



4.2 STRUCTURAL DESIGN

4.2.1 Design Principles

The forces acting on a cross section of pipeline arise from three main sources:

- A) Weight of overlying fill, including any local surcharge.
- B) Soil pressures transmitted to the pipe from surface loads, i.e. traffic and other transient loads.
- C) Supporting reaction below the pipe.

The weight of water within the pipe is only significant for larger diameter pipes.

A: Weight of overlying fill

There are four main conditions in which pipes are installed:

- a) "Narrow" trench.
- b) "Wide" trench, or on the surface of ground over which an embankment is then built (positive projection condition).
- c) Narrow trench over which an embankment is then built (negative projection condition).
- d) Tunnel, heading or by jacking.

The load W_c imposed by the backfill on a pipe in a "narrow" trench can be found from Marston's formula from which the Tables have been compiled in Section 4.2.7.

These Tables are only applicable to rigid pipes laid in "Narrow" trench conditions.

B: Traffic and other transient loads

Measurements have shown that on large civil engineering works pipes may well be subjected to their highest loads during construction. Here, three categories of traffic loading are considered and rigid pipes should normally be designed to withstand the most onerous likely to occur.

If during construction it is clear that excessive site traffic loading will occur, the design should be checked accordingly or special crossing places must be designated.

- a) **Main road loading** is intended to apply to all main traffic routes and to roads liable to be used for the temporary diversion of heavy traffic.

As a guide it may be assumed that such roads carry at least 200 commercial vehicles per day in each direction. HA and HB loading are assumed to use such roads (see BS 5400).

- b) **Light road loading** applies to all other roads where

heavy traffic is unlikely to pass.

- c) **Field loading** applies to fields, gardens and lightly trafficked access tracks. This loading is also considered to be adequate to cater for occasional heaps or stacks of materials on the ground surface. Massive heaps or stacks likely to produce a more severe loading should be treated as a special design.

In assessing the loading category, regard should be paid to the possible future upgrading of a road. Pipes under verges should normally be treated as though under the road, with the possible exception of motorways and trunk roads and should take account of any planned road improvement. For non-public roads such as estate roads or roads within works, an assessment should be made of the heaviest vehicle likely to use the road, and one of the above three loading conditions selected as appropriate.

C: Supporting reaction below the pipe

British Standards for concrete pipes give maximum crushing loads for each diameter and strength class of pipe. Loads are applied in a 3 edged loading test described in BS 5911. The pipe must not collapse under the maximum load specified.

Proof test loads are also specified. Reinforced pipes must not crack by more than a specified amount under the proof load. The only proof load test for unreinforced pipes is the maximum load.

Pipes of a small diameter (up to DN 300) may fail as a beam. BS 5911 includes suitable values of bending moment resistance.

Pipe bedding

This term is used to describe the complete arc of material within the trench, or in the case of class "C" or class "D" beddings, a special preparation of the trench bottom.

Bedding factor

In the standard test on pipes the vertical loading and supporting reactions are line loads and any trench situation in the field is unlikely to produce such an onerous loading condition. The strength of the pipe determined in the crushing test can therefore be multiplied by a bedding factor which represents the amount by which the stresses in the pipe are reduced because of the spreading properties of the bedding for load and reaction.

The value of a bedding factor for a particular method of construction is not a precise figure but is affected by the quality of workmanship. The values given whilst being conservative assume a reasonable standard of workmanship and supervision. If the designer needs a somewhat higher bedding factor than stated a high standard of workmanship and supervision must be specified and guaranteed; alternatively a higher strength pipe may be considered. If this is an extra strength pipe adequate time must be allowed for the manufacturer to supply.

Factor of safety

To allow for unexpected site conditions or the vagaries of construction a minimum factor of safety of 1.25 should be allowed on the calculated loads.

4.2.2 Design Assumptions

Surface Conditions

The Tables in Section 4.2.7 are applicable only to a single pipeline laid in its own trench, and have been set out to give the loads on pipes under three surface conditions, Main Roads, Light Roads and Fields.

Backfill loads

The Tables are calculated using an equivalent soil density of 19.6 kN/m³ (approximately 2 tonnes/m³).

Traffic loads

The loads referred to in the design principles have values as follows:-

a) Main roads

Static wheel load of 86.5kN and an impact factor of 1.3, giving a Total Static wheel load of 112.5kN; contact pressure 1100kN/m².

b) Light roads

Static wheel load of 70kN and an impact factor of 1.5, giving a Total Static wheel load of 105kN; contact pressure 700 kN/m².

c) Fields

Static wheel load of 30kN and an impact factor of 2.0, giving a Total Static wheel load of 60 kN; contact pressure 400kN/m².

