Section 7.5.1 (1975)

In the second sentence, change the title of the welding standard to " 'Reinforcing Steel Welding Code' (AWS D12.1-75)."

Section 7.6 (1976)

Retitle Section 7.6 as follows:

7.6—Splices of deformed bars and deformed wire in tension

Note: Wherever in Section 7.6 the word "bar" is used, the words "bar or wire" should be substituted, as the tension splice provisions apply equally to deformed bars and deformed wire.

Section 7.6.1 (1973)

Revise the first paragraph of Section 7.6.1 as follows:

7.6.1 *Classification of tension lap splices.* The minimum length of lap for tension lap splices shall be at least that given in this section, but not less than 12 in. l_d is the tensile development length for the full f_y as given in Sections 12.5(a), 12.5(b), 12.5(c), and 12.5(d).

Section 7.6.3 (1973)

Revise Section 7.6.3.1 through 7.6.3.2 as follows:

7.6.3.1—In regions of high computed stress. Splices in regions where the maximum computed design load stress in the bar equals or exceeds $0.5f_y$ shall meet the following requirements:

7.6.3.1.1—If no more than one-half of the bars are lap spliced within a required lap length, splices shall meet the requirements for Class B splices (lap of $1.3l_d$).

7.6.3.1.2—If more than one-half of the bars are lap spliced within a required lap length, splices shall meet the requirements for Class C splices (lap of $1.7l_d$).

7.6.3.1.3—Welded splices or positive connections if used shall meet the requirements of Section 7.5.5.1 or 7.5.5.2.

7.6.3.2—In regions of low computed stress. Splices in regions where the maximum computed design load stress in the bar is always less than $0.5f_y$ shall meet the following requirements:

Section 7.8 (1976)

Revise Section 7.8 to read as follows:

7.8—Splices of welded smooth wire fabric

7.8.1—Lapped splices in regions where the maximum computed design load stress in the wires equals or exceeds $0.5f_y$ shall be so made that the overlap measured between outermost cross wires of each fabric sheet is not less than the spacing of the cross wires plus 2 in., nor less than $1.5l_d$ or 6 in., whichever is greater, where l_d is the development length for the full f_y as given in Section 12.10.1.

7.8.2—Lapped splices in regions where the maximum computed design load stress in the wires is less than $0.5f_y$ shall be so made that the overlap measured between outermost cross wires of each fabric sheet is not less than 2 in. nor less than $1.5l_d$, where l_d is the development length for the full f_y as given in Section 12.10.1.

Note: Wherever in the Code the wording welded "plain" wire fabric is used the word "smooth" should be substituted to agree with current industry designation.

Section 7.9 (1974)

Revise Section 7.9 by adding the following at the end: "where A_w and s_w are the area and spacing of the wire to be spliced."

Section 7.9 (1976)

Revise Section 7.9 to read as follows:

7.9—Splices of welded deformed wire fabric

Lapped splices shall be so made that the overlap measured between outermost cross wires of each fabric sheet is not less than 2 in. The overall lapped splice length measured between the ends of each fabric sheet shall be not less than $1.7l_d$ nor 8 in., where l_d is the development length for the full f_y as given in Section 12.10.2.

Section 7.10.1 (1974)

Revise Section 7.10.1 by deleting the last sentence: "Bundled bars shall not be offset bent."

Section 7.12.3 (1975)

Revise the third sentence of Section 7.12.3 as follows:

The ties shall be so arranged that every corner and alternate longitudinal bar shall have lateral support provided by the corner of a tie having an included angle of not more than 135 deg and no bar shall be farther than 6 in. clear on either side along the tie from such a laterally supported bar.

CHAPTER 10—FLEXURE AND AXIAL LOADS

Section 10.2.7 (1976)

Revise the last sentence of Section 10.2.7 to read as follows:

The factor β_1 shall be taken as 0.85 for strengths, f_c', up to and including 4000 psi. For strengths above 4000 psi, β_1 shall be reduced continuously at a rate of 0.05 for each 1000 psi of strength in

excess of 4000 psi, but β_1 shall not be taken less than 0.65.

Section 10.3.4 (1973)

Revise Section 10.3.4 as follows:

10.3.4—All cross sections subject to a compression load shall be designed for the applied moments which can accompany this compression load, including slenderness effects according to the requirements of Sections 10.10 and 10.11.

Section 10.3.6 (1973)

Revise Section 10.3.6 as follows:

10.3.6—All members subjected to a compression load shall be designed for an eccentricity e equal to the greatest of

(a) that corresponding to the maximum moment which can accompany this compression load, or

(b) $0.05h$ for spirally reinforced and composite steel encased compression members, or $0.10h$ for tied compression members, about either principal axis, or

(c) 1 in. about either principal axis

For precast members, the 1 in. limit in Subsection (c) may be reduced to 0.6 in. provided the manufacturing and erection tolerances are limited to one-third of the minimum design eccentricity.

Slenderness effects shall be included according to the requirements of Sections 10.10 and 10.11.

Section 10.7 (1974)

Revise the first sentence of Section 10.7 as follows:

Flexural members with overall depth to clear span ratios greater than 2/5 for continuous spans, or 4/5 for simple spans, shall be designed as deep beams taking account of nonlinear distribution of stress and lateral buckling.

Section 10.12 (1973)

Revise Section 10.12 as follows:

10.12—Axially loaded members supporting slab systems

All axially loaded members supporting slab systems included in the scope of Section 13.1 shall be designed as provided in this chapter and in accordance with the additional requirements of Chapter 13.

Section 10.16.6 (1973)

Revise Section 10.16.6 as follows:

10.16.6—Horizontal reinforcement shall be spaced not further apart than three times the wall thickness nor 18 in.

CHAPTER 11—SHEAR AND TORSION

Section 11.0 (1973)

Change definition of M_{cr} to read:

M_{cr} = bending moment causing flexural cracking at the section considered due to superimposed loads. See Section 11.5.2.

Revise notation M_{max} as follows:

M_{max} = maximum bending moment at the section considered due to externally applied design loads

Delete V_i and substitute:

V_i = shear force at the section considered due to externally applied design loads occurring simultaneously with M_{max}

Section 11.4.3 (1975)

Revise Section 11.4.3 as follows:

11.4.3—For members subjected to axial compression the nominal shear stress v_c shall not exceed

$$v_c = 2\left(1 + \frac{N_u}{2000A_g}\right)\sqrt{f_c'} \qquad (11\text{-}6)$$

unless calculated in accordance with Section 11.4.3.1 or 11.4.3.2. The quantity N_u/A_g shall be expressed in psi.

11.4.3.1—When

$$M_m = M_u - N_u\frac{(4h - d)}{8} \qquad (11\text{-}5)$$

is positive, Eq (11-4) may be used to determine v_c with M_m substituted for M_u and $V_u d/M_m$ not then limited to 1.0. However, v_c shall not exceed

$$v_c = 3.5\sqrt{f_c'}\sqrt{1 + \frac{N_u}{500A_g}} \qquad (11\text{-}7)$$

The quantity N_u/A_g shall be expressed in psi.

11.4.3.2—When M_m as computed by Eq. (11-5) is negative, v_c shall be computed by Eq. (11-7).