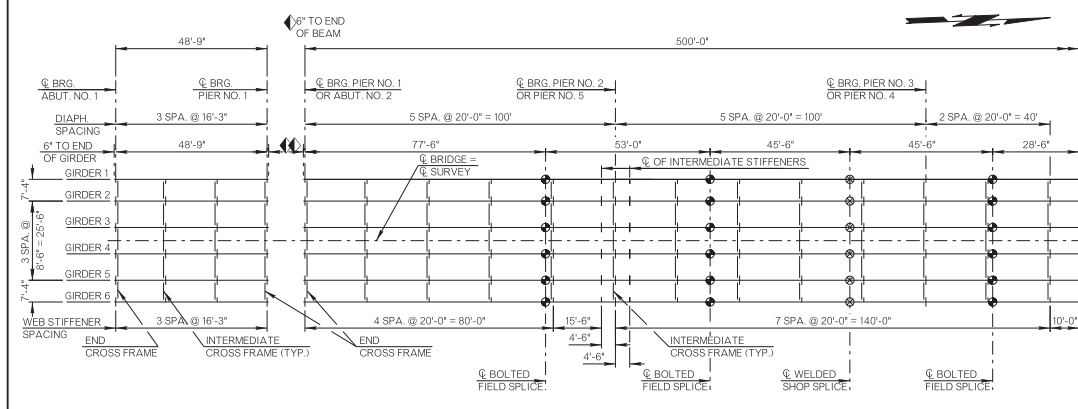


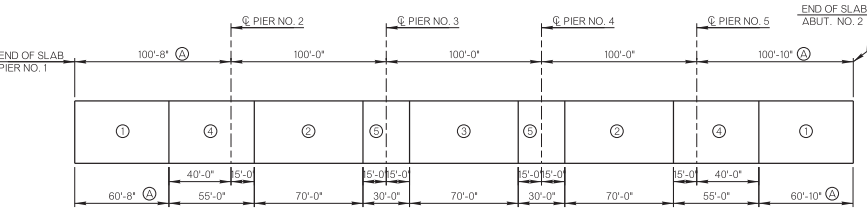
OKLAHOMA DEPARTMENT OF TRANSPORTATION					
DESIGN NO.	STATE	PROJECT NO.	SECTION NO.	SHEET NO.	TOTAL SHEETS
6	OKLA.				
DESCRIPTION		REVISIONS	DATE		



LEGEND

● = B.F.S. = BOLTED FIELD SPLICE
 ⊗ = W.S.S. = WELDED SHOP SPLICE

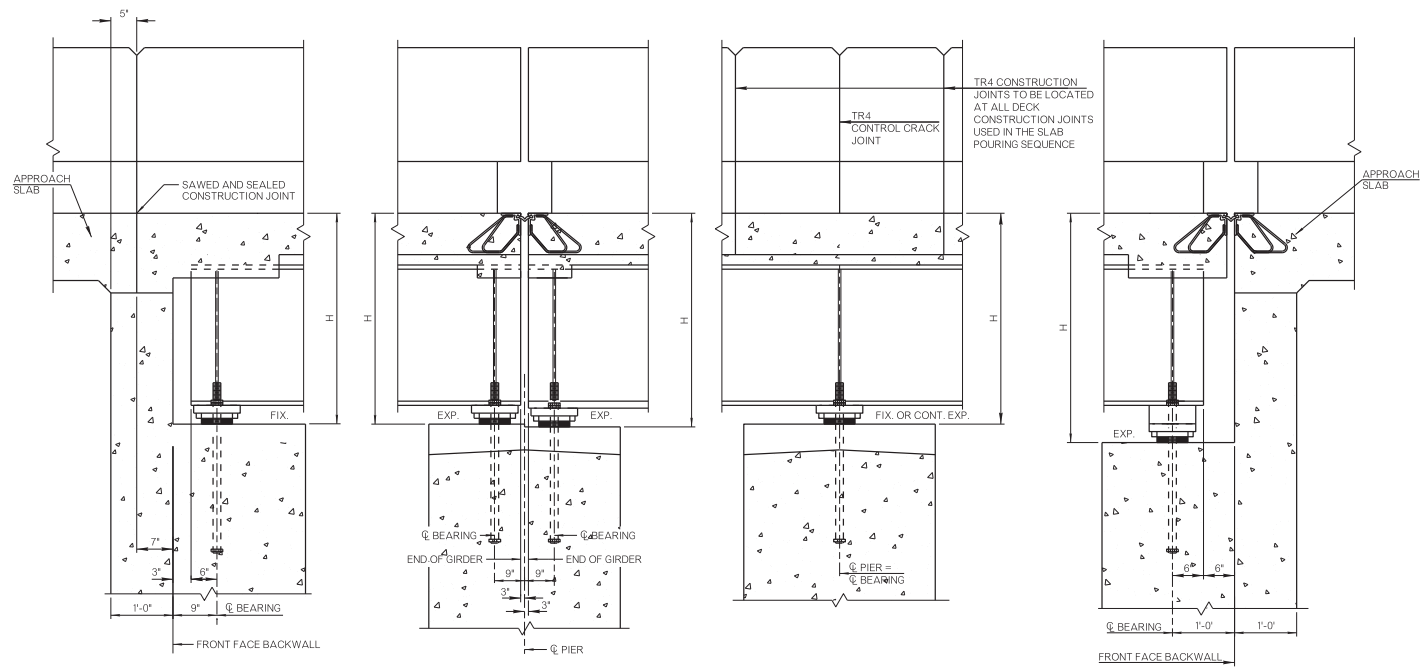
NOTE:
 ⊗ OF WELDED SHOP SPLICE IS REFERENCED TO SPLICE IN FLANGES. SEE "GIRDER DETAILS" SHEET.