Superimposed loads

These are not included in the Tables. If however such loads are encountered and are of sufficient magnitude, an allowance should be made.

Water Loads

These are included in the Tables. If the pipe is laid below the ground water table, an allowance for this load is not needed. However, as these loads are small by comparison with other loads on the pipe, it has been considered appropriate to include them only for pipes of DN 600 and over.

Frictional factor $K\mu$

A value of 0.13 has been used for narrow trench conditions.

Minimum cover over pipe

a) It is advisable that pipes laid under roads should have cover over the pipe of not less than 1.2m. This cover should be maintained for main roads, light roads (which may on occasion carry main road traffic) and for pipes laid under grass verges adjacent to a road (Tables B3 and B4). Where pipes have to be laid with less than 1.2m cover special consideration is needed to reduce the risk of damage. Loads in columns headed 0.9 and 1.0 in Tables B3 and B4 should be used only as a guide.

b) For pipes laid in fields a minimum cover of 0.6m should be provided. At shallower depths there is a risk of damage from agricultural operations.

4.2.3 Design Method

The established method for calculation of loads on buried rigid pipes is summarised in BS EN 1295 National Annex A, the principles of which are explained below.

In general pipelines are laid in trenches and the pipes used are designed to carry the backfill, traffic loads and, when the diameter is 600mm or more, some part of the water load under working conditions.

In order to improve the load carrying capacity of the pipe it is laid on one of several classes of bedding (see Table B2). Each type of bedding is allocated a "bedding factor" (F_m) which may be regarded as a multiplier applied to the test load of the pipe.

The trench is excavated in the natural soil, the pipe is laid on the selected bedding and the trench backfilled. Load on the pipe due to the backfill develops as the fill material settles. The load on the pipe due to the backfill is therefore the weight of the backfill taken over the full trench width but reduced by the shear force from the trench walls acting upwards (see Fig.B1). The backfill load is calculated by using the Marston formula:

$$W_c = C_d w B_d^2$$

where:

W_c = Backfill load (KN/m)

C_d = Load coefficient, dependent on soil type and the ratio of cover depth to trench width

w = Soil density (kN/m³)

B_d = Width of trench (m)

Provided that the trench width does not exceed the values given in the tables, the loads given are conservative and may be used with confidence.

The trench widths given will provide adequate working space around the pipe for laying and jointing and also sufficient room to place and consolidate the bedding specified.

As indicated, the friction acting against the backfill is provided by the trench walls and is roughly constant at a particular depth. If however the trench width is increased radically, B_d^2 in the Marston formula is also increased and a reappraisal of the load on the pipe must be considered.



Fig. B1

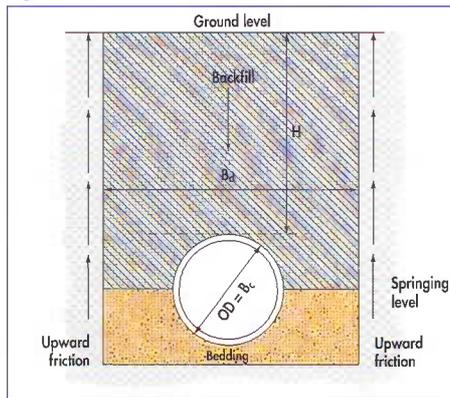
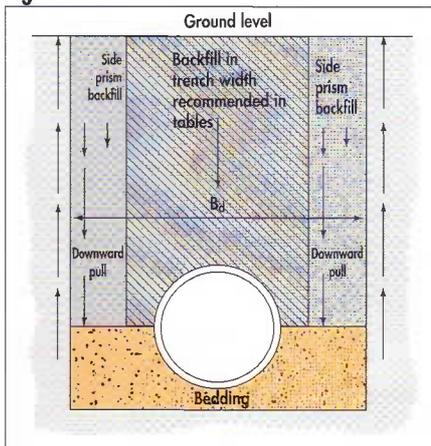


Fig. B2



For any depth there is a trench width where friction planes from the trench walls become remote from the pipe and no longer contribute to the reduction of the fill load. In fact the settlement of the side prisms of backfill tend to increase the

**Table B1.
Crushing Test Loads: W_c**

Crushing test loads kN/m			
Nominal size of pipe DN	Class L Maximum load	Class M Maximum load	Class H Maximum load
225	25	29	35
300	25	29	40
375	25	39	45
450	25	44	52
525	25	48	58
600	25	58	68
675	25	63	75
750	48	67	81
825	52	72	86
900	58	84	106
975	60	90	114
1050	64	95	120
1125	67	103	133
1200	72	109	138
1350	79	120	153
1500	87	130	165
1650	94	145	183
1800	103	155	193
1950	110	169	212
2100	120	183	230
2250	128	194	244
2400	135	207	263
2550	145	222	279
2700	155	233	294
2850	163	244	314
3000	169	259	326

load (see Fig.B2). This state is called the wide trench condition. It is a positive projection condition.

In preparing the tables, due consideration has been given as to whether at any trench width and depth, the narrow or wide trench condition and load is applicable, and the standard practice of using the lesser of these values has been adopted.

The tables give the total loads for pipes of all diameters specified in BS 5911: Part 100. This load includes loading from backfill and traffic for depths of cover over the top of the pipe as follows:

Main Roads and Light Roads - 1.2m to 8.0m
Fields - 0.6m to 8.0m

For DN 600 and above the water load shown is also included.

4.2.4 Pipe Bedding

The load bearing capacity of an installed pipeline relates directly to the construction of the bedding which is intended to level out any irregularities in the formation, and provide uniform support around and along the length of the pipe barrel.

Pipe settlement will be kept to a minimum by the proper selection and compaction of the bedding material. The bedding should be compacted to a density not less than that of the natural soil in the sides and bottom of the trench. The bedding directly beneath or above the pipeline must not be over compacted otherwise line loading of the pipes will result.

On steep gradients, or where dewatering has taken place, it is important to restrict ground water movement within the completed trench. Selection of bedding or clay dams across the full width of the trench will assist in this.

Under no circumstances should blocks or bricks be placed beneath pipes. Any pegs used for setting out or levelling must be removed.

Bedding materials

Any stable soil will act adequately as a bedding material provided that it is placed and compacted around the pipeline. From a practical point of view granular material is compacted more readily and has become widely accepted.

The bedding material should be of similar particle size to that in the trench sides. Where the ground is clay or silt, bedding material must consist of all-in gravels to prevent the trench from becoming a drainage channel and carrying away fines from the trench walls and bedding and causing settlement of the pipes.

Granular bedding material

The ideal is crushed rock or gravel but similar locally available material having an angular or an irregular shape may be used. Rounded single sized material is not recommended as it may not provide a stable bed especially for heavy larger diameter pipes.

WRc Information and Guidance Note (IGN) 4-08-01

provides guidance on the particle size of material relating to pipe diameter.

Sands containing an excess of fine particles are more difficult to place and compact and will require a greater degree of supervision on site to achieve a stable embedment for the pipeline.

Selected bedding and fill material

This should consist of uniform readily compatible material, free from tree roots, vegetable matter, building rubbish and frozen soil. When used as fill, the material should not contain large clay lumps or cobbles. When used as bedding, all clay lumps should be excluded.

“As dug” material may be used provided that it is readily compatible and provides stable embedment.

Classes of bedding and bedding factors

The strength of an installed pipeline depends on a combination of the strength of the pipe and the class of bedding.

The selection of the bedding class is influenced by many factors, which include the nature of the ground, the loads acting on the pipeline in the trench, availability of a particular strength class of pipe, and the local cost and availability of the bedding material.

Where imported granular material is expensive and where supervision of site work cannot be guaranteed, it is preferable to specify a lower class of bedding with a higher strength pipe, which is proved by works testing, rather than vice-versa.

Taking into account the cost of labour, it is generally more economical to lay the pipes on a bedding of non-cohesive materials, or alternatively scarify the trench bottom rather than hand trim the formation.

Normally loading calculations are made considering the pipeline in complete lengths, between manholes. The calculated strength class for a pipe to satisfy the most severe loading condition between each pair of manholes is then used throughout the length. However there are occasions when it may be necessary to use a higher bedding class for a short distance where locally the load is increased, for instance at a road crossing or in an embankment.