Ⓐ THESE DIMENSIONS ARE CALCULATED USING A 2" EXPANSION JOINT OPENING. DIMENSIONS MAY VARY DUE TO TEMPERATURE AT TIME OF POURING.

NOTE:
 DO NOT TIE WITHIN 6" OF CONSTRUCTION JOINTS.

SLAB POURING SEQUENCE:
 THE POURING SEQUENCE SHALL BE IN THE NUMERICAL SEQUENCE INDICATED. ALL POURS WITH THE SAME NUMBER MAY BE Poured IN ANY SEQUENCE, BUT ALL POURS WITH SAME NUMBER SHALL BE COMPLETED BEFORE BEGINNING WITH THE NEXT POUR NUMBER. THERE SHALL BE A LAISEZ OF AT LEAST 48 HOURS BETWEEN POUR PHASES. EACH PHASE OF SUPERSTRUCTURE DECK CONCRETE PLACEMENT SHALL BE MADE IN ONE POUR, EACH POUR SHALL BE AT A MINIMUM RATE OF 25 C.Y. PER HOUR. IN THE EVENT OF AN EMERGENCY SITUATION, A KEVED CONSTRUCTION JOINT SHALL BE MADE PERPENDICULAR TO THE DIRECTION OF TRAFFIC, AS DIRECTED BY THE ENGINEER. AFTER AT LEAST 48 HOURS HAS ELAPSED SINCE CONCRETE PLACEMENT AND UNTIL THE SLAB IS IN PLACE ON BOTH SIDES OF THE CONSTRUCTION JOINT, THE SLAB IS UNSUPPORTED AND NO HEAVY EQUIPMENT WILL BE PERMITTED ON THE FINISHED SLAB WITHIN 5' OF THE CONSTRUCTION JOINTS.



ABUTMENT NO. 1 PIER NO. 1 PIER NOS. 2 & 5 (CONT. EXP.) PIER NOS. 3 & 4 (FIX.) ABUTMENT NO. 2

LOCATION	DIMENSION
ABUTMENT 1	5' - 10 3/8"
PIER 1 - SPAN 1	5' - 10 3/8"
PIER 1 - SPAN 2	5' - 10 7/8"
PIER 2	5' - 10 7/8"
PIER 3	5' - 10 3/4"
PIER 4	5' - 10 3/4"
PIER 5	5' - 10 7/8"
ABUTMENT 2	6' - 1 7/8"

DIMENSION "H" IS FROM TOP OF DECK SLAB TO BOTTOM OF BEARING ASSEMBLY AT G BEARING.