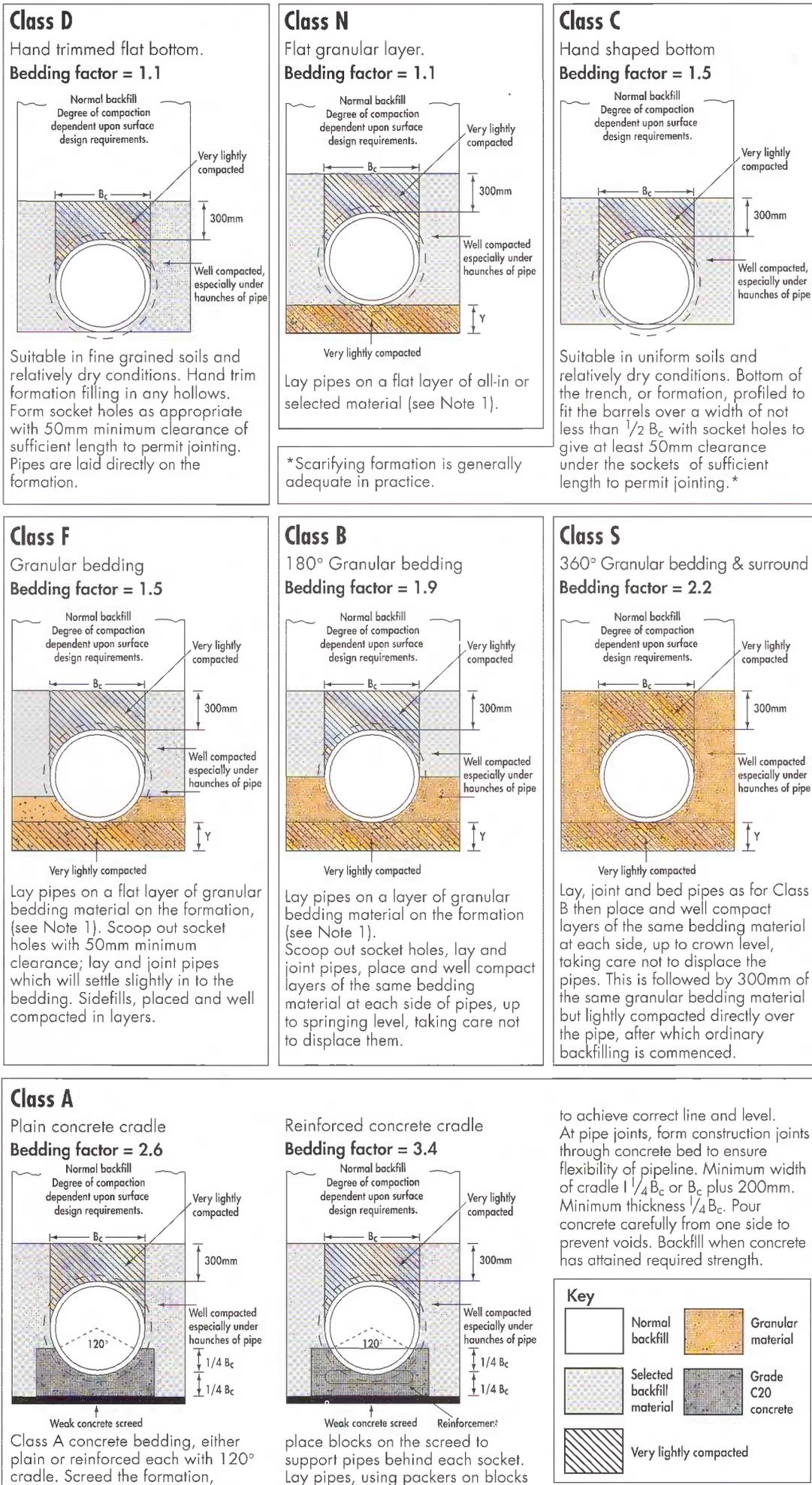
The normally accepted classes of pipe bedding are shown in Table B2 and in Fig. B3.

Table B2.
Types of Bedding

Bedding Class	Bedding factor	Description	Suitability
Class D	1.1	Hand trimmed flat bottom/formation	Fine grained soils, relatively dry conditions
Class N	1.1	Flat bed of granular all-in or selected material	Rock, mixed soils
Class C	1.5	Shaped formation (or scarify)	Uniform soils relatively dry
Class F	1.5	Shaped bedding of granular material	General
Class B	1.9	180° non cohesive bedding material	General
Class S	2.2	Complete surround of non cohesive bedding material	General
Class A Plain	2.6	Plain concrete cradle	Seldom necessary. Higher strength pipe with granular bedding is more practicable and economic option.
Class A Reinforced	3.4	Reinforced concrete cradle	
Concrete Arches	Sometimes used where an existing pipeline is to be surcharged with additional fill or a road construction. Use the same bedding factor as Class A cradles but calculate loadings using the arch dimensions instead of the outside diameter of the pipe.		
Geotextiles	Where appropriate, geotextiles may be used to contain bedding materials e.g. in running sand		



Fig. B3
Types of bedding





NOTES:

1. Generally thickness of bedding (Y), minimum of 100mm under barrels and 50mm under sockets. In rock 200mm under barrels and 150mm under sockets. Minimal compaction directly beneath pipe.
2. Sidefills, whether of bedding material or of selected material, must be well compacted.
3. Backfill or bedding material to be highly compacted above sidefills to 300mm above the crown but lightly compacted directly over the pipe.
4. Normal backfill to be compacted as appropriate.
5. With reasonable workmanship and supervision these bedding factors are conservative.

4.2.5 Design Calculations

The calculated load "W_e", which is the total load a concrete pipe in a trench is required to sustain, is used in the design formula as follows:

$$W_t = \frac{W_e \times F_{se}}{F_m}$$

where W_t is the required BS test strength.
 W_e is the load from Tables B3 or B4.
 F_{se} is the factor of safety.
 F_m is the bedding factor chosen.

Test strength of pipe (W_t)

The test strength of a concrete pipe may be referred to as W_p or W_t. Values are specified in BS 5911. (See Table B1).

For a reinforced concrete pipe W_p is the load which the pipe will sustain without developing a crack exceeding 0.25mm in width over a length of 300mm and W_t is the load which the pipe will sustain without collapse, irrespective of crack width.

However, to further simplify the procedure it is more straightforward to use in the calculation the maximum test load in every case and applying the factor of safety of 1.25 throughout.

4.2.6 Worked Examples

The symbols used in the examples are those referred to in Design Calculations (Section 4.2.4).

Example 1

What strength class is required of a 900mm concrete pipe to be laid under a main road with a cover over the top of the pipe 4.6m and on a 180° granular bed? (F_m = 1.9).

From Table B3 Main Roads.

$$\begin{aligned} \text{Total load} &= 149 \text{ kN/m} \\ F_{se} &= 1.25 \\ F_m &= 1.9 \end{aligned}$$

$$W_t = \frac{149 \times 1.25}{1.9} = 98.0 \text{ kN/m}$$

From Table B1.

900mm Class H Pipe - W_t = 106 kN/m

900mm Class M Pipe - W_t = 84 kN/m

Class H Pipe is required.

Note

Using Class S bedding W_t = 84.7kN/m and a Class M pipe could be used.

Example 2

The same pipe as in Example 1 passes under a light road with a depth cover of 7.4m. What class of bedding is required?

W_t = 106 kN/m (see Example 1)

From Table B4. Light Roads

$$W_e = 181 \text{ kN/m}$$

$$W_t = \frac{W_e \times 1.25}{F_m}$$

$$F_m = \frac{181 \times 1.25}{106} = 2.13$$

From Table B2, it will be seen that either Class A Plain bedding with F_m = 2.6, or Class S bedding with F_m = 2.2 may be used.

However it would be advisable (at this diameter) to compare the costs of the two beddings and the standard of workmanship before making a decision as to the bedding to be used.

Example 3

A 900mm pipeline consisting of Class M concrete pipes on a Class B bedding is to be laid across fields. What is the greatest depth of cover at which these pipes can be laid?

$$W_t = 84 \text{ kN/m}$$

$$F_m = 1.9$$

$$\text{Total load to be carried} = \frac{84 \times 1.9}{1.25} = 128.0 \text{ kN/m}$$

From Table B5. Fields.

Total load at 4.2m = 126 kN/m.

Total load at 4.4m = 130 kN/m.

Maximum cover under these conditions is 4.3m (approx.)