NOTE:
 2" OPENING AT 60° FOR JOINTS AND BARRIERS (SEE STD. E.J.-SQ FOR MORE INFORMATION)

LOCHNER	
DESIGN	PRY 4/15
DRAWN	PRY 4/15
CHECKED	MLJ 1/16
FILE NAME	Long

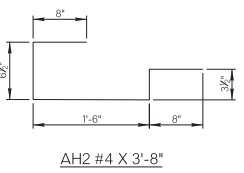
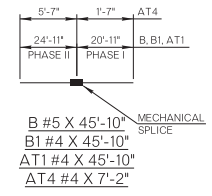
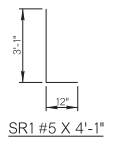
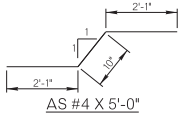
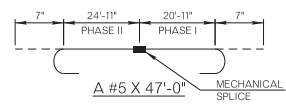
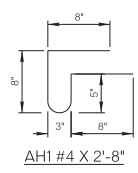
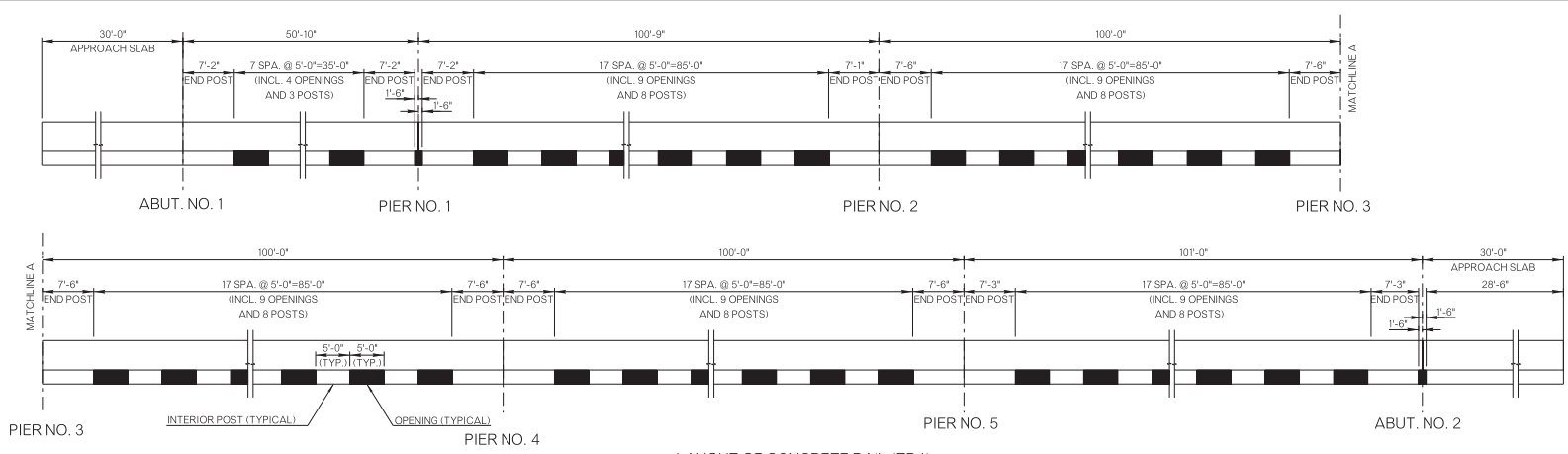
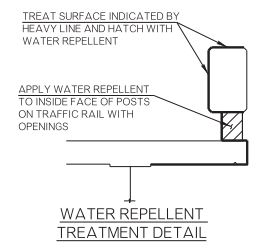
OKLAHOMA DEPARTMENT OF TRANSPORTATION

LONGITUDINAL SECTION AND FRAMING PLAN

STATE JOB NO. 23966704 SHEET NO. B013
 GARVIN COUNTY US-77

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OKLAHOMA DEPARTMENT OF TRANSPORTATION					
DESIGN NO.	STATE	JOB PROJECT NO.	PROJECT USER	SHEET NO.	TOTAL SHEETS
6	OKLA				
DESCRIPTION		REVISIONS	DATE		