4.2.7 Load Tables

Table B3

Total Design Loads - Main Roads. "H" = 0.9 metres to 8.0 metres

Nominal Dia. in mm	Recom. Outside Trench Width	Water Load included in kN/m	Total design load "W _e " in kilonewtons per metre of pipe for cover depths "H" in metres																																																						
			0.9	1.0	1.2	1.4	1.6	1.8	2.0	2.2	2.4	2.6	2.8	3.0	3.2	3.4	3.6	3.8	4.0	4.2	4.4	4.6	4.8	5.0	5.2	5.4	5.6	5.8	6.0	6.2	6.4	6.6	6.8	7.0	7.2	7.4	7.6	7.8	8.0																		
275	280	0.7	31.9	31.4	28.5	28.3	27.7	27.9	28.5	28.6	29.9	30.7	31.4	32.0	32.2	32.4	32.6	32.9	33.2	33.3	33.5	33.7	33.9	34.1	34.2	34.3	34.5	34.7	34.9	35	35.1	35.1	35.2	35.2	35.3	35.3	35.4	35.4	35.4																		
300	300	0.75	43.4	38.9	37.8	38.3	37.8	37.9	38.0	38.1	38.2	38.3	38.5	38.7	38.9	39.0	39.2	39.5	39.8	40.1	40.4	40.6	40.8	41.1	41.3	41.5	41.7	41.9	42.1	42.3	42.4	42.5	42.7	42.8	42.9	43.0	43.2	43.2	43.3	43.3	43.3																
375	500	1.05	55.7	53.9	50.1	49.1	48.1	49.4	50.5	51.6	52.6	54.3	56.1	57.8	58.8	59.9	60.9	61.9	62.9	63.7	64.5	65.4	66.3	67.3	67.9	68.7	69.4	70.2	70.9	71.5	72.2	72.8	73.5	74.0	74.6	75.1	75.7	76.3	76.9	77.5	78.1	78.7															
450	580	1.15	64.3	62.8	55.7	58.4	57.3	58.0	59.3	60.7	62.1	63.2	64.2	65.4	66.1	66.8	67.8	68.8	69.7	70.8	71.9	73.0	74.0	74.8	75.7	76.7	77.5	78.4	79.2	80.0	80.9	81.7	82.5	83.2	83.9	84.4	85.0	85.6	86.2	86.8	87.4	88.0	88.6														
525	670	1.20	73.2	71.6	66.7	65.7	66.2	66.8	67.6	68.4	69.0	70.0	71.0	72.0	72.8	73.7	74.7	75.8	76.9	78.0	79.1	80.3	81.4	82.5	83.7	84.5	85.3	86.2	87.3	88.4	89.4	90.4	91.3	92.3	93.0	93.7	94.5	95.2	95.8	96.5	97.2	97.9	98.6	99.3													
600	790	1.35	86.5	82.4	79.2	78.9	79.1	79.6	80.4	81.4	82.4	83.5	84.7	86.1	87.0	88.0	89.4	90.9	92.3	93.7	95.1	96.6	98.1	99.5	101	102	104	105	107	108	109	110	111	112	113	114	115	116	117	118	119	120	121	122	123	124											
675	880	1.45	95.8	93.2	87.3	86.7	87.0	87.3	88.1	88.9	89.5	90.8	92.1	93.4	94.4	95.4	97.0	98.6	100	102	103	105	107	108	110	111	113	114	116	117	119	120	121	122	124	125	126	128	129	130	131	132	133	134	135	136	137	138	139	140	141						
750	950	1.50	104	100	94.5	93.4	93.6	94.0	94.7	95.5	96.2	97.6	99.0	100	102	103	104	106	108	109	111	113	115	117	119	120	122	123	125	126	128	129	131	132	134	135	137	138	139	140	141	142	143	144	145	146	147	148	149	150	151	152	153				
825	1040	1.60	113	109	103	102	101	101	102	103	104	105	107	108	109	110	112	114	116	118	119	121	123	125	127	129	131	133	135	136	138	140	142	143	145	147	148	149	150	151	152	153	154	155	156	157	158	159	160	161	162	163	164				
900	1120	1.90	122	118	111	110	111	111	112	115	118	121	124	126	128	130	132	135	137	140	143	146	149	152	155	157	160	163	166	169	171	174	177	179	182	184	186	189	192	193	194	195	196	197	198	199	200	201	202	203	204	205	206				
975	1200	2.00	131	127	119	117	117	119	123	126	129	131	133	136	138	140	143	146	148	151	155	158	161	164	167	170	173	175	178	181	184	187	190	192	195	198	201	204	207	210	214	218	221	225	228	231	234	238	241	245	249	253					
1050	1300	2.05	141	135	128	125	127	128	130	132	136	139	141	144	146	148	151	154	157	160	164	167	170	173	176	179	183	186	189	192	195	198	201	204	207	210	214	218	221	225	228	231	234	238	241	245	249	253	257	261	265						
1125	1370	2.20	147	143	136	132	133	134	138	143	147	150	152	155	158	160	164	167	171	175	178	182	185	189	193	196	200	204	207	211	214	218	221	225	228	231	234	238	241	245	249	253	257	261	265	269	273	277	281	285	289						
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1500	1830	2.60	180	177	171	166	167	170	174	178	181	184	187	190	193	196	198	201	205	209	214	218	223	228	232	237	241	246	251	255	260	264	269	273	278	282	287	291	296	300	305	310	315	320	325	330	335	340	345	350	355	360	365	370			
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2400	2890	3.55	279	273	267	262	263	267	271	275	279	283	287	291	295	299	303	307	311	315	319	323	327	331	335	339	343	347	351	355	359	363	367	371	375	379	383	387	391	395	399	403	407	411	415	419	423	427	431	435	439	443	447				
2550	3050	3.70	299	293	287	282	283	287	291	295	299	303	307	311	315	319	323	327	331	335	339	343	347	351	355	359	363	367	371	375	379	383	387	391	395	399	403	407	411	415	419	423	427	431	435	439	443	447	451	455	459	463	467				
2700	3210	3.85	320	314	308	303	304	308	312	316	320	324	328	332	336	340	344	348	352	356	360	364	368	372	376	380	384	388	392	396	400	404	408	412	416	420	424	428	432	436	440	444	448	452	456	460	464	468	472	476	480	484	488				
2850	3370	4.00	342	336	330	325	326	330	334	338	342	346	350	354	358	362	366	370	374	378	382	386	390	394	398	402	406	410	414	418	422	426	430	434	438	442	446	450	454	458	462	466	470	474	478	482	486	490	494	498	502	506	510				
3000	3540	4.15	364	358	352	347	348	352	356	360	364	368	372	376	380	384	388	392	396	400	404	408	412	416	420	424	428	432	436	440	444	448	452	456	460	464	468	472	476	480	484	488	492	496	500	504	508	512	516	520	524	528	532	536	540		
3000	3540	4.15	396	390	384	379	380	384	388	392	396	400	404	408	412	416	420	424	428	432	436	440	444	448	452	456	460	464	468	472	476	480	484	488	492	496	500	504	508	512	516	520	524	528	532	536	540	544	548	552	556	560	564	568	572	576	580

Note 1. Pipes at shallower depths will need special consideration.



**PART B
SYSTEM
DESIGN**

**SECTION 4.
SYSTEM
DESIGN**

**4.1
HYDRAULICS**

**4.2
STRUCTURAL
DESIGN**

**4.3
MANHOLE
DESIGN**

**Table B5
Total Design Loads - Fields, etc.. "H" = 0.6 metres to 8.0 metres**

Nominal Dia. in mm	Water Load included in kW/m	Recom- mended Trench Width	Total design load "W" in kilonewtons per metre of pipe for cover depths "H" in metres																																									
			0.6	0.9	1.0	1.2	1.4	1.6	1.8	2.0	2.2	2.4	2.6	2.8	3.0	3.2	3.4	3.6	3.8	4.0	4.2	4.4	4.6	4.8	5.0	5.2	5.4	5.6	5.8	6.0	6.2	6.4	6.6	6.8	7.0	7.2	7.4	7.6	7.8	8.0				
225	280	0.7	23.5	21.0	20.0	18.2	18.0	18.1	19.0	20.2	21.4	22.8	24.0	25.0	26.0	26.5	27.0	27.5	28.0	28.5	28.9	29.4	29.9	30.3	30.7	31.0	31.3	31.6	31.9	32.1	32.5	32.8	32.9	33.1	33.2	33.4	33.5	33.6	33.7	33.9	32.5			
300	380	0.75	31.2	25.7	24.7	23.9	24.0	24.7	25.8	26.5	27.3	28.0	29.0	29.7	30.5	31.0	31.7	32.3	33.0	33.7	34.3	34.8	35.5	36.0	36.5	37.0	37.5	37.9	38.3	38.5	38.8	39.2	39.5	39.8	40.1	40.4	40.6	40.9	41.2	41.4	41.4	300		
375	500	1.05	39.1	33.0	32.0	31.0	31.2	32.1	33.7	35.8	37.7	39.6	42.0	44.4	46.9	48.6	50.3	51.8	53.3	54.9	56.2	57.6	58.8	60.0	61.2	62.5	63.6	64.6	65.7	66.6	67.5	68.4	69.1	69.9	70.4	71.4	72.2	72.8	73.4	74.0	375			
450	580	1.15	46.1	37.7	36.9	36.4	36.8	37.7	39.3	41.6	43.5	45.4	47.2	49.1	51.0	52.9	54.8	56.6	58.5	60.5	61.9	63.7	65.2	66.6	68.0	69.4	70.6	71.8	73.0	74.0	75.1	76.3	77.3	78.2	79.1	80.0	80.7	81.5	82.3	83.1	450			
525	670	1.20	52.2	42.7	41.9	41.4	42.2	43.2	45.4	47.6	49.6	51.3	53.3	55.3	57.4	58.9	60.5	62.4	64.3	66.2	67.9	69.7	71.5	73.1	74.6	76.3	77.7	79.0	80.5	81.8	83.1	84.3	85.4	86.5	87.7	88.7	89.6	90.6	91.6	92.6	525			
600	790	1.35	61.7	50.7	49.5	48.9	50.5	53.0	54.8	57.4	59.3	61.4	63.7	66.2	69.0	70.9	72.7	75.0	77.4	79.6	81.8	83.9	86.2	88.3	90.3	92.5	94.3	96.0	97.9	99.6	101	103	104	106	107	108	109	111	112	113	600			
675	880	1.45	68.0	56.1	53.9	53.5	54.9	56.9	59.3	61.7	64.0	66.4	69.1	71.4	74.0	76.5	78.5	81.0	83.6	86.2	88.7	91.1	93.6	95.8	98.1	100	102	104	106	108	110	112	113	115	116	118	120	121	123	124	124	675		
750	950	1.50	73.1	60.9	58.8	57.8	59.3	61.3	63.7	65.7	68.7	71	74.0	76.9	79.8	82.4	85.0	87.6	90.2	92.7	95.1	97.8	101	103	106	108	110	113	115	117	118	120	122	124	126	128	130	132	134	136	136	750		
825	1040	1.60	78.2	66.2	64.7	62.8	64.2	66.2	68.4	70.6	73.6	76.1	79.4	82.4	85.4	87.8	90.1	93.2	96.2	99.2	102	105	108	111	113	116	119	121	124	126	128	130	132	134	136	138	140	142	144	146	825			
900	1120	1.90	83.8	71.1	68.7	67.3	68.2	71.1	76.2	82.4	87.8	91.7	96.1	100	104	107	111	114	118	123	126	130	134	138	142	146	149	152	156	159	162	165	168	172	175	177	180	182	185	187	900			
975	1200	2.00	89.1	76.1	71.1	72.1	73.1	76.5	81.0	87.3	92.7	96.9	101	105	110	113	118	121	125	129	134	138	142	146	150	154	158	161	164	168	171	175	178	181	184	188	191	194	197	200	975			