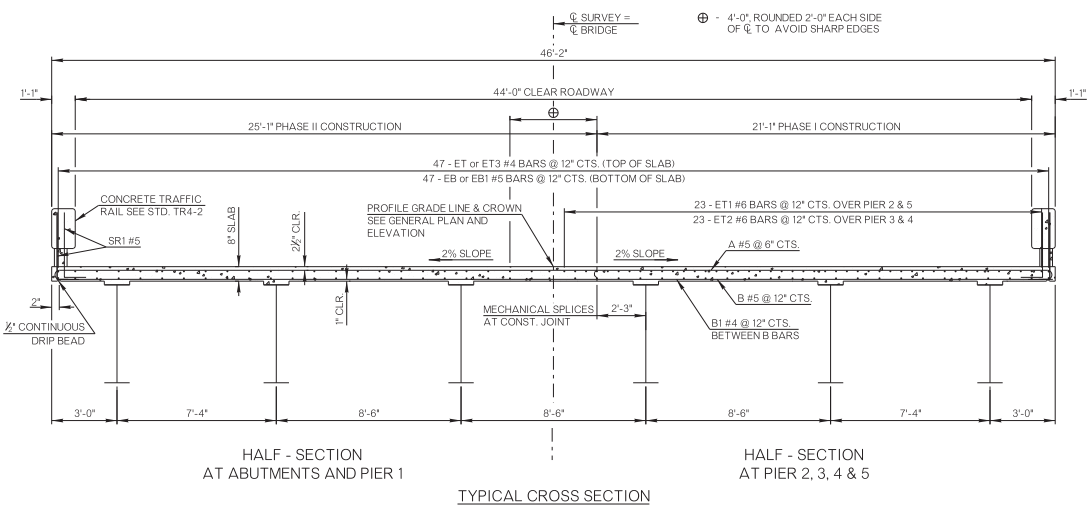


BAR LIST - SUPERSTRUCTURE					
EPOXY COATED					
SPAN 1					
MARK	NO.	SIZE	FORM	LENGTH	VARIANCE
A	102	#5	BNT	47'-0"	
B	54	#5	STR	45'-10"	
B1	50	#4	STR	45'-10"	
EB1	47	#5	STR	50'-5"	
ET3	47	#4	STR	50'-5"	
SR1	106	#5	BNT	4'-1"	

BAR LIST - SUPERSTRUCTURE					
EPOXY COATED					
SPANS 2-6					
MARK	NO.	SIZE	FORM	LENGTH	VARIANCE
A	1003	#5	BNT	47'-0"	
B	506	#5	STR	45'-10"	
B1	501	#4	STR	45'-10"	
EB	423	#5	STR	52'-6"	
ET	423	#4	STR	51'-3"	
ET1	92	#6	STR	58'-4"	
ET2	92	#6	STR	51'-11"	
SR1	2076	#5	BNT	4'-1"	

LONGITUDINAL PIER BAR LOCATIONS				
MARK	SIZE	PIER	START	END
#6	ET1	2	25'-8" SOUTH OF CL PIER	32'-8" NORTH OF CL PIER
#6	ET2	3	27'-1" SOUTH OF CL PIER	24'-10" NORTH OF CL PIER
#6	ET2	4	24'-10" SOUTH OF CL PIER	27'-1" NORTH OF CL PIER
#6	ET1	5	32'-8" SOUTH OF CL PIER	25'-8" NORTH OF CL PIER

SUPERSTRUCTURE QUANTITIES			
ITEM	UNIT	TOTAL	
SAW-CUT GROOVING	S.Y.	2,994.1	
SEALED EXPANSION JOINT	L.F.	94.3	
CONCRETE RAIL (TR4)	L.F.	1,224.5	
CLASS AA CONCRETE	C.Y.	648.3	
MECHANICAL SPLICES	EA.	2,224	
EPOXY-COATED REINFORCING STEEL (GR. 60)	L.B.	169,260	
WATER REPELLENT (VISUALLY INSPECTED)	S.Y.	981	



BAR LIST - DIAPHRAGMS					
EPOXY COATED					
MARK	NO.	SIZE	FORM	LENGTH	VARIANCE
AH1	12	#4	BNT	2'-8"	
AH2	170	#4	BNT	3'-8"	
AS	47	#4	BNT	5'-0"	
AT1	1	#4	STR	45'-10"	
AT2	14	#4	STR	6'-1"	
AT3	14	#4	STR	7'-3"	
AT4	7	#4	STR	7'-2"	
PT1	12	#4	STR	2'-1"	

- INCLUDES 8 LAPS 3'-0" LONG EACH
- INCLUDES 8 LAPS 2'-0" LONG EACH
- PHASE BAR WITH MECHANICAL SPLICE. SEE BAR DETAILS.

LOCHNER		OKLAHOMA DEPARTMENT OF TRANSPORTATION	
DESIGN	PRY	4/15	
DRAWN	RCL	4/15	
CHECKED	KLW	12/15	
FILE NAME	Typ		

TYPICAL SECTION & CONCRETE TRAFFIC RAIL

STATE JOB NO. 29967(04) SHEET NO. B014

GARVIN COUNTY US-77

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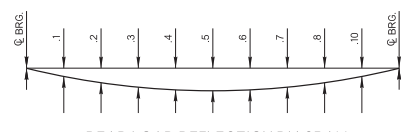
DEFLECTION SCHEDULE - GIRDERS 1 - 6												
SPAN	BEAM AND DIAPHRAGM DEFLECTION						SLAB, HAUNCH, S.I.P. STEEL DECK FORMS AND TRAFFIC RAIL DEFLECTION ②					
	CL. BRG.	1 & 9	2 & 8	3 & 7	4 & 6	0.5	CL. BRG.	1 & 9	2 & 8	3 & 7	4 & 6	0.5
1	0.00"	0.02"	0.03"	0.04"	0.05"	0.06"	0.00"	0.06"	0.12"	0.16"	0.19"	0.20"

DEFLECTION SCHEDULE - GIRDERS 1 - 6										
SPAN	BEAM AND DIAPHRAGM DEFLECTION									
	CL. BRG.	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9
2 & 6	0.00"	0.14"	0.25"	0.33"	0.37"	0.37"	0.32"	0.25"	0.15"	0.06"
3 & 5	0.00"	0.02"	0.00"	0.03"	0.05"	0.07"	0.07"	0.05"	0.02"	0.00"
4	0.00"	0.03"	0.06"	0.13"	0.17"	0.19"	SYM. ABOUT @ BRIDGE			

DEFLECTION SCHEDULE - GIRDERS 1 - 6											
SPAN	DECK SLAB, HAUNCH, S.I.P. STEEL DECK FORMS AND TRAFFIC RAIL DEFLECTION ②										
	CL. BRG.	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	
2 & 6	0.00"	0.33"	0.61"	0.81"	0.90"	0.89"	0.78"	0.59"	0.36"	0.15"	
3 & 5	0.00"	0.01"	0.01"	0.09"	0.16"	0.20"	0.20"	0.14"	0.07"	0.01"	
4	0.00"	0.08"	0.21"	0.35"	0.45"	0.49"	SYM. ABOUT @ BRIDGE				

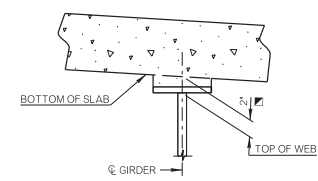
DEFLECTION SCHEDULE - GIRDERS 2 - 5										
SPAN	BEAM AND DIAPHRAGM DEFLECTION									
	CL. BRG.	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9
2 & 6	0.00"	0.14"	0.25"	0.34"	0.38"	0.38"	0.33"	0.25"	0.15"	0.06"
3 & 5	0.00"	0.02"	0.00"	0.03"	0.06"	0.08"	0.07"	0.05"	0.03"	0.00"
4	0.00"	0.03"	0.06"	0.14"	0.19"	0.19"	SYM. ABOUT @ BRIDGE			

DEFLECTION SCHEDULE - GIRDERS 1 - 6											
SPAN	DECK SLAB, HAUNCH, S.I.P. STEEL DECK FORMS AND TRAFFIC RAIL DEFLECTION ②										
	CL. BRG.	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	
2 & 6	0.00"	0.42"	0.77"	1.02"	1.14"	1.13"	0.99"	0.75"	0.46"	0.19"	
3 & 5	0.00"	0.04"	0.01"	0.11"	0.20"	0.25"	0.24"	0.18"	0.09"	0.01"	
4	0.00"	0.10"	0.27"	0.44"	0.56"	0.61"	SYM. ABOUT @ BRIDGE				