1050	1300	2.05	94.4	81.3	78.9	77.2	78.5	81.4	85.9	92.2	98.1	102	107	112	116	119	123	127	132	137	141	146	150	154	158	162	167	171	176	179	182	185	188	192	195	198	202	205	209	213	1050			
1125	1370	2.20	99.2	86.0	83.9	81.2	83.2	86.3	90.6	97.1	104	109	116	122	126	130	133	139	144	149	154	159	164	169	173	178	182	187	191	195	199	203	208	212	216	219	223	226	230	234	1125			
1200	1490	2.30	102	91.9	89.2	87.9	89.0	92.2	97.1	104	110	116	122	127	132	136	140	145	150	156	161	167	172	177	182	187	192	196	201	205	209	213	218	222	227	231	235	238	242	246	1200			
1350	1650	2.45	106	95.1	92.1	90.8	91.9	95.1	100	107	115	120	127	132	138	144	150	155	160	166	171	176	182	188	193	199	204	209	214	219	224	229	234	239	243	248	252	257	261	266	271	1350		
1500	1830	2.60	112	100	98.1	96.8	97.9	101	106	113	118	126	131	138	144	152	156	161	167	173	179	185	191	198	204	210	216	222	228	233	239	244	249	255	260	265	270	275	280	285	290	1500		
1650	2010	2.80	117	105	103	101	102	107	114	120	125	129	137	144	152	159	167	174	180	187	193	199	206	213	221	227	234	241	247	254	260	267	273	279	285	291	297	303	309	314	320	326	332	1650
1800	2240	2.95	124	110	108	106	107	112	119	124	128	132	138	144	152	159	167	174	180	187	193	199	206	213	221	227	234	241	247	254	260	267	273	279	285	291	297	303	309	314	320	326	332	1800
1950	2400	3.10	129	115	113	111	112	117	124	129	133	137	142	148	156	163	170	176	182	188	194	200	206	213	221	227	234	241	247	254	260	267	273	279	285	291	297	303	309	314	320	326	332	1950
2100	2560	3.25	134	120	118	116	117	122	129	134	138	143	148	154	162	169	176	182	188	194	200	206	213	221	227	234	241	247	254	260	267	273	279	285	291	297	303	309	314	320	326	332	2100	
2250	2730	3.40	139	125	123	121	122	127	134	139	143	148	154	160	168	175	182	188	194	200	206	213	221	227	234	241	247	254	260	267	273	279	285	291	297	303	309	314	320	326	332	2250		
2400	2890	3.55	144	130	128	126	127	132	139	144	148	154	160	166	174	181	188	194	200	206	213	221	227	234	241	247	254	260	267	273	279	285	291	297	303	309	314	320	326	332	2400			
2550	3050	3.70	149	135	133	131	132	137	144	149	153	159	165	171	178	185	192	198	204	210	216	222	228	234	240	246	252	258	264	270	276	282	288	294	299	305	311	317	323	329	335	2550		
2700	3210	3.85	154	140	138	136	137	142	149	154	158	164	170	176	182	188	194	200	206	212	218	224	230	236	242	248	254	260	266	272	278	284	290	296	302	308	314	320	326	332	2700			
2850	3370	4.00	159	145	143	141	142	147	154	159	163	169	175	181	187	193	199	205	211	217	223	229	235	241	247	253	259	265	271	277	283	289	295	301	307	313	319	325	331	337	2850			
3000	3540	4.15	164	150	148	146	147	152	159	164	168	174	180	186	192	198	204	210	216	222	228	234	240	246	252	258	264	270	276	282	288	294	300	306	312	318	324	330	336	342	348	3000		