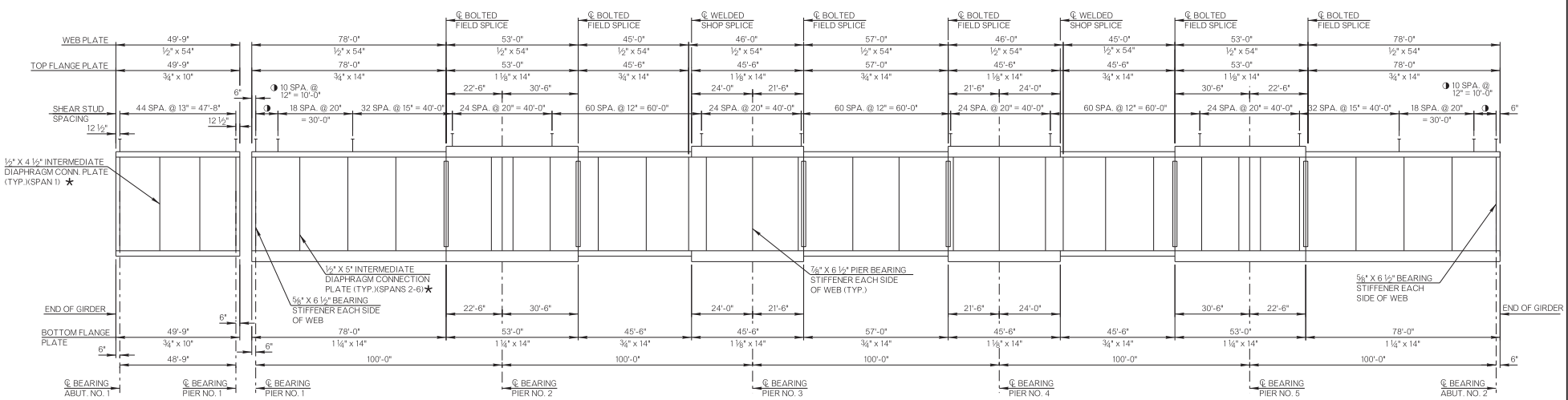
DEAD LOAD DEFLECTION DIAGRAM

② THE DEAD LOAD DEFLECTION SHOWN AT THE TENTH POINTS ARE THE DEFLECTIONS DUE TO DECK SLAB + HAUNCH + SIP STEEL DECK FORM ALLOWANCE + CONCRETE TRAFFIC RAIL. IT DOES NOT INCLUDE THE BEAM WEIGHT, DIAPHRAGMS OR FUTURE WEARING SURFACE.



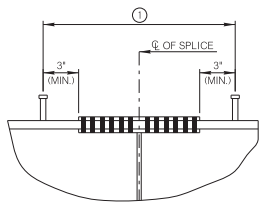
THEORETICAL SLAB HAUNCH

Ⓝ DIMENSION (BOTTOM OF SLAB TO TOP OF WEB) MAY VARY IF GIRDER CAMBER AFTER ERECTION DIFFERS FROM PLAN CAMBER BY MORE THAN THE % OF DEAD LOAD DEFLECTION DUE TO WEIGHT OF STRUCTURAL STEEL. NO PAYMENT WILL BE MADE FOR ADDITIONAL FORMING OR CONCRETE REQUIRED FOR VARIABLE HAUNCHING.



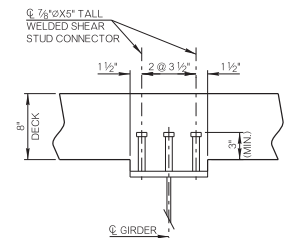
GIRDER ELEVATION

SUMMARY OF STRUCTURAL STEEL QUANTITIES	
ITEM	LBS.
AASHTO M270 GRADE 50W	
GIRDERS	602,700
TYPICAL WEB STIFFENER PLATE	9,910
BEARING STIFFENERS	6,710
SPLICE PLATES	16,080
FILL PLATES	1,620
INTERMEDIATE CROSS FRAMES	31,700
END CROSS FRAMES	10,870
TOTAL AASHTO M270 GRADE 50W	679,590
WELDED SHEAR STUD CONNECTORS	7,650

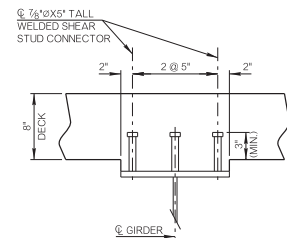


CLEARANCE AT SPLICE PLATE

① SHEAR CONNECTORS IN SPLICE PLATE REGION SHALL BE ADJUSTED BY FABRICATOR TO EACH SIDE OF CENTERLINE OF SPLICE.



10" FLANGE SHEAR STUD CONNECTOR DETAIL (SPAN 1)



14" FLANGE SHEAR STUD CONNECTOR DETAIL (SPANS 2-6)

* CONNECTION PLATES ARE ATTACHED ON BOTH SIDES OF THE WEB. OMIT CONNECTION PLATES ON EXTERIOR FACES OF GIRDERS 1 & 6. REFER TO FRAMING PLAN ON SUPERSTRUCTURE DETAILS FOR DIAPHRAGM CONNECTION PLATE SPACING.

LOCHNER		
DESIGN	JCG	1/15
DRAWN	JTC	1/15
CHECKED	FRY	1/16
FILE NAME	Girder	

OKLAHOMA DEPARTMENT OF TRANSPORTATION

GIRDER DETAILS

